TRANSPORTATION IMPACT STUDY FOR

Arborview

Owner : Nelson Wingert etal.

Applicant/Developer: 1849 Development, LLC 611 Gettysburg Pike Suite 101 Mechanicsburg, PA 17055 REP: Don Farinelli

Site Location:
Gettysburg Pike
Upper Allen Township,
Cumberland County, Pennsylvania

February 28, 2018 Revised April 9, 2018

Prepared by:



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FORWARD

This report provides a traffic impact analysis for proposed residential facilities in Upper Allen Township. The report is organized into 3 sections.

- Executive Summary A brief 4 page summary of the study, results, and recommendations. Also included within the executive summary is a tabular summary of estimated intersection capacity level-of-service, delay, and volumeto-capacity ratios.
- II. Traffic Impact Study A stand-alone text document describing in more detail elements of analysis.
- III. Appendix A Supporting documents including; Existing Volume/LOS Figures, Trip Distribution Percentage and Volumes Figures, Opening Year Conditions Figures, Horizon Year Conditions Figures, Site Photos, Existing Data, Traffic Count Data Sheets, Growth Rates and Volume Worksheets, and Trip Generation Data Sheets, Turn Lane Analysis, and Correspondence.

REVISION NOTES

April 09, 2018 - Revisions per Township Memo dated March 19, 2018.

- Include capacity analysis for design horizon year 2030.
- Updates to figures 4 and 5A.

February 28, 2018 – The initial study to be submitted to Upper Allen Township for review as part of the subdivision and land development application process.

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Executive Summary

EXECUTIVE SUMMARY

ALPHA Consulting Engineers Inc. has prepared a traffic impact study for 1849 Development, LLC to estimate traffic impacts related to proposed residential facilities. As part of the study, this executive summary is provided as a brief, concise, project overview.

1849 Development, LLC is proposing to construct residential facilities on approximately 15 acres of land located along Gettysburg Pike in Upper Allen Township. Development will include the construction of 22 single family detached residential units along with the construction of approximately 1,330 linear feet of public street. The proposed development site is bounded by Gettysburg Pike on the east, the 'Arborfield' residential development on the south, and the 'Meadowview' residential development on the west. Vehicular access to the facility is proposed via a full movement site driveway along Gettysburg Pike near the eastern limits of the property and a extension of Coventry Drive into the site.

The new development is estimated to generate approximately 258 new vehicle trips on an average weekday. The trip generation estimate includes approximately 20 vehicle trips during the morning or AM peak hour of the street and approximately 24 vehicle trips during the evening or PM peak hour of the street.

Traffic analysis was conducted at the following offsite intersections for traffic conditions occurring during the current 2018 year along with future scenarios under the 2020 opening year and 2030 horizon year:

Fisher Road – Gettysburg Pike, un-signalized intersection.

Traffic analysis for future development scenarios was conducted under the 2020 opening year and 2030 horizon year at the following proposed site driveway intersection:

• Site Driveway 1 – Gettysburg Pike, un-signalized intersection.

Analysis indicates that the proposed site driveway intersection will operate at acceptable levels as describe under Township criteria for all build scenarios. Acceptable levels for urban areas are considered a level of service (LOS) 'D' or better.

During the AM peak hour, the intersection of Fisher Road and Gettysburg Pike currently operates at an acceptable LOS 'A' and is estimated to continue to operate at LOS 'A' under the 2020 and 2030 design years both with and without the development.

During the PM peak hour, the intersection of Fisher Road and Gettysburg Pike currently operates at an acceptable LOS 'A' and is estimated to continue to operate at LOS 'A' under the 2020 and 2030 design years both with and without the development.

Average intersection delay is estimated to increase by less than 10 seconds for the peak hours with the addition of the site generated traffic. The development generated traffic is not estimated to impact the offsite intersection at levels that would require mitigation/improvements.

Queue lengths (95th percentile) along Gettysburg Pike at the southbound approach to the intersection with Fisher are estimated to continue to be less than the distance to the proposed full movement driveway. Average queue lengths will not impact normal turning movements at the site driveway.

Right and left turn lane warrant analysis were conducted for the proposed site driveway intersection with Gettysburg Pike. Neither right nor left turn lanes are warranted at the entrance of this development.

In summary, offsite improvements are not recommended as the additional traffic generated by the proposed development will not impact the study intersections at levels that would normally require mitigation.

Site access is recommended to be constructed as follows:

- Construct full movement driveway onto Gettysburg Pike, 34 feet in width per township specifications. A 'stop' sign shall be provided for the exiting movement.
- Construct secondary access via the extension of Coventry Drive, 34 feet in width per township specifications. A 'stop' sign shall be provided for the Coventry Drive approach to the site driveway.

TABLE 1 LEVELS OF SERVICE [DELAY] SUMMARY SIGNALIZED AND UN-SIGNALIZED INTERSECTIONS

SIGNALIZED AND CN-SIGNALIZED INTERSECTIONS											
					AM PEAK HOUR	STREET					
	Mayra		_	2020		2030					
Intersection	Move	2018		Opening \	⁄ear		Horizon Year				
	ment	Baseline	Base	Projected	Mitigation	Base	Projected	Mitigation			
			No-Build	Build	Build	No-Build	Build	Build			
Fisher Road	ILOS										
- Gettysburg Pike		Α	Α	Α		Α	Α				
Gettysburg i me		[5]	[5]	[5]		[5]	[5]				
UN-SIGNALIZED											
Site Driveway 1	ILOS										
(Full Movement)											
-				Α			Α				
Gettysburg Pike				[1]			[1]				
UN-SIGNALIZED											
OIN-SIGNALIZED											

			PM PEAK HOUR STREET										
	Move			2020			2030						
Intersection	ment	2018		Opening \	'ear		Horizon Y	ear					
	mem	Baseline	Base	Projected	Mitigation	Base	Projected	Mitigation					
			No-Build	Build	Build	No-Build	Build	Build					
Fisher Road	ILOS												
-		Α	Α	Α		Α	Α						
Gettysburg Pike		[3]	[3]	[3]		[3]	[3]						
UN-SIGNALIZED													
Site Driveway 1	ILOS												
(Full Movement)													
-				Α			Α						
Gettysburg Pike				[1]			[1]						
UN-SIGNALIZED													
	11.1.7.1.1						11.1						

Base = No-Build (without proposed development) scenario for design year conditions Projected = Build (with proposed development) scenario for design year conditions ILOS = Overall Intersection Level of Service \Box = Mitigation not required.

TABLE 1a LEVELS OF SERVICE (V/C RATIO) [DELAY] SUMMARY BY MOVEMENT UN-SIGNALIZED INTERSECTIONS

			Į.	AM PEAK H	OUR STREE	Р	M PEAK HO	OUR STREET		
lakana aki an		. /	20		20		202		20	
Intersection		pproach / lovement	Openin	g Year	Horizon Year		Openin	g Year	Horizon Year	
	IV	novement	No-Build	Build	No-Build	Build	No-Build	Build	No-Build	Build
Fisher Road	EB	_	В	В	С	С	В	В	В	В
- Gettysburg Pike		Approach	[14]	[14]	[15.6]	[15.6]	[11.1]	[11.1]	[11.6]	[11.6]
detty soung i me			С	С	С	С	В	В	В	В
UN-SIGNALIZED		EBL	[15.3]	[15.3]	[17.2]	[17.2]	[12.1]	[12.1]	[12.7]	[12.7]
TWSC			(0.32)	(0.32)	(0.39)	(0.38)	(0.13)	(0.13)	(0.16)	(0.15)
			Α	Α	Α	Α	Α	Α	Α	Α
		EBR	[8.7]	[8.7]	[8.7]	[8.8]	[9.3]	[9.3]	[9.4]	[9.4]
	ND		(0.04)	(0.04)	(0.04)	(0.04)	(0.05)	(0.05)	(0.05)	(0.05)
	NB	Approach	[1.2]	[1.2]	[1.2]	[1.2]	[1.7]	[1.7]	[1.7]	[1.7]
			Α	Α	Α	Α	Α	Α	Α	Α
		NBL	[8.4]	[8.4]	[8.4]	[8.5]	[8.7]	[8.7]	[8.7]	[8.7]
			(0.04)	(0.04)	(0.04)	(0.05)	(0.04)	(0.04)	(0.04)	(0.04)
		NBT	Α	А	А	А	Α	А	А	Α
		INBT	[0]	[0]	[0]	[0]	[0]	[0]	[0]	[0]
	SB									
		Approach	[0]	[0]	[0]	[0]	[0]	[0]	[0]	[0]
		SBT	-	-	-	-	-	-	-	-
		SBR	-	-	-	-	-	-	-	-

Values shown as provided on the HCM 2010 Worksheet for un- signalized intersections

⁻ indicates estimated operation with no delay
L.A. = Limited Access scenario

TABLE 1a LEVELS OF SERVICE (V/C RATIO) [DELAY] SUMMARY BY MOVEMENT **UN-SIGNALIZED INTERSECTIONS**

			P	AM PEAK H	OUR STREET	Γ	Р	M PEAK HO	OUR STREET	JR STREET	
		. ,	20	20	20	30	202	20	20	30	
Intersection		pproach /	Openin	g Year	Horizon Year		Openin	g Year	Horizon Year		
	IV	Novement	No-Build	Build	No-Build	Build	No-Build	Build	No-Build	Build	
Site Driveway 1 (Full Movement)	EB	Approach	NA	B [12.1]	NA	B [12.7]	NA	B [12.2]	NA	B [13.5]	
Gettysburg Pike UN-SIGNALIZED		EBL/R	NA	B [12.1] (0.04)	NA	B [12.7] (0.04)	NA	B [12.2] (0.03)	NA	B [13.5] (0.03)	
TWSC	NB Approach		NA	[0]	NA	[0]	NA	[0.1]	NA	[0.1]	
		NBL	NA	A [8.4] (0.01)	NA	A [8.4] (0.01)	NA	A [9.0] (0.01)	NA	A [9.1] (0.01)	
		NBT	NA	A [0]	NA	A [0]	NA	A [0]	NA	A [0]	
	SB	Approach	NA	[0]	NA	[0]	NA	[0]	NA	[0]	
		SBT	NA	-	NA	-	NA	ı	NA	-	
		SBT	NA	-	NA	-	NA	-	NA	-	

Traffic Impact Study

INTRODUCTION

This report provides a traffic impact analysis for proposed residential facilities located in Upper Allen Township, Cumberland County, Pennsylvania. The analysis presented follows standard traffic engineering practice as defined for travel impacts associated with proposed land use developments, and follows the guidelines presented in the Institute of Transportation Engineers (ITE) publication 'Transportation Impact Analyses for Site Development'. General formatting is based on Pennsylvania Department of Transportation's (PennDOT) publication 'Policies and Procedures for Transportation Impact Studies' dated January 28, 2009 and last revised November 25, 2013.

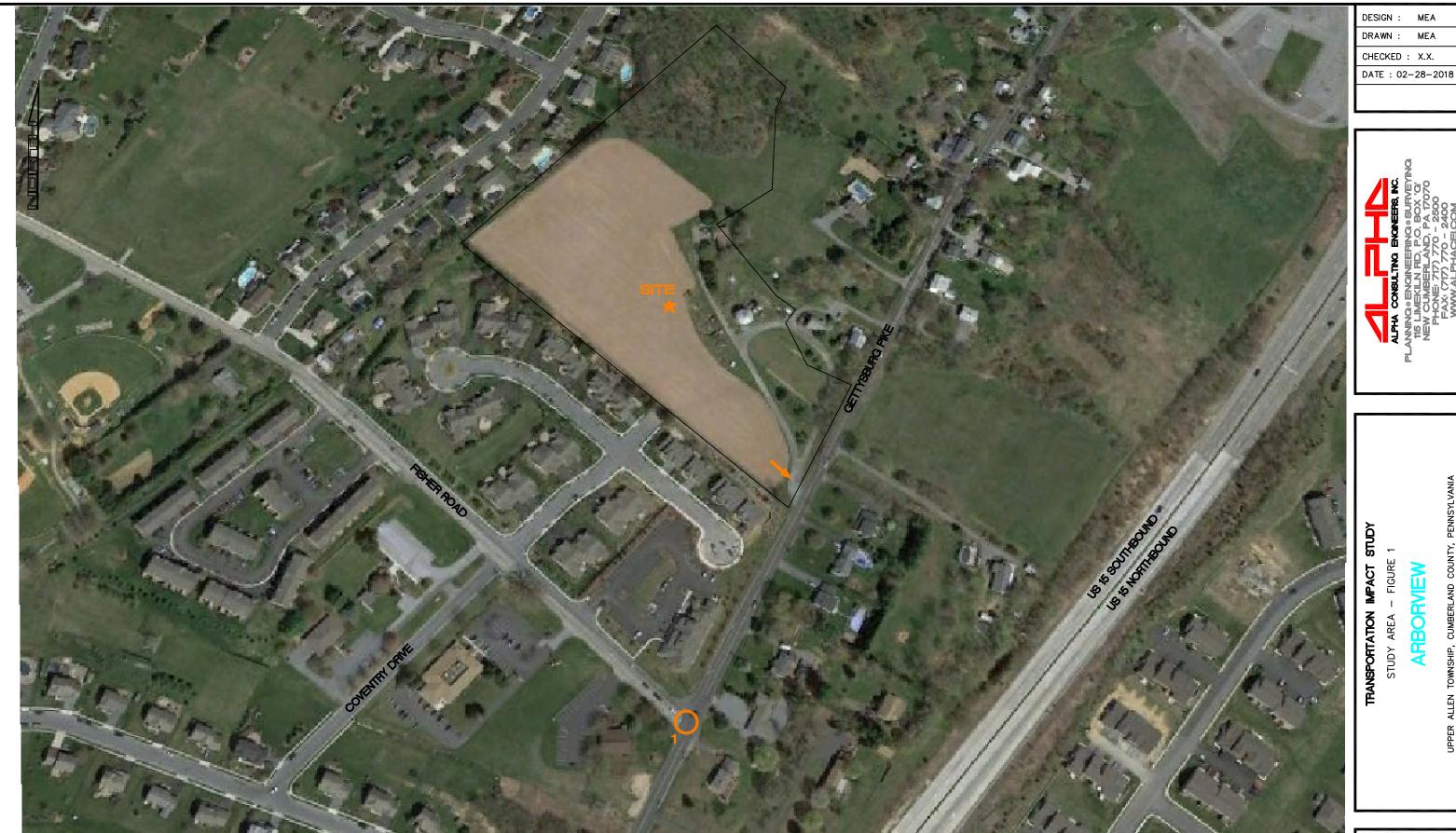
Requirement: Transportation Impact Studies (TIS) also referred to as traffic impact studies or reports are required for land developments by the Township when certain quantitative criteria or thresholds as defined under §220-11.F [SALDO] are met. The proposed land development meets the quantitative criteria under this section of the Township's ordinance. A TIS is therefore required by the Township. Transportation Impact Studies may be required by PennDOT as part of any application for Highway Occupancy Permits (HOP). An HOP as administered by PennDOT under Section 420 of the Act of June 1, 1945 (P.L. 1242, No. 428), known as the "State Highway Law" is required for access to and occupancy of state highways. As part of the noted facility construction, the property owner is not requesting access to any State Route. Therefore, neither a HOP nor TIS will be required by PennDOT for this proposed land development.

<u>Scope:</u> Per discussion with Township representatives, the scope of this report includes an analysis of the following area intersections as shown on **Figure 1**:

- Fisher Road Gettysburg Pike, un-signalized intersection,
- Site Driveway 1 Gettysburg Pike, un-signalized intersection.

Elements of the report were agreed to be the following: Data collection shall be performed during mid-week morning (6:00 to 9:00 AM), and evening (3:00 to 7:00 PM) hours while public school is in session; Turn movement data shall be collected at the adjacent intersection; No turn movement data is collected at the site driveways as the site driveways do not exist; Trip generation shall be based on data available within the manual, *Trip Generation*, Tenth Edition, 2017, an Institute of Transportation Engineers (ITE) Informational Report; Distribution and assignment of trips are to be based on existing data collected at the adjoining intersections (i.e. directional percentage); The opening year shall be 2020 and the horizon year shall be 2030; Growth rates shall 0.87% based on current published data from PennDOT; queue analysis shall be included for the Fisher Road intersection with Gettysburg Pike and any other study intersection that will require mitigation;

<u>Location:</u> The subject site is a 15-acre tract of land located along the west side of Gettysburg Pike approximately 550 feet north of Fisher Road in Upper Allen Township, Cumberland County, Pennsylvania as shown on **Figure 1a.** The site is currently undeveloped as shown on **Figure 1b**. The analysis herein only applies to the facility as shown on **Figure 2.**



TRANSPORTATION IMPACT STUDY AREA - FIGURE

ARBORVIEW

DESIGN : MEA

CHECKED : X.X.

PROJECT NO. 317565

SURVEY BOOK :
Z:\Surveyor\Year\Project.txt

SCALE : 1"=200'

SITE DRIVEWAYS

PROPOSED STUDY INTERSECTIONS:

1 FISHER ROAD - GETTYSBURG PIKE



DESIGN : MEA DRAWN: MEA

CHECKED : X.X.

DATE: 02-28-2018

Transportation impact study Aerial — Figure 10

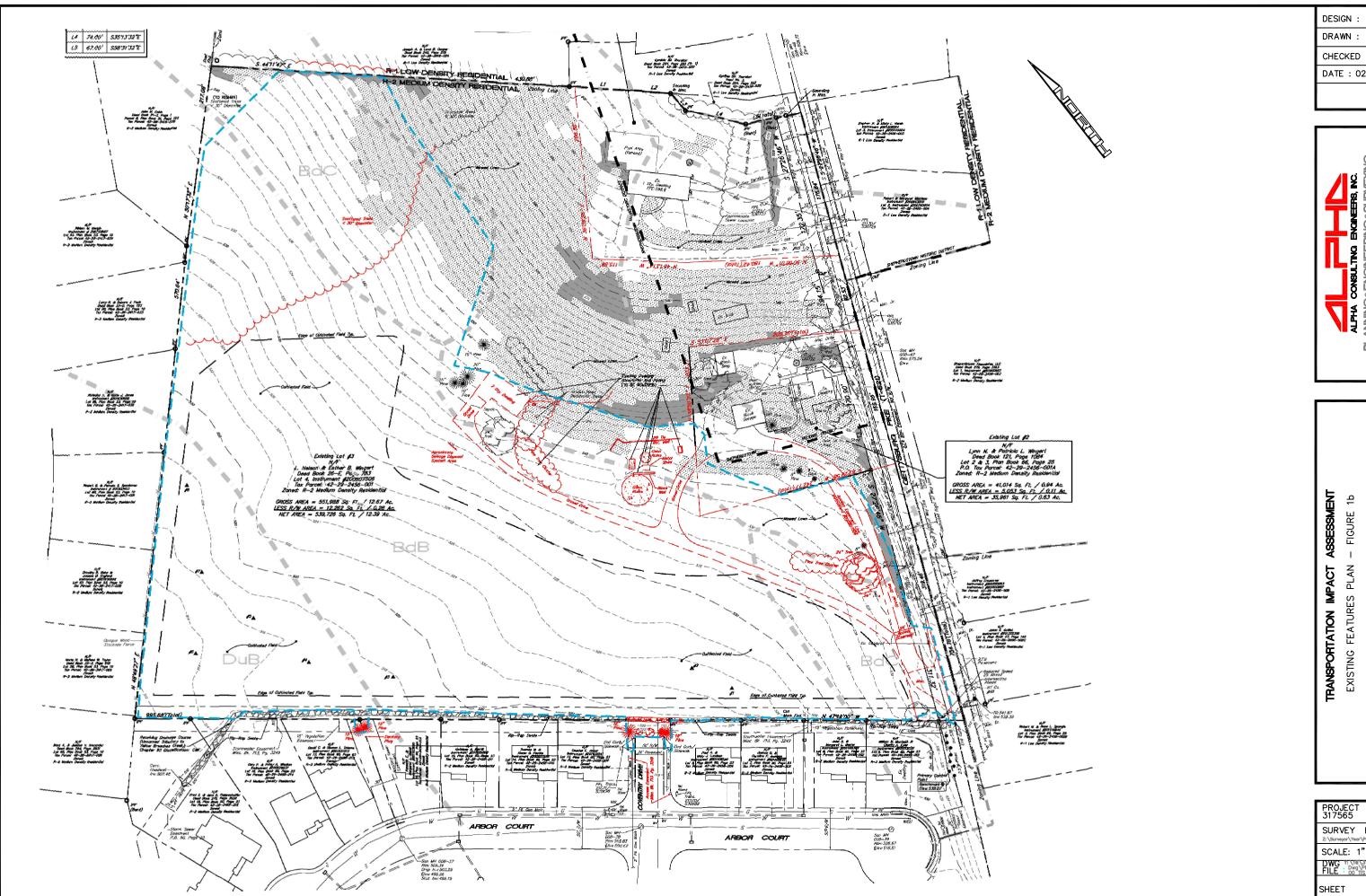
ARBORVIEW

PROJECT NO. 317565

SURVEY BOOK :
Z:\Surveyor\Year\Project.txt

SCALE : 1" = 100'

SHEET



DESIGN: MEA

CHECKED : X.X.

DATE: 02-28-2018

MEA

FIGURE 1b ARBORVIE EXISTING FEATURES PLAN

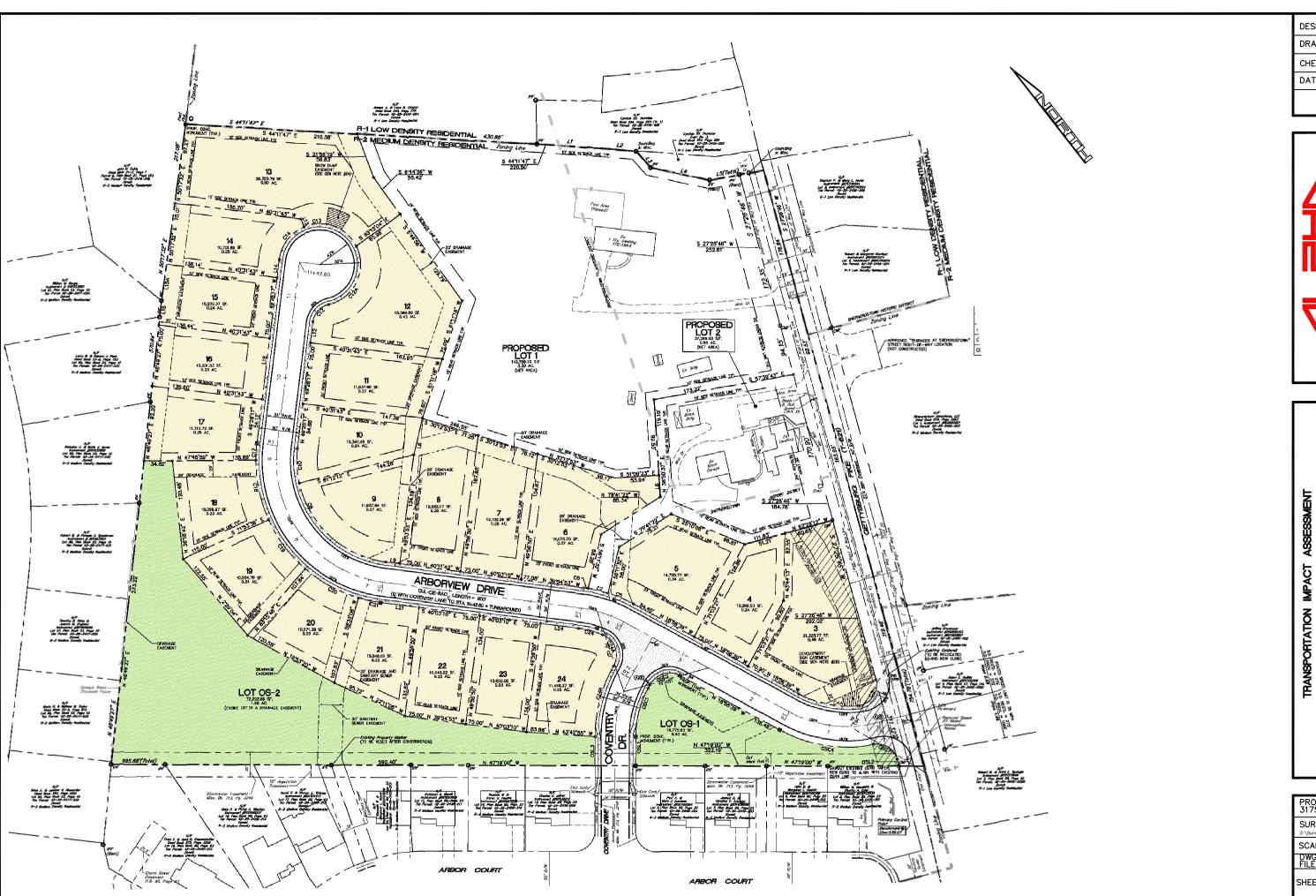
PROJECT NO. 317565

SURVEY BOOK:

SCALE: 1" = 50'

9

SHEET



DESIGN : MEA

DRAWN : MEA

CHECKED : X.X.

DATE: 02-28-2018

ALPHA CONSULTING ENGINEERS, INC.
PLANNING® ENGINEERING® SURVEYING
115 LIMEKILIN PD, P.O. BOX 'G'
NEW CUMBERLAND, PA 17070
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SITE PLAN – FIGURE 2

ARBORVIEW

PROJECT NO. 317565

SURVEY BOOK : Z:\Surveyor\Year\Project.txt SCALE: 1" = 50'

DWG 1: \16\316366.com \316 Dwg\Plans\HOP\TIS\ FILE 00 TIS LOC.dwg

SHEET

10

LAND USE CONTEXT

"Pennsylvania and New Jersey Departments of Transportation have partnered in the development of the 'Smart Transportation Guide Book' (March 2008) to guide the development of non-limited access roads as context sensitive." To achieve the objectives of the Guide Book, land use context must be determined in order to provide appropriate roadway design. Land use context for the proposed development and the immediate surrounding area is predominately 'Suburban Corridor'. The area is characterized predominantly by a mix of commercial uses with single family residential homes lying further to the east and west along the Gettysburg Pike corridor. This context coincides with Upper Allen Township's current zoning of the site being 'Highway Commercial'. The land use context may be referred to throughout this report in the comparison and selection of appropriate design criteria.

EXISTING ROADWAY NETWORK

The existing roadway network affected by the proposed development as agreed upon with the Upper Allen Township consists of the Gettysburg Pike corridor immediately adjacent to the site and the previously noted study intersections. The Gettysburg Pike corridor falls within PennDOT's designated urbanized area boundary. Existing lane configurations and intersection controls are illustrated in **Figure 3.** Photographs of the intersection and approaches are provide in the appendix / tabbed section of the study.

Corridors

Gettysburg Pike

Gettysburg Pike is classified as an 'Urban Collector', and falls under Traffic Pattern Group 5 (TPG-5) as designated by PennDOT. Upper Allen Township has classified Gettysburg Pike as a 'Community Arterial' north of the intersection with South Market Street and as a "Community Collector' for sections of the roadway located south of the intersection with South Market Street. Traffic flows in a north/south direction for the section of the roadway adjacent to the site with an Annual Average Daily Traffic approaching 4,600 vehicles. The speed limit is posted at 35mph for sections of the road located north of and south of the intersection with Fisher Road. The noted speed limit is within the range recommended for the land use context. The alignment approaching the site from the north is straight having grades that vary from approximately 5 to 1 percent, providing greater than minimum sight distances for turning movements. The alignment approaching the site from the south is slightly curvilinear having grades that vary from 10 to 1 percent, also providing greater than minimum sight distances for turning movements. The wearing surface is bituminous and is in good shape. Lane widths average 10 to 12 feet over the length of the roadway. Shoulders are essentially nonexistent. Uses along the adjacent Gettysburg Pike corridor consist of primarily residential uses with some commercial, service, and agricultural uses.

Intersections

Fisher Road – Gettysburg Pike, un-signalized intersection:

This is a stop controlled 3-leg intersection with the eastbound (Fisher Road) approach controlled. The eastbound approach consist of two exclusive turn lanes approximately 12 feet in width. The northbound approach consists of a single lane approximately 11

feet in width. The southbound approach consists of an exclusive right turn lane along with a separate through lane both being approximately 10 feet in width. Speed limits are posted as 25 MPH for fisher Road and 35 MPH for the Gettysburg Pike approaches. Curb is provided along the northern side of the eastbound approach and along the west side of the southbound approach. Sidewalks are not located at the intersection. Sidewalks are located along the northern side of Fisher Road beginning approximately 150 feet west of the intersection. Intersection capacity currently operates at a LOS A for all peak hours.

- Multimodal Transportation

Capital Area Transit (CAT) does not currently operate any transit routes along Gettysburg Pike in front of the proposed development site. The nearest transit route is (bus route 120) the Winding Hill Express. This route connects the Winding Hills Road Park-n-ride to the Capitol Complex in Harrisburg. This route also has direct connection to the Harrisburg Transit Center which houses the Amtrak Station, Capitol Trailways and Greyhound Bus terminals. Connecting routes provide access to Harrisburg International Airport. For bicyclist, bike racks are provided on CAT's busses and bike racks are provided at some of the Park-n-rides. Nearest Park-n-ride site is located at the intersection of East Winding Hill Road and Orchard Boulevard (1 mile from site). Connecting routes, Park-n-ride sites, and time tables for route 120 are included within the 'Existing Conditions' tabbed section of the appendix.

Rabbittransit operates a route between Gettysburg and Harrisburg along the adjacent US 15 corridor. The only direct connection is located at the Harrisburg Transit Center.

EXISTING TRAFFIC VOLUMES AND ANALYSIS

Manual traffic counts were conducted on February 28, 2018 during the weekday (6:00 to 9:00 AM) morning and (3:00 to 7:00 PM) evening periods to obtain peak hour data. Data was collected using 'Jamar Technologies, Inc' model TDC-12 hand held recorders. Peak hours and volumes for the individual intersections are illustrated in **Table 2**. Turn movement vehicle volume data is included in the appendix. Existing condition traffic volumes for the weekday AM, and weekday PM peak hours are illustrated and included in the appendix as part of **Figure 3**. **Table 1** as included within the executive summary details the average LOS and control delay for each intersection. Each LOS is illustrated and included in the appendix as part of **Figure 3**.

TABLE 2
Peak Hour and Volume

		Peak	Hour	
Intersection	AM (Volume)	PM (Volume)		
South Market Street (SR0014)- Gettysburg Pike	7:15 – 8:15 (582)	5:00 – 6:00 (603)		

SEASONAL ADJUSTMENT AND GROWTH FACTORS

PennDOT publishes forward-looking growth projections for a one-year period in a one-page document entitled "Growth Factors for August 2017 to July 2018". For purposes of this analysis, the published value is 0.87% for urban non-interstate highways in Cumberland County. While the land use context is 'Suburban' the study area falls within PennDOT's urban boundary. This factor was applied to arrive at the 2020 base volumes for the design opening year. Traffic volume worksheets are included in a separate tabbed section of the appendix detailing future volumes anticipated per movement, per intersection.

NO-BUILD FUTURE TRAFFIC VOLUMES

Baseline year is 2018 to coincide with the previously noted data collection. Opening year is assumed to be 2020 based on the anticipated development schedule. Opening year - base condition (no-build) traffic volumes for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 5a.** Opening year - base condition (no-build) LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 5e. Table 1** details the LOS for each intersection within the study area.

Upper Allen Township's ordinance requires the design horizon year to be 10 years beyond the opening year or 2030. Design horizon year - base condition (no-build) traffic volumes for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 6a**. Design horizon year- base condition (no-build) LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 6e**. **Table 1** details the LOS for each intersection within the study area.

PROJECT DESCRIPTION

1849 Development, LLC is proposing to construct new residential units on approximately 15 acres of land located along Gettysburg Pike in Upper Allen Township. The site is undeveloped, currently used for agricultural purposes as shown on **Figures 1a and 1b**. Proposed facilities will include 22 residential lots, approximately 1,330 linear feet of new public streets, associated driveways, stormwater facilities, lawns, etc. A conceptual sketch plan is attached as **Figure 2**. The public streets will include a paved section 34 feet in width, concrete sidewalks 4 feet in width, all within a 50 feet wide right-of-way. Sidewalks will connect to the sidewalk system along Coventry Drive. The proposed development is consistent with the zoning. Construction is anticipated to start in 2018 and be completed in the same year to achieve a use prior to 2020. The streets are intended to be dedicated to the municipality.

PROPOSE SITE ACCESS

Vehicular access to the facility is proposed via a full movement site entrance along Gettysburg Pike near the eastern limits of the property. This access point will be located approximately 520 feet from the intersection with Fisher Road. Secondary access is proposed via the extension of Coventry Drive. Site driveways are classified as low-volume driveways. Proposed access is shown on **Figure 2.**

TRIP GENERATION

The trip generation equations for the proposed development were obtained from the manual, *Trip Generation*, Tenth Edition, 2017, an Institute of Transportation Engineers (ITE) Informational Report. For this analysis, Land Use Code 210 (Single Family Detached Housing), was used to calculate the average number of vehicular trips the development is estimated to generate during the weekday, weekday AM peak, weekday PM peak, and weekday generator peak periods. Peak hour trips calculated are representative of volume that occurs only during the peak hour of the generator and or adjacent street traffic. **Table 3a** shows the equations and directional percentages for the analyzed time periods. **Table 3b** list the estimated trips generated by the proposed development at full build out. Trip generation data sheets are included in a separate tabbed section of the appendix.

TABLE 3a
ITE TRIP GENERATION EQUATIONS

Land Use Description	ITE #	Time Period	Equations	Independent Variable (X)	Entering %	Exiting %
	Weekday $LN(T) = 0.92LN(X)+2.71$		50%	50%		
		AM Peak Hour of Adj Street	T = 0.71(X) + 4.80		25%	75%
Single Family Detached Housing	210	PM Peak Hour of Adj Street	LN(T) = 0.96LN(X) + 0.20	(22) Units	63%	37%
		AM Peak Hour of Generator	LN(T) = 0.91LN(X) + 0.20		26%	74%
		PM Peak Hour of Generator	LN(T) = 0.94LN(X) + 0.34		64%	36%

T = number of site-generated vehicular trips AR = Trip Generation Rate, No equation provided. M= Measured Trip Rate SNA = Split Not Available

TABLE 3b
TRIP GENERATION
PROPOSED DEVELOPMENT – FULL BUILD OUT

Time Period	New Trips										
	Total										
	Total	Total Enter Exit									
Weekday	258	129	129								
Weekday AM Adj.	20	5	15								
Weekday PM Adj.	24	15	9								
Weekday AM Gen.	20	5	15								
Weekday PM Gen.	26	17	9								

The proposed development is expected to generate approximately 258 vehicle trips on an average weekday while school is in session. The trip generation estimate includes approximately 20 vehicle trips during the morning or AM peak hour of the adjacent street and approximately 24 vehicle trips during the PM peak hour of the adjacent street.

TRIP DISTRIBUTION

The distribution and assignment of site-generated trips was based upon an analysis of the following: (1) existing traffic patterns and distributions within the study area; (2) the available routes for travel; and (3) the proposed site driveway location and configuration.

Additional trips were added to the distribution for conservative modeling of northbound trips from and southbound trips to the adjacent 'Arborfield' development. Travel patterns and distributions of site-specific traffic are illustrated in the appendix as part of **Figure 4**. The resulting assignment is shown in **Tables 4a and 4b**.

TABLE 4a
TRIP ASSIGNMENT (% of development generated vehicles at intersection)

Time Period			eway 1 - urg Pike		Fisher Road – Gettysburg Pike							
	Enter Exit				Enter Exit							
	NBL	SBR	EBL	EBR	NBL	NBT	SBR	EBL	EBR	SBT		
AM	38%	62%	81%	19%	0%	38%	0%	0%	0%	19%		
PM	21%	79%	66%	34%	0%	21%	0%	0%	0%	34%		

TABLE 4a
TRIP ASSIGNMENT (% of development generated vehicles at intersection)

Tiı	Time Period Site D Getty							Fisher Road – Gettysburg Pike				
		Enter				Exit		En	ter	E	xit	
		NBL	SBT	SBR	EBL	EBR	NBT	NBT	SBR	EBL	SBT	
	ARBORVIEW	2	0	3	12	3	0	2	0	0	3	
AM	ARBORFIELD	0	-1	1	5	0	-5	0	-1	-5	0	
	TOTAL	2	-1	17	17	3	-5	0	-1	-5	0	
	ARBORVIEW	3	0	12	6	3	0	3	0	0	3	
PM	ARBORFIELD	0	-6	6	3	0	-3	0	-6	-3	0	
	TOTAL	3	-6	18	9	3	-3	0	-6	-3	3	

BUILD FUTURE TRAFFIC VOLUMES (OPENING YEAR)

The site-generated trips for the proposed development were added to the 2020 opening year - base condition (no-build) to calculate 2020 opening year - projected (full build out) conditions. Projected condition traffic volumes for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 5c.** Opening year - projected condition (build) LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 5g. Table 1** details the LOS for each Intersection within the study area.

BUILD FUTURE TRAFFIC VOLUMES (DESIGN HORIZON YEAR)

The site-generated trips for the proposed development were added to the 2030 horizon year - base condition (no-build) to calculate 2030 horizon year - projected (full build out) conditions. Projected condition traffic volumes for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 6c.** Horizon year - projected condition (build) LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 6g. Table 1** details the LOS for each intersection within the study area.

CAPACITY ANALYSIS

Level of Service (LOS) generally describes operational characteristics in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort and convenience and safety. Six Levels of Service are defined for each type of traffic facility, ranging from A to F. Level of Service "A" indicates free flow; Level of Service "B" indicates stable flow; Level of Service "C" indicates stable, but inhibited flow; Level of Service "D" indicates high density, restricted stable flow; Level of Service "E" indicates operation at or near capacity; Level of Service "F" is indicative of flow breakdown. Levels of Service criteria are also quantified in terms of average control delay as illustrated in **Table 5** per vehicle for a one-hour period. PennDOT policy sets acceptable LOS for intersections as overall intersection LOS C in rural areas and overall intersection LOS D in urban areas. Individual municipalities may have defined differing values for acceptable LOS by ordinance.

TABLE 5
Control Delay per Levels Of Service

Control Delay per Levels Of Service										
	Control Delay Per	Vehicle (Seconds)								
Level-of-Service	Signalized Intersections	Un-Signalized Intersections								
А	≤ 10	≤ 10								
В	> 10 and <u><</u> 20	> 10 and <u><</u> 15								
С	> 20 and <u><</u> 35	> 15 and <u><</u> 25								
D	> 35 and <u><</u> 55	> 25 and <u><</u> 35								
Е	> 55 and <u><</u> 80	> 35 and <u><</u> 50								
F	> 80	> 50								

Signalized and un-signalized intersection capacity analysis was conducted utilizing SYNCRO 8 Software. HCM data sheets are included in a separately tabbed section of the appendix. Capacity analysis is conducted per methodologies and procedures outlined in the Transportation Research Board publication HCM 2010.

As previously stated above opening year and design horizon year- projected conditions (build) LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figures 5g and 6g**, respectively. For comparison, existing LOS for the weekday AM and PM peak hours are illustrated and included in the appendix as part of **Figure 3**. Levels of Service (LOS) for intersections within the study area have been summarized in **Table 1**. The summaries have been prepared outlining existing 2018 baseline conditions, opening year 2020 base (no-build) and projected (build) conditions, and horizon year 2030 base (no-build) and projected (build) conditions. 'Baseline' refers to the existing development scenario represented by the measured traffic volumes listed in the *Existing traffic volumes and analysis* section of this report. 'No-Build' refers to a development scenario whereby traffic growth on the adjacent street is the only additional development. 'Build' refers to a development scenario that consists of the addition of the residential development and related driveway construction. During the future 2020 and 2030 design years the following two study intersections are estimated to operate at varying levels of service dependent upon a specific peak hour.

- **Fisher Road & Gettysburg Pike** During both the AM and PM peak hour this intersection currently operates at an acceptable LOS 'A'. Average intersection delay is estimated to increase negligibly over the 2-year design period without the development. With the addition of the development average intersection delay is estimated to increase by less than 1 second.
- Site Driveway 1 & Gettysburg Pike During the both the AM and PM peak hours
 this intersection is estimated to operate at LOS 'A' under the opening 2020 design
 year with the development. Average intersection delay is estimated to be negligible
 being 1 second or less with the development. All movements are estimated to
 operate at LOS 'C' or better for all build scenarios.

TURN LANE WARRANT ANALYSIS

Volumes of right turning traffic into the site are estimated to be below the minimum thresholds required for warranting a right turn lane. Volumes of left turning traffic into the site are estimated at less than 2% of the advancing volume. The advancing traffic volumes are estimated to be below the minimum thresholds required for warranting a left turn lane. Turn lane warrant analysis worksheets for the 2020 build scenario are included in a separately tabbed section of the appendix.

TURN RESTRICTION WARRANT ANALYSIS

Turn restriction warrants were evaluated per 67 PA Code § 212.111 for the proposed site driveway intersection. None of the six warrants were met for the build development scenarios.

QUEUE ANALYSIS

Queue lengths were calculated utilizing SYNCRO 8 Software. Calculated 95th% queue lengths for each movement at each intersection are indicated in **Table 6a** for the peak hours. Queuing analysis indicates that all design scenario queue lengths either fall within the available storage lengths or do not extend no-build scenario queue lengths by a car length (20 feet). Queue lengths at the southbound approach to the intersection with Fisher Road are estimated to continue to be less than the distance to the proposed full movement driveway.

TABLE 6a CACULATED 95TH % QUEUE LENGTHS

CHOCKITED SO SUCCESSION SOLUTION													
				AM	Peak H	our		PM	l Peak H	Hour			
Intersection	Move	Storage	2017	20	2020		30	2018	20	20	20	30	
intersection	ment	Length	No-	No-	Build	No-	Build	No-	No-	Build	No-	Duild	
			Build	Build	Bullu	Build	Bullu	Build	Build	Bullu	Build	Build	
Fisher Road -	EBL	210	28	28	28	36	36	10	10	10	12	10	
Gettysburg Pike	EBR	470	2	2	2	2	2	2	2	2	4	4	
	NBL/T	500+	2	2	2	4	4	2	2	2	2	2	
	SBT	*500	0	0	0	0	0	0	0	0	0	0	
	SBR	170	0	0	0	0	0	0	0	0	0	0	
Site Driveway 1	EBL/R	100+	NA	NA	2	NA	2	NA	NA	2	NA	2	
-	NBL/T	500	NA	NA	0	NA	0	NA	NA	0	NA	0	
Gettysburg Pike	SBT/R	500+	NA	NA	0	NA	0	NA	NA	0	NA	0	

Lengths are in feet.

= Length greater than storage length.

* Distance to SD1

SIGHT DISTANCE ANALYSIS

A sight distance analysis was performed for the site driveway intersections. In general, recommended safe sight distances depend upon the posted speed limit, roadway grades, and the number of travel lanes. The measured existing sight distances were compared to PennDOT's safe stopping sight distance (SSSD) standard as calculated by the following equation:

SSSD =
$$1.47VT + V^2/[30(f\pm g)]$$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Velocity of Vehicle (posted)

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements (average of 0.30)

g = Percent of Roadway Grade Divided by 100

PennDOT's safe stopping sight distance standards both exceed the stopping sight distance requirements as specified in A Policy on Geometric Design of Highways and Streets, of the American Association of State Highway and Transportation Officials (AASHTO), Chapter III, "Elements of Design," 2004. The existing sight distances at the site driveways were measured and compared to the minimum sight distance standards as specified in Title 67 of the PA Code, Chapter 441, "Access to and Occupancy of Highways by Driveways and Local Roads," August, 1996. **Table 7** shows the measured and calculated sight distances at the site driveways for vehicles entering and exiting the site.

TABLE 7 SIGHT DISTANCE ANALYSIS FOR GETTYSBURG PIKE – SITE DRIVEWAY 1 UN-SIGNALIZED INTERSECTION

	Direction	Speed	Grade	Sight Distances (feet)		
		(mph)	(%)	Calculated MIN	Measured	Desirable
Exiting Right	To the left	35	-5	269	370	NA
Turns	10 ine tejt		3	20)	2.0	111
Exiting Left Turns	To the right	35	+5	233	1,069	NA
Entering Left Turns	From Behind	35	+5	233	1,049	NA
Entering Left turns	Opposing	35	-5	269	943	NA

RECOMMENDED IMPROVEMENTS

Offsite improvements are not recommended as the additional traffic generated by the proposed development will not impact the study intersections at levels that would normally require mitigation.

Site access is recommended to be constructed as follows:

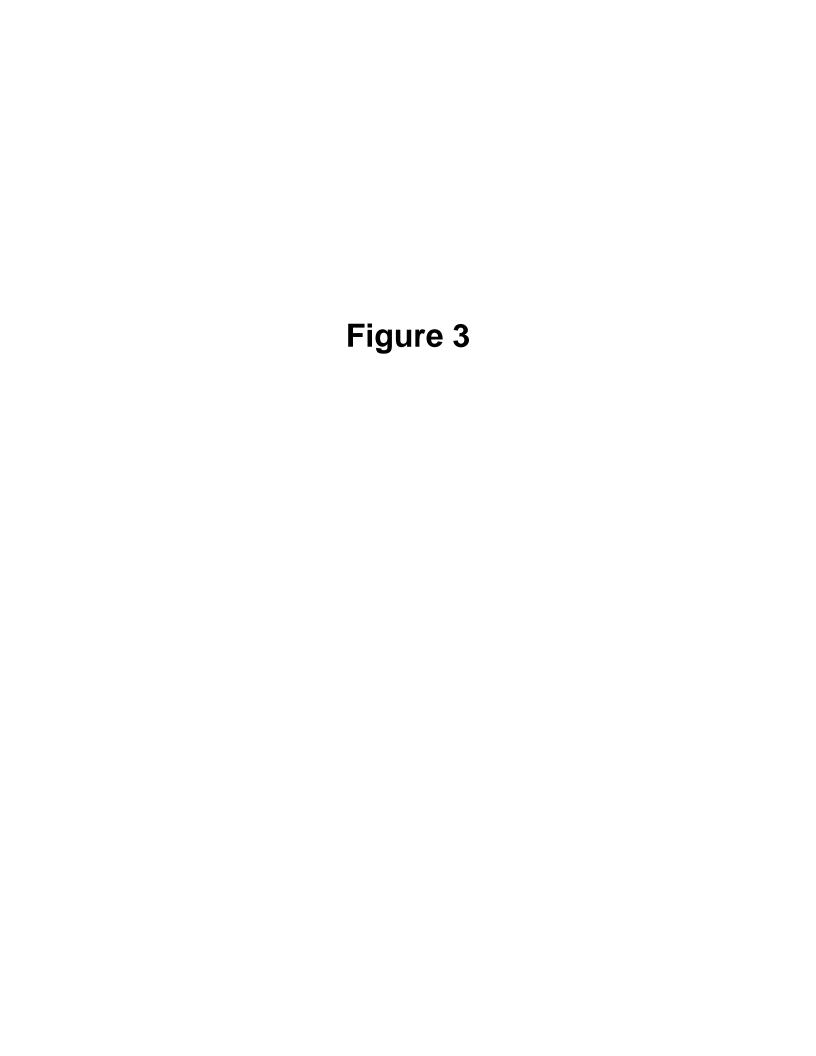
- Construct full movement driveway onto Gettysburg Pike, 34 feet in width per township specifications. A 'stop' sign shall be provided for the exiting movement.
- Construct secondary access via the extension of Coventry Drive, 34 feet in width per township specifications. A 'stop' sign shall be provided for the Coventry Drive approach to the site driveway.

Appendices

Included **☑** Figure 3: Existing Volume/LOS ☐ Figure 3a: Existing Signal Plan (if applicable) **☑** Figure 4: Trip Distribution Percentage and Volumes **Opening Year Conditions:** ▼ Figure 5a: Opening Year Traffic Volumes without Development (AM, PM, Site Peak) ☐ Figure 5b: Opening Year Traffic Volume without Development & with Committed Development Figure 5c: Opening Year Traffic Volumes with Development ☐ Figure 5d: Opening Year Traffic Volumes with Development & Committed Development ▼ Figure 5e: Opening Year Levels of Service without Development ☐ Figure 5f: Opening Year Levels of Service without Development & with Committed Development ▼ Figure 5g: Opening Year Levels of Service with Development ☐ Figure 5h: Opening Year Levels of Service with Development & Committed Development ☐ Figure 5i: Opening Year Levels of Service with Development & Recommended Mitigation ☐ Figure 5j: Opening Year Levels of Service with Development, Committed Development, & Recommended Mitigation Design Horizon Year Conditions: Design Horizon Year Traffic Volumes without Development (AM, PM, Site Peak) ☐ Figure 6b: Design Horizon Year Traffic Volumes without Development & with Committed Development Design Horizon Year Traffic Volumes with Development Design Horizon Year Traffic Volumes with Development & Committed Development ☐ Figure 6d: Design Horizon Year Levels of Service without Development ☐ Figure 6f: Design Horizon Year Levels of Service without Development & with Committed Development Design Horizon Year Levels of Service with Development ☐ Figure 6h: Design Horizon Year Levels of Service with Development & Committed Development ☐ Figure 6i: Design Horizon Year Levels of Service with Development & Recommended Mitigation Design Horizon Year Levels of Service with Development, Committed Development, & ☐ Figure 6j: Recommended Mitigation Misc. Site Photographs Existing Conditions (sketches, Transit Data, etc.) ☑ Turning Movement Counts, 24 Hour Volumes Growth Rate and Volume Worksheets ▼ Trip Generation Worksheets ☐ Gap Analysis □ Delay Analysis □ Traffic Signal Warrant Analysis ▼ Turn Lane Analysis

Correspondence

Figures





SITE

GETTYSBURG PIKE

FISHER ROAD

= Channelized Island

= Signal-Controlled Approach

STOP = Stop-Controlled Approach

= AM Peak Hour Data
[#] = PM Peak Hour Data

SD : Site Driveway ILOS : Intersection Level of Service Schematic Drawing : Not To Scale ARBORVIEW



FIGURE 3

SHEET 1 OF 3

EXISTING LANE CONFIGURATION AND INTERSECTION CONTROL



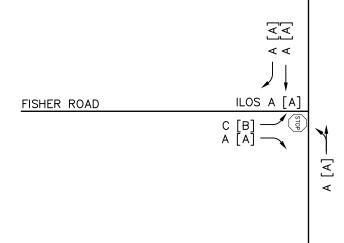
Schematic Drawing: Not To Scale

GETTYSBURG PIKE SITE FISHER ROAD 136 [75] 31 [38] = Channelized Island = Signal-Controlled Approach **ARBORVIEW** = Stop-Controlled Approach # = AM Peak Hour Data [#] = PM Peak Hour Data ALPHA CONSULTING ENGINEERS, INC. SD : Site Driveway FIGURE 3 ILOS: Intersection Level of Service 2018 BASELINE CONDITIONS PEAK HOUR TRAFFIC VOLUMES



SITE

GETTYSBURG PIKE



= Channelized Island

= Signal-Controlled Approach

STOP = Stop-Controlled Approach

= AM Peak Hour Data
[#] = PM Peak Hour Data

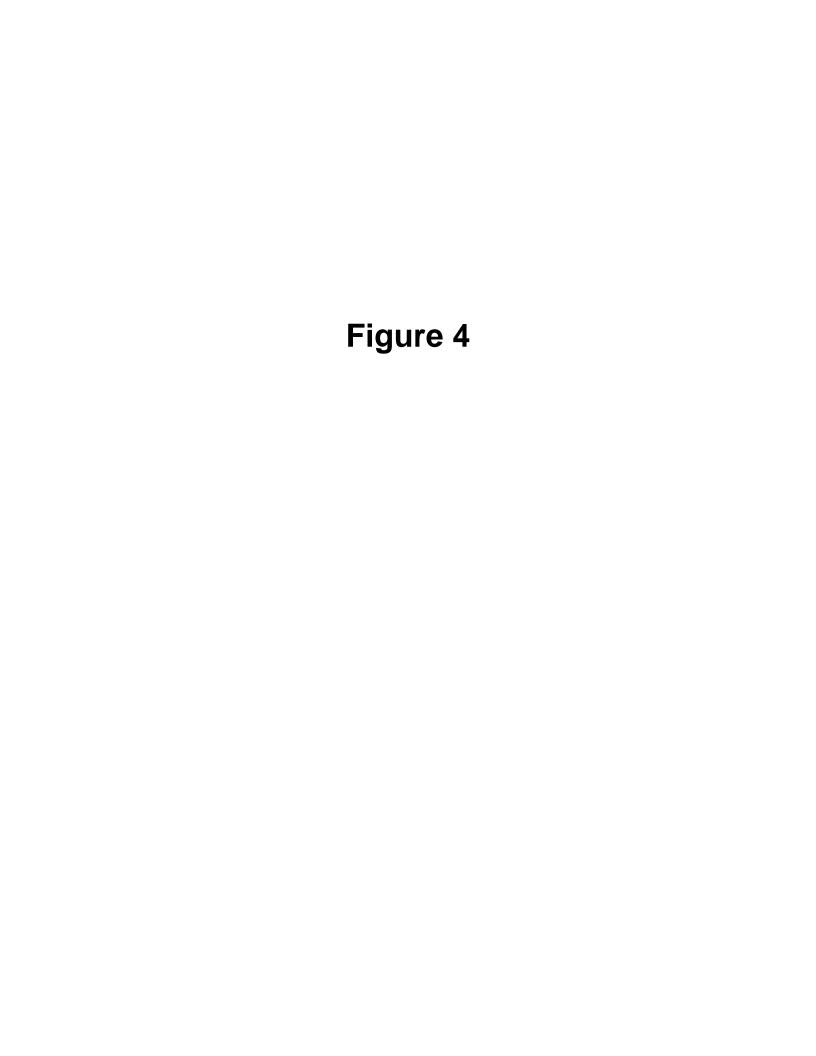
SD : Site Driveway ILOS : Intersection Level of Service Schematic Drawing : Not To Scale ARBORVIEW



FIGURE 3

SHEET 3 OF 3

2018 BASELINE CONDITIONS PEAK HOUR LOS





SITE

FISHER ROAD

19% [34%] -

81% [66%]

GETTYSBURG PIKE

= Channelized Island

= Signal-Controlled Approach

= Stop-Controlled Approach

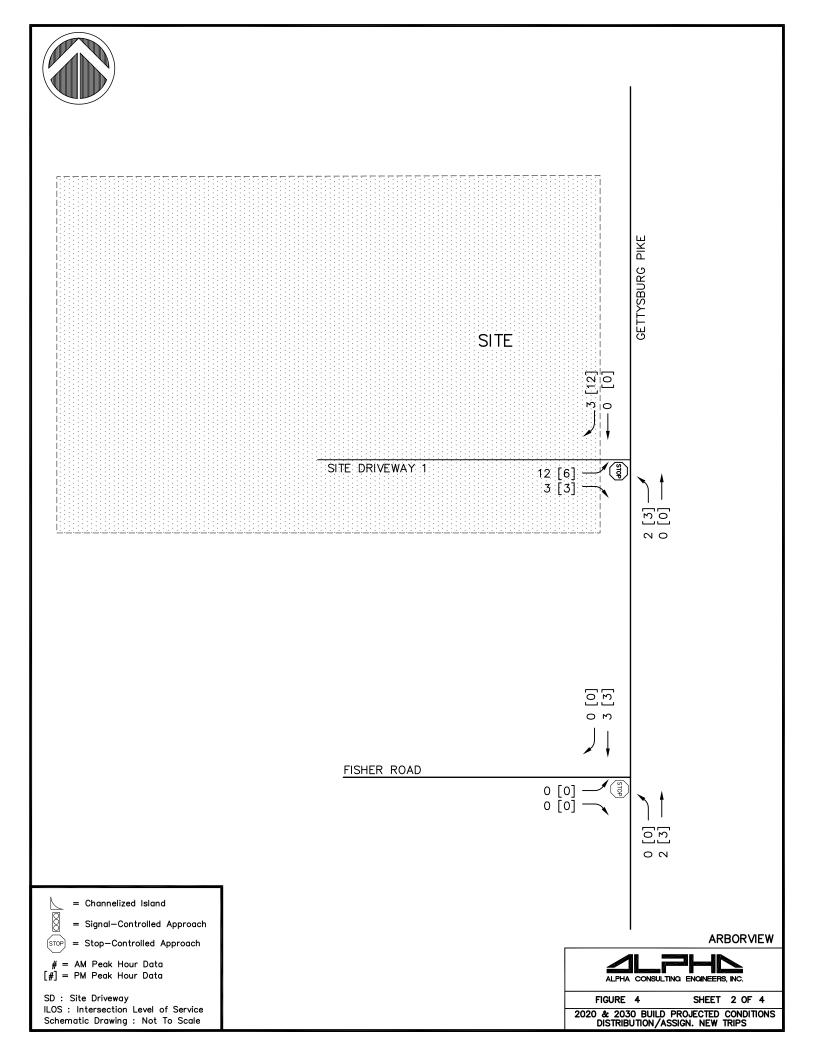
= AM Peak Hour Data [#] = PM Peak Hour Data

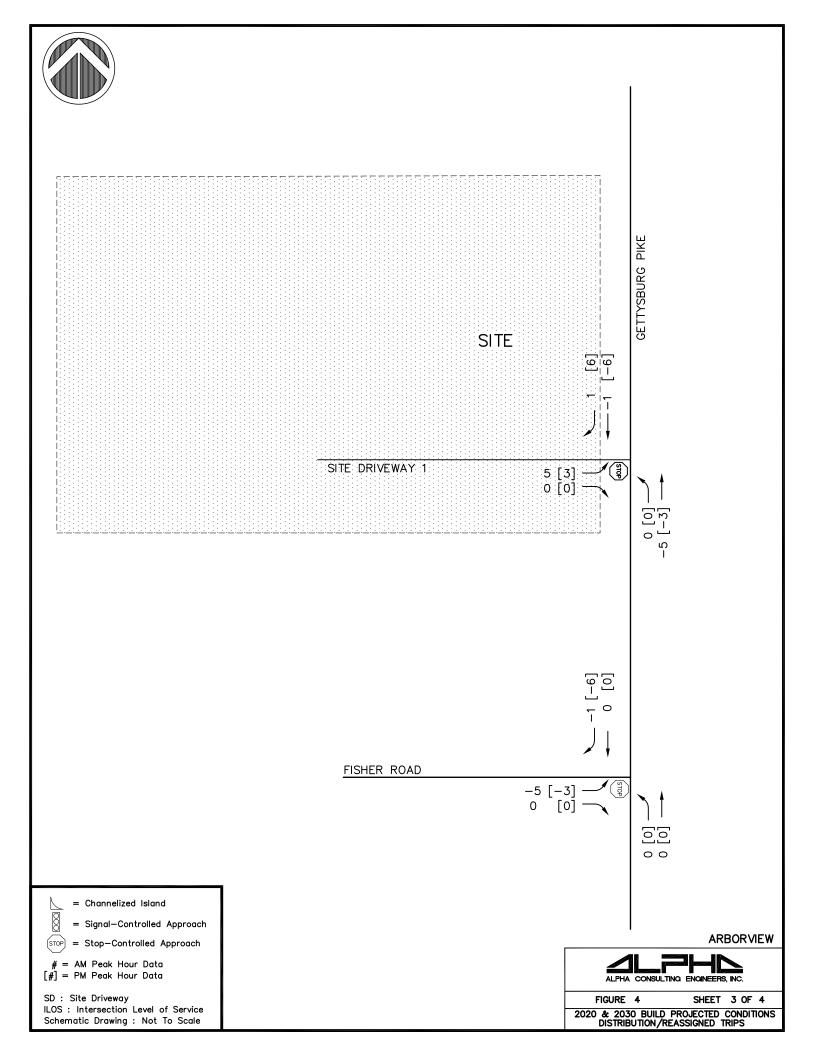
SD : Site Driveway ILOS: Intersection Level of Service Schematic Drawing: Not To Scale

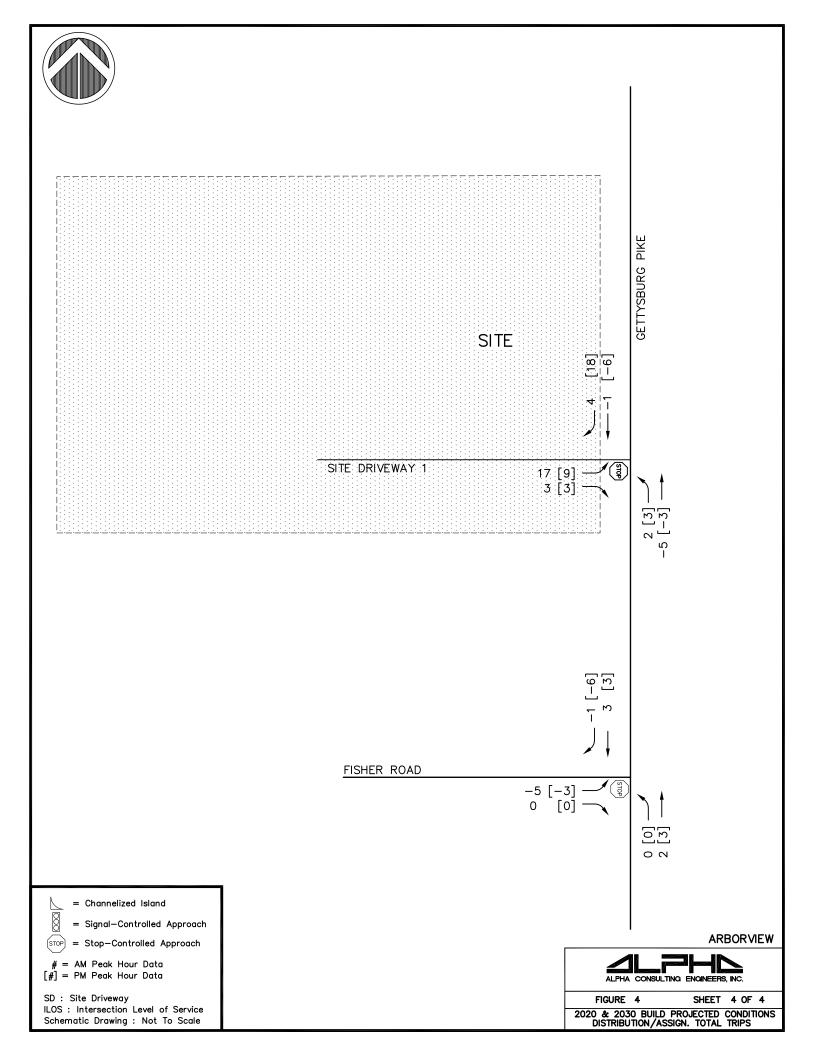
ARBORVIEW

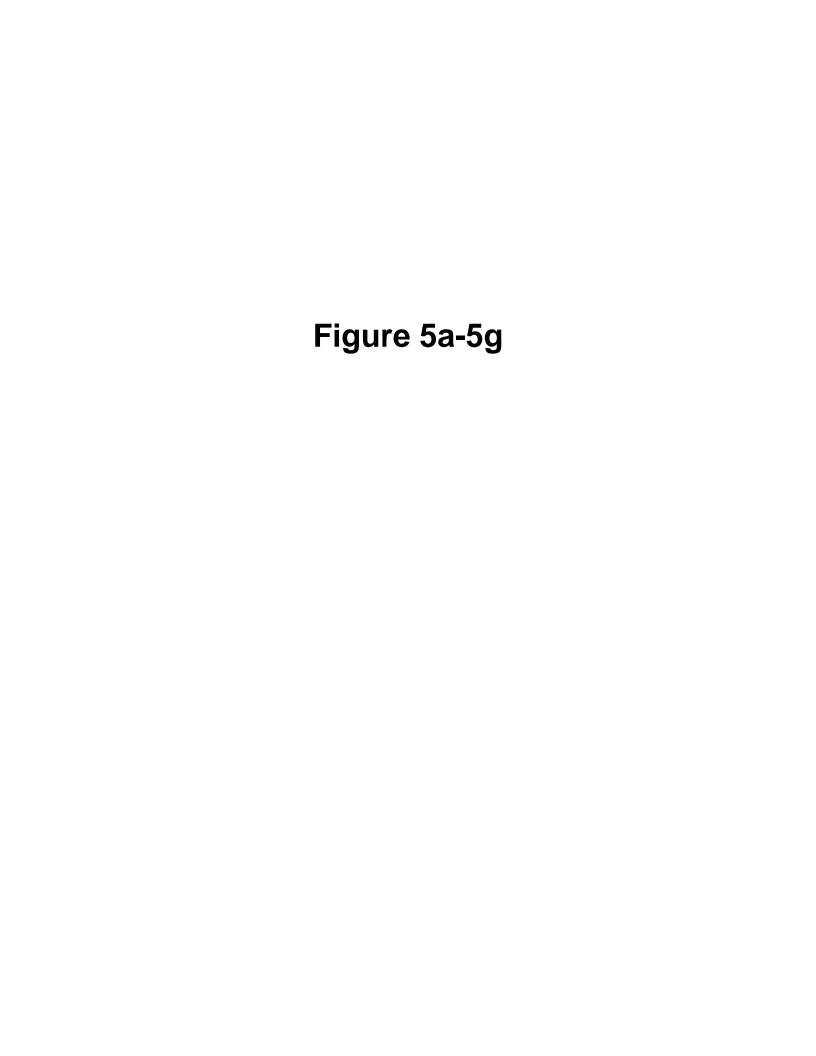


SHEET 1 OF 4 EXISTING DISTRIBUTION PERCENTAGE











SITE

FISHER ROAD

138 [26] [35] (40 [35] (129] (135] (

= Channelized Island

= Signal-Controlled Approach

STOP = Stop-Controlled Approach

= AM Peak Hour Data
[#] = PM Peak Hour Data

SD : Site Driveway ILOS : Intersection Level of Service Schematic Drawing : Not To Scale ARBORVIEW

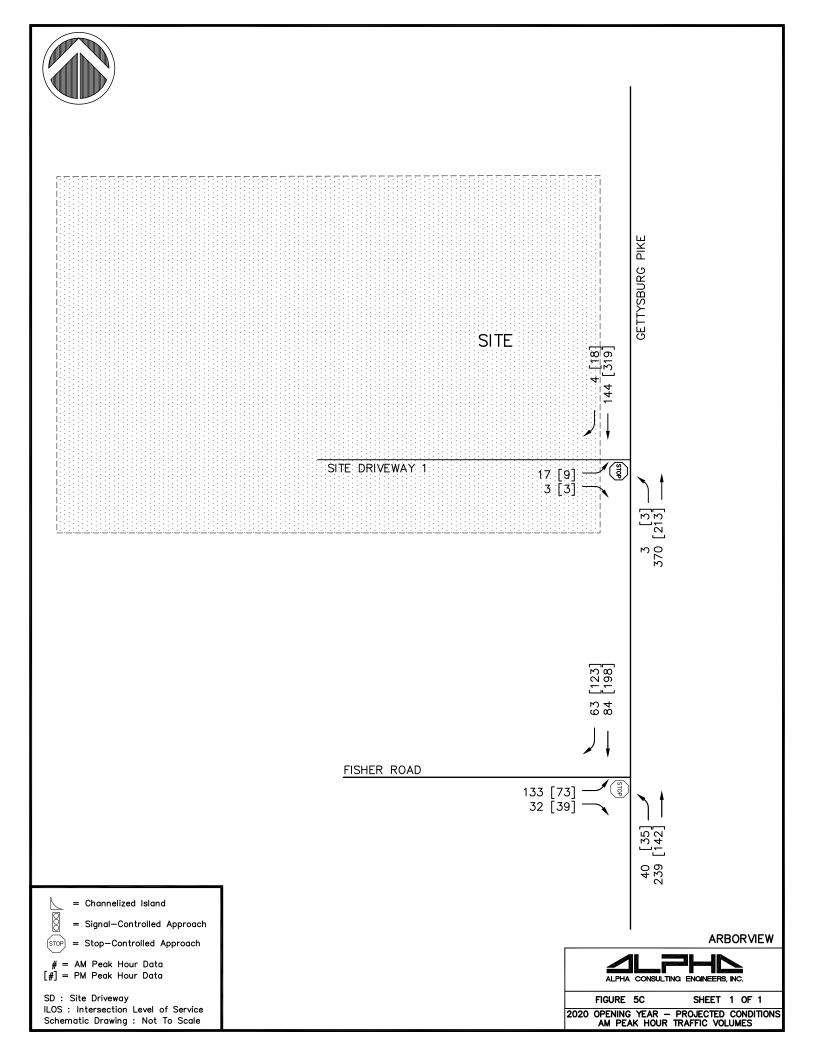


FIGURE 5A

GETTYSBURG PIKE

HEET 1 OF 1

2020 OPENING YEAR - BASE CONDITIONS PEAK HOUR TRAFFIC VOLUMES





SITE

FISHER ROAD

ILOS A [A]

C [B]

A [A]

= Channelized Island

= Signal-Controlled Approach

STOP = Stop-Controlled Approach

= AM Peak Hour Data
[#] = PM Peak Hour Data

SD : Site Driveway ILOS : Intersection Level of Service Schematic Drawing : Not To Scale ARBORVIEW

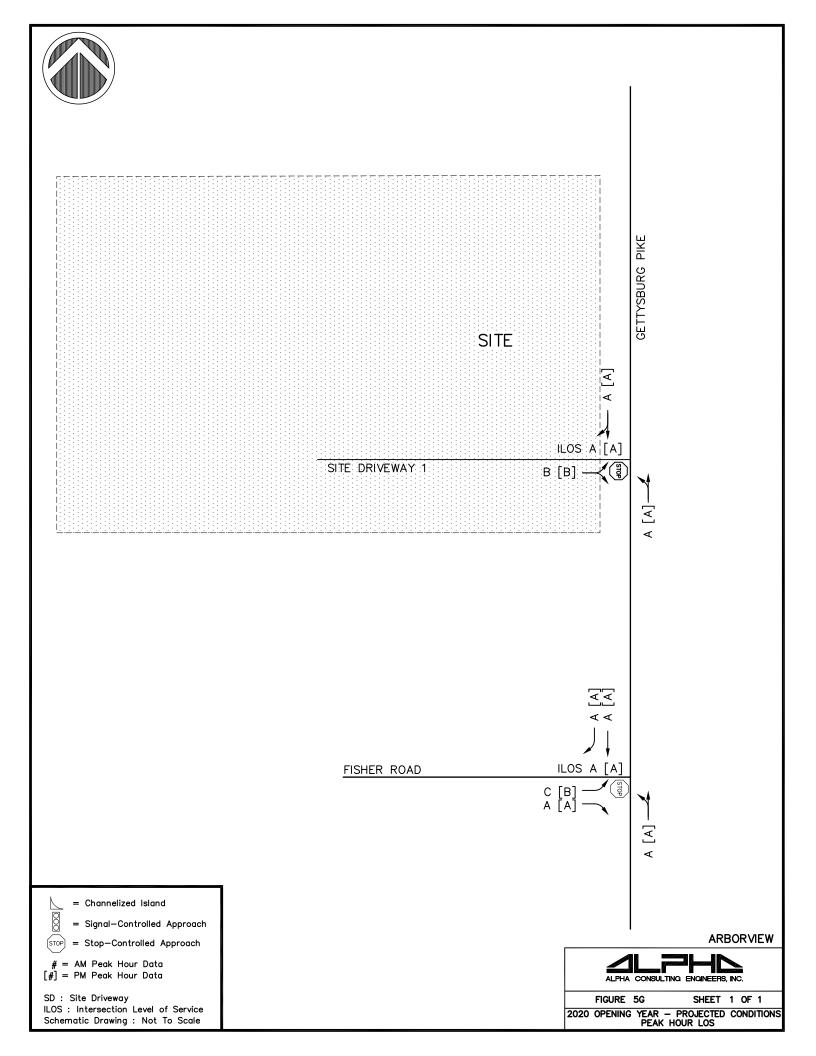


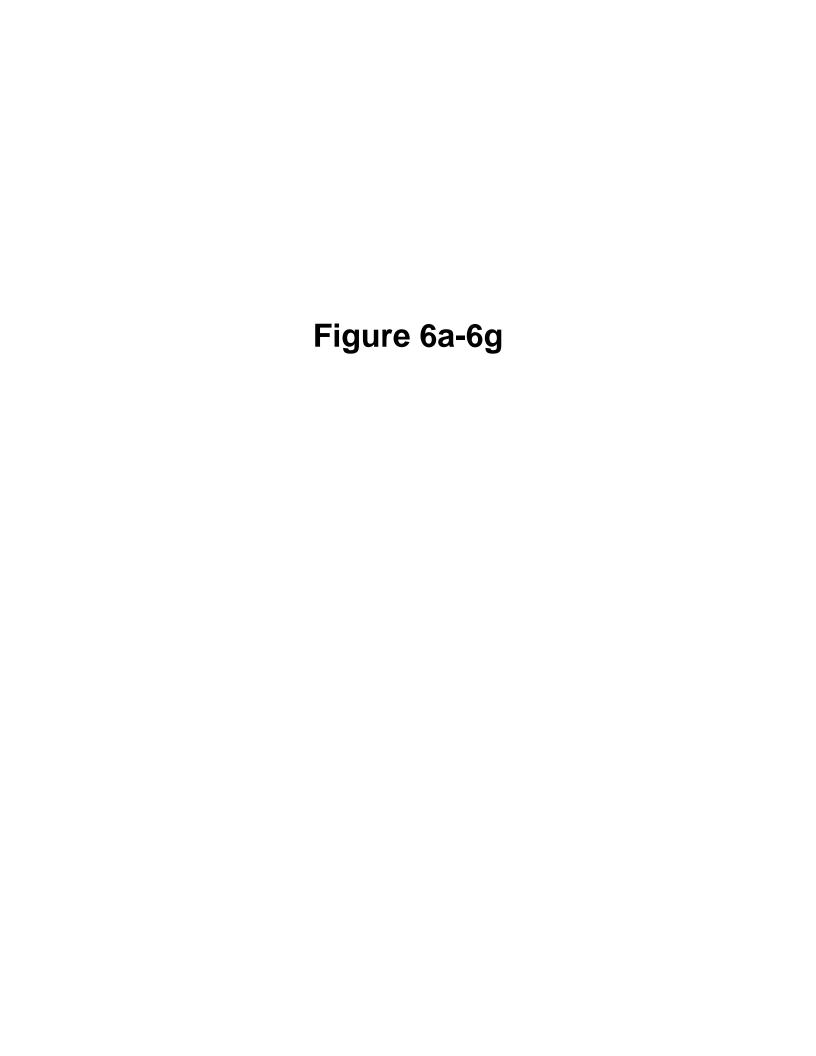
FIGURE 5E

GETTYSBURG PIKE

SHEET 1 OF 1

2020 OPENING YEAR - BASE CONDITIONS PEAK HOUR LOS

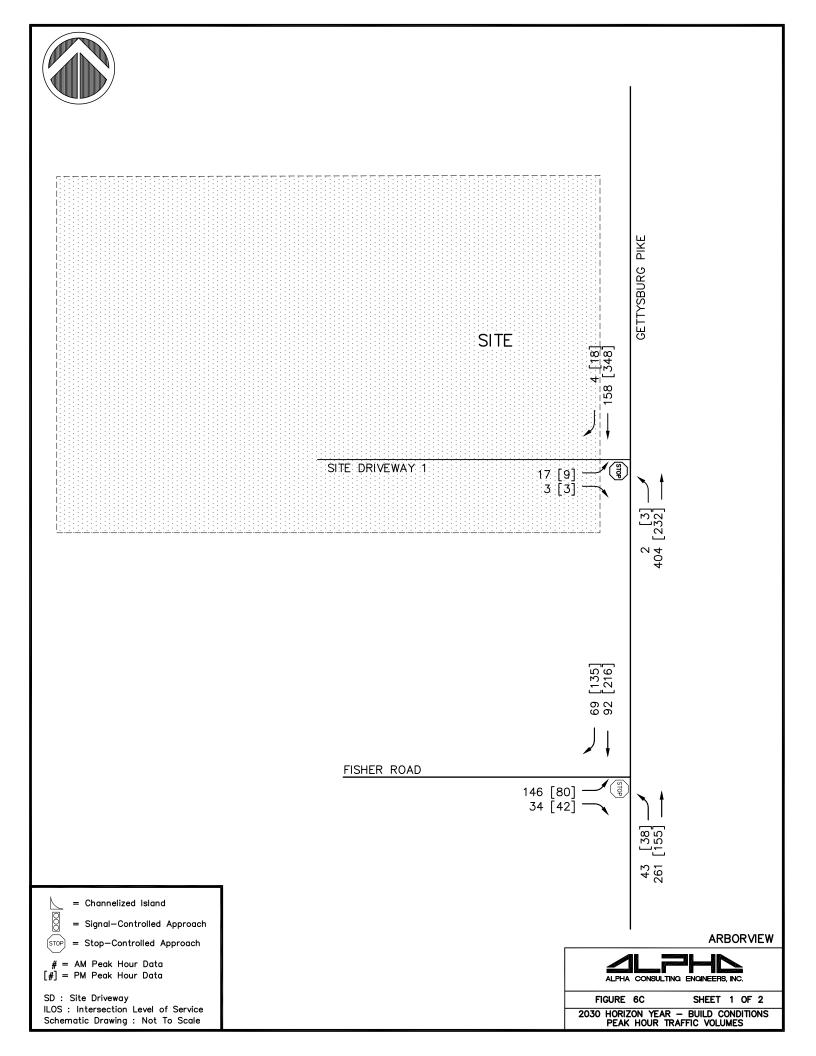






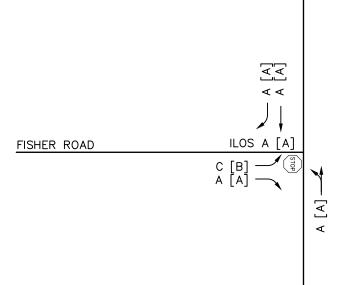
Schematic Drawing: Not To Scale

GETTYSBURG PIKE SITE FISHER ROAD 151 [83] ⁻ 34 [42] ⁻ = Channelized Island = Signal-Controlled Approach **ARBORVIEW** = Stop-Controlled Approach # = AM Peak Hour Data [#] = PM Peak Hour Data ALPHA CONSULTING ENGINEERS, INC. SD : Site Driveway FIGURE 6A ILOS: Intersection Level of Service 2030 HORIZON YEAR - BASE CONDITIONS PEAK HOUR TRAFFIC VOLUMES





SITE



= Channelized Island

= Signal-Controlled Approach

(STOP) = Stop—Controlled Approach

= AM Peak Hour Data

[#] = PM Peak Hour Data

SD : Site Driveway ILOS : Intersection Level of Service Schematic Drawing : Not To Scale ARBORVIEW

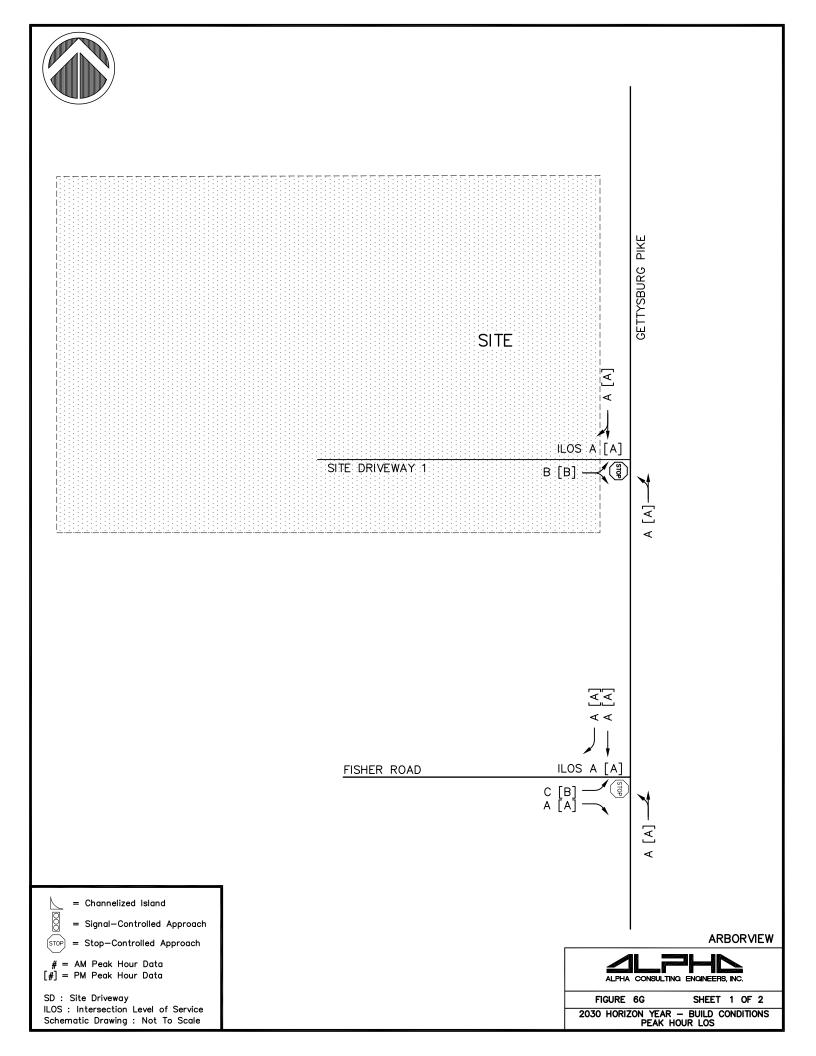


FIGURE 6E

GETTYSBURG PIKE

SHEET 1 OF 1

2030 HORIZON YEAR — BASE CONDITIONS PEAK HOUR LOS



Site Photographs

Fisher Road -Gettysburg Pike

Job Number: 317565 Date Taken: February 27, 2018 Fisher Road & Gettysburg Pike



Eastbound on Fisher Road – Approaching intersection



Eastbound on Fisher Road – Approaching intersection



Job Number: 317565

Westbound on Fisher Road – Departing intersection



Northbound on Gettysburg Pike – Approaching intersection



Job Number: 317565

Northbound on Gettysburg Pike – Departing intersection



Southbound on Gettysburg Pike – Approaching intersection

Job Number: 317565 Date Taken: February 27, 2018 Fisher Road & Gettysburg Pike



Southbound on Gettysburg Pike – Departing intersection



Southbound on Gettysburg Pike – Departing intersection

Site Driveway 1 -Gettysburg Pike



Eastbound on Site Driveway – Approaching intersection



Westbound on Site Driveway – Departing intersection



Westbound on Site Driveway – Departing intersection – Existing Driveways



Northbound on Gettysburg Pike – Approaching intersection



Northbound on Gettysburg Pike – Departing intersection



Northbound on Gettysburg Pike – Departing intersection



Southbound on Gettysburg Pike – Approaching intersection



Southbound on Gettysburg Pike – Approaching intersection



Southbound on Gettysburg Pike – Departing intersection

Existing Conditions

SITE NO: 28368	
County	CUMBERLAND (21)
Route	D013
Segment	0100
Dir	В
Current Avg Daily Traffic	4597
Current Avg Daily Truck Volume	92
K Factor	11
D Factor	60
T Factor	1
Truck Percent	2
Base Traffic Year	2014
Traffic Pattern Group	URBAN - MINOR ARTERIALS, COLLECTORS, LOCAL ROADS



Manual Turn Movement Data



Fisher Road - Gettysburg Pike AM

Weather: 30 Clear File Name: 317565 AM

Serial # 1626 Site Code : 01

By: Julie K. Start Date: 2/27/2018

Upper Allen Twp., Cumberland Co., PA Page No : 1

Groups Printed- Passenger Veh - Heavy Veh - Bus

			sher Ro				Get	tysburg		,			tysburg rom No			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
06:00	10	0	2	0	12	1	9	0	0	10	0	12	3	0	15	37
06:15	11	0	3	0	14	4	14	0	0	18	0	7	2	0	9	41
06:30	14	0	4	0	18	1	20	0	0	21	0	14	9	0	23	62
06:45	30	0	4	0	34	7	33	0	0	40	0	10	10	0	20	94
Total	65	0	13	0	78	13	76	0	0	89	0	43	24	0	67	234
07:00	29	0	4	0	33	3	58	0	0	61	0	10	12	0	22	116
07:15	37	0	4	0	41	6	44	0	0	50	0	17	10	0	27	118
07:30	38	0	15	0	53	11	81	0	0	92	0	20	18	0	38	183
07:45	33	0	9	0	42	15	63	0	0	78	0	20	21	0	41	161
Total	137	0	32	0	169	35	246	0	0	281	0	67	61	0	128	578
08:00	28	0	3	0	31	7	45	0	0	52	0	23	14	0	37	120
08:15	20	0	2	0	22	6	35	0	0	41	0	21	17	0	38	101
08:30	10	0	6	0	16	9	25	0	0	34	0	27	8	0	35	85
08:45	27	0	13	0	40	12	42	0	0	54	0	28	16	0	44	138
Total	85	0	24	0	109	34	147	0	0	181	0	99	55	0	154	444
Grand Total	287	0	69	0	356	82	469	0	0	551	0	209	140	0	349	1256
Apprch %	80.6	0	19.4	0		14.9	85.1	0	0		0	59.9	40.1	0		
Total %	22.9	0	5.5	0	28.3	6.5	37.3	0	0	43.9	0	16.6	11.1	0	27.8	
Passenger Veh	285	0	67	0	352	81	464	0	0	545	0	202	138	0	340	1237
% Passenger Veh	99.3	0	97.1	0	98.9	98.8	98.9	0	0	98.9	0	96.7	98.6	0	97.4	98.5
Heavy Veh	0	0	1	0	1	1	0	0	0	1	0	2	0	0	2	4
% Heavy Veh	0	0	1.4	0	0.3	1.2	0	0	0	0.2	0	1	0	0	0.6	0.3
Bus	2	0	1	0	3	0	5	0	0	5	0	5	2	0	7	15
% Bus	0.7	0	1.4	0	8.0	0	1.1	0	0	0.9	0	2.4	1.4	0	2	1.2



Fisher Road - Gettysburg Pike AM

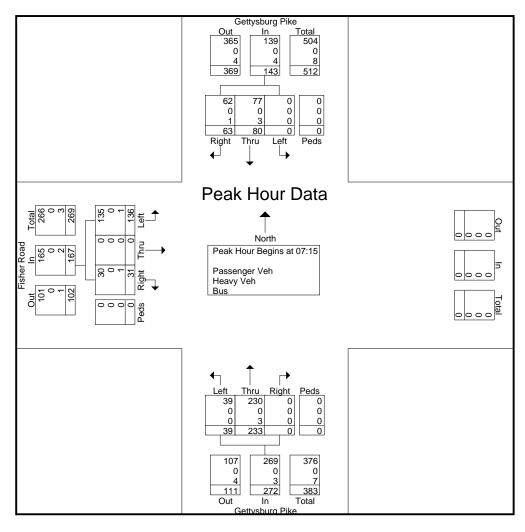
Weather: 30 Clear File Name: 317565 AM

Serial # 1626 Site Code : 01

By: Julie K. Start Date : 2/27/2018

Upper Allen Twp., Cumberland Co., PA Page No : 2

			sher Ro rom We					ysburg om Sou					tysburg rom No			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Analy						1 of 1										
Peak Hour for Er	ntire Inter	section	Begins a	at 7:15:0	00 AM											
7:15:00 AM	37	0	4	0	41	6	44	0	0	50	0	17	10	0	27	118
7:30:00 AM	38	0	15	0	53	11	81	0	0	92	0	20	18	0	38	183
7:45:00 AM	33	0	9	0	42	15	63	0	0	78	0	20	21	0	41	161
8:00:00 AM	28	0	3	0	31	7	45	0	0	52	0	23	14	0	37	120
Total Volume	136	0	31	0	167	39	233	0	0	272	0	80	63	0	143	582
% App. Total	81.4	0	18.6	0		14.3	85.7	0	0		0	55.9	44.1	0		
PHF	.895	.000	.517	.000	.788	.650	.719	.000	.000	.739	.000	.870	.750	.000	.872	.795
Passenger Veh	135	0	30	0	165	39	230	0	0	269	0	77	62	0	139	573
% Passenger Veh	99.3	0	96.8	0	98.8	100	98.7	0	0	98.9	0	96.3	98.4	0	97.2	98.5
Heavy Veh	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Heavy Veh	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bus	1	0	1	0	2	0	3	0	0	3	0	3	1	0	4	9
% Bus	0.7	0	3.2	0	1.2	0	1.3	0	0	1.1	0	3.8	1.6	0	2.8	1.5





Fisher Road - Gettysburg Pike PM

Weather: 50 Clear File Name: 317565 PM

Serial # 1626 Site Code : 01

By: Julie K. Start Date : 2/27/2018

Upper Allen Twp., Cumberland Co., PA Page No : 1

Groups Printed- Passenger Veh - Heavy Veh - Bus

	Fisher Road					Gettysburg Pike					Dus	Cott	tysburg	Diko		
			rom We					rom Soi					rom No			
O T:					I						1 6					
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
15:00	17	0	8	0	25	9	26	0	0	35	0	37	20	0	57	117
15:15	17	0	6	1	24	11	27	0	0	38	0	36	12	0	48	110
15:30	12	0	8	0	20	9	39	0	0	48	0	43	30	0	73	141
15:45	22	0	14	0	36	3	32	0	0	35	0	22	30	0	52	123
Total	68	0	36	1	105	32	124	0	0	156	0	138	92	0	230	491
16:00	18	0	10	0	28	10	19	0	0	29	0	47	23	0	70	127
16:15	11	0	9	0	20	6	36	0	0	42	0	51	25	0	76	138
16:30	25	0	8	0	33	5	32	0	0	37	0	58	27	0	85	155
16:45	18	0	12	0	30	6	22	0	0	28	0	44	24	0	68	126
Total	72	0	39	0	111	27	109	0	0	136	0	200	99	0	299	546
17:00	19	0	11	0	30	4	31	0	0	35	0	55	38	0	93	158
17:15	15	0	6	0	21	7	30	0	0	37	0	64	36	0	100	158
17:30	23	0	10	0	33	14	43	0	0	57	0	41	27	0	68	158
17:45	18	0	11	0	29	9	33	0	0	42	0	32	26	0	58	129
Total	75	0	38	0	113	34	137	0	0	171	0	192	127	0	319	603
18:00	16	0	3	0	19	5	23	0	0	28	0	31	15	0	46	93
18:15	19	0	4	0	23	6	23 31	0	0	37	0	33	23	0	56	116
18:30	19	0	6	0	23 25	5	33	0	0	38	0	აა 29	23 14	0	43	106
18:45	31	0	7	0	38	4	28	0	0	32	0	29	18	0	38	
Total	85	0	20	0	105	20	<u>2o_</u> 115	0	0	135	0	113	70	0	183	108 423
Total	65	U	20	U	105	20	115	U	U	135	U	113	70	U	103	423
Grand Total	300	0	133	1	434	113	485	0	0	598	0	643	388	0	1031	2063
Apprch %	69.1	0	30.6	0.2		18.9	81.1	0	0		0	62.4	37.6	0		
Total %	14.5	0	6.4	0	21	5.5	23.5	0	0	29	0	31.2	18.8	0	50	
Passenger Veh	300	0	131	1	432	113	482	0	0	595	0	642	387	0	1029	2056
% Passenger Veh	100	0	98.5	100	99.5	100	99.4	0	0	99.5	0	99.8	99.7	0	99.8	99.7
Heavy Veh	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
% Heavy Veh	0	0	0	0	0	0	0.2	0	0	0.2	0	0	0	0	0	0_
Bus	0	0	2	0	2	0	2	0	0	2	0	1	1	0	2	6
% Bus	0	0	1.5	0	0.5	0	0.4	0	0	0.3	0	0.2	0.3	0	0.2	0.3



Fisher Road - Gettysburg Pike PM

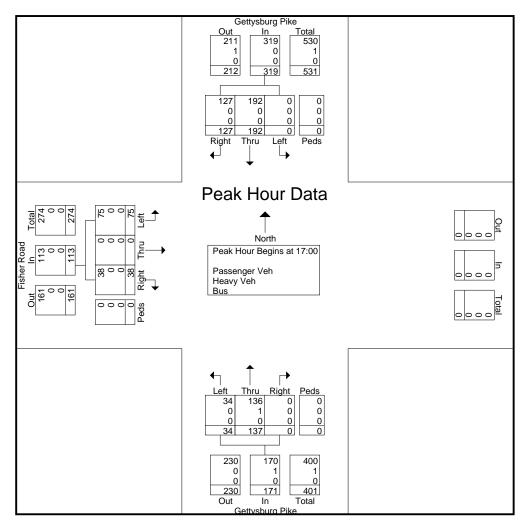
Weather: 50 Clear File Name: 317565 PM

Serial # 1626 Site Code : 01

By: Julie K. Start Date: 2/27/2018

Upper Allen Twp., Cumberland Co., PA Page No : 2

			sher Ro rom We					tysburg rom Sou					tysburg rom No			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Analy	sis From	3:00:00) PM to	6:45:00	PM - Peak	1 of 1							_			
Peak Hour for Er	ntire Inter	section	Begins a	at 5:00:0	00 PM											
5:00:00 PM	19	0	11	0	30	4	31	0	0	35	0	55	38	0	93	158
5:15:00 PM	15	0	6	0	21	7	30	0	0	37	0	64	36	0	100	158
5:30:00 PM	23	0	10	0	33	14	43	0	0	57	0	41	27	0	68	158
5:45:00 PM	18	0	11	0	29	9	33	0	0	42	0	32	26	0	58	129
Total Volume	75	0	38	0	113	34	137	0	0	171	0	192	127	0	319	603
% App. Total	66.4	0	33.6	0		19.9	80.1	0	0		0	60.2	39.8	0		
PHF	.815	.000	.864	.000	.856	.607	.797	.000	.000	.750	.000	.750	.836	.000	.798	.954
Passenger Veh	75	0	38	0	113	34	136	0	0	170	0	192	127	0	319	602
% Passenger Veh	100	0	100	0	100	100	99.3	0	0	99.4	0	100	100	0	100	99.8
Heavy Veh	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
% Heavy Veh	0	0	0	0	0	0	0.7	0	0	0.6	0	0	0	0	0	0.2
Bus	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bus	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0





	Growth I	actors for August 20	17 to July 2018	
County	Urban Interstate	Rural Interstate	Urban Non-Interstate	Rural Non-Interstate
ADAMS	*	*	1.06	0.76
ALLEGHENY	0.92	2.18	0.00	0.39
ARMSTRONG	0.93	*	0.00	0.40
BEAVER	0.87	1.98	0.00	0.36
BEDFORD	*	2.14	*	0.46
BERKS	1.23	2.44	0.33	0.60
BLAIR	0.88	1.94	0.00	0.38
BRADFORD	1.22	*	0.14	0.52
BUCKS	1.42	2.34	0.67	0.62
BUTLER	1.84	2.76	0.78	0.77
CAMBRIA CAMERON	0.47 *	*	0.00	0.21
CARBON	1.40	2.61	0.46	0.18 0.65
CENTRE	1.59	2.56	0.78	0.71
CHESTER	1.80	3.04	0.65	0.83
CLARION	1.03	2.04	0.05	0.43
CLEARFIELD	1.05	2.10	0.08	0.45
CLINTON	1.03	2.30	0.00	0.49
COLUMBIA	1.26	2.31	0.43	0.57
CRAWFORD	1.02	2.01	0.16	0.45
CUMBERLAND	1.63	2.57	0.87	0.72
DAUPHIN	1.42	*	0.54	0.66
DELAWARE	1.06	*	0.00	*
ELK	*	*	0.00	0.32
ERIE	1.07	2.16	0.06	0.46
FAYETTE	0.91	*	0.00	0.41
FOREST	*	*	*	0.68
FRANKLIN	1.42	2.58	0.60	0.68
FULTON	*	2.14	*	0.54
GREENE	1.29	2.63	0.03	0.59
HUNTINGDON	*	1.99	0.00	0.41
INDIANA	1.28	*	0.24	0.55
JEFFERSON	*	2.14	0.04 *	0.45
JUNIATA				0.59
LANCASTER	0.92	2.31	0.00 1.21	0.45
LANCASTER LAWRENCE	1.86 0.88	2.68 2.11	0.00	0.82 0.39
LEBANON	1.37	2.11	0.52	
LEHIGH	1.64	2.88	0.55	0.64 0.75
LUZERNE	0.84	2.17	0.00	0.42
LYCOMING	1.09	2.19	0.11	0.48
MCKEAN	0.73	*	0.00	0.35
MERCER	0.77	2.00	0.00	0.36
MIFFLIN	0.87	*	0.00	0.40
MONROE	1.50	2.49	0.81	0.70
MONTGOMERY	1.26	*	0.41	0.59
MONTOUR	1.59	2.66	0.41	0.68
NORTHAMPTON	1.39	2.56	0.54	0.66
NORTHUMBERLAND	0.91	2.12	0.00	0.43
PERRY	*	*	1.05	0.67
PHILADELPHIA	0.81	*	0.00	*
PIKE	2.26	2.87	1.72	1.00
POTTER	*	*	*	0.49
SCHUYLKILL	0.71	1.94	0.00	0.36
SNYDER	1.28	*	0.48	0.59
SOMERSET	0.73 *	1.78 *	0.00	0.35
SULLIVAN SUSQUEHANNA	1.22	2.27	0.40	0.45 0.56
TIOGA	*	2.21 *	0.40 *	0.56
UNION	1.63	2.48	0.95	0.52
VENANGO	0.73	1.73	0.00	0.72
WARREN	*	*	0.00	0.39
WARREN	1.38	2.63	0.00	0.59
WAYNE	*	2.26	0.29	0.54
WESTMORELAND	1.03	2.11	0.29	0.44
WYOMING	*	*	0.00	0.45
YORK	1.45	2.57	0.67	0.69
* = Functional Class Does		-		

^{* =} Functional Class Doesn't Exist in County

 $Questions?\ \ Please\ contact\ Andrew\ O'Neill\ at\ the\ Bureau\ of\ Planning\ and\ Research,\ 717-346-3250\ or\ and oneill\ @pa.gov$

NOTE: The projected growth factors are derived using historical VMT (Vehicle Miles Traveled) data (1994 to 2016), as well as Woods and Poole demographic and economic data. The factors should be compounded when calculating future values. The factors should not be used to project traffic beyond a 20-year period. Please be aware that these factors are estimates, and unforeseen events (opening of shopping centers, fast food franchises, gas stations, etc) could cause growth to change over time.



Future Volume Work Sheet:

Arborview 317565 1

Upper Allen Township, Cumberland Co., PA

Study Year: 2018 Growth Rate: 0.87

Time Period: Weekday AM Peak Hour of the Street Intersection: Fisher Road - Gettysburg Pike

		Fi	sher Road E	В		WB		Get	tysburg Pike	NB	Get	tysburg Pike	SB
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Study Year	2018	136		31				39	233			80	63
Opening Year	2020	138	0	32	0	0	0	40	237	0	0	81	64
Design Horizon Year	2030	151	0	34	0	0	0	43	259	0	0	89	70
Development Generation	1	-5							2			3	-1
With Development	2020	133	0	32	0	0	0	40	239	0	0	84	63
With Development	2030	146	0	34	0	0	0	43	261	0	0	92	69

Study Year: 2018 Growth Rate: 0.87

Time Period: Weekday PM Peak Hour of the Street Intersection: Fisher Road - Gettysburg Pike

		Fi	sher Road E	В		WB		Get	tysburg Pike	NB	Get	tysburg Pike	SB
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Study Year	2018	75		38				34	137			192	127
Opening Year	2020	76	0	39	0	0	0	35	139	0	0	195	129
Design Horizon Year	2030	83	0	42	0	0	0	38	152	0	0	213	141
Development Generatio	n	-3							3			3	-6
With Development	2020	73	0	39	0	0	0	35	142	0	0	198	123
With Development	2030	80	0	42	0	0	0	38	155	0	0	216	135

Study Year: 2018 Growth Rate: 0.87

Time Period: Weekday AM Peak Hour of the Street Intersection: Site Driveway - Gettysburg Pike

		Sit	e Driveway	EB		WB		Get	tysburg Pike	NB	Get	tysburg Pike	SB
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Study Year	2018								369			143	
Opening Year	2020	0	0	0	0	0	0	0	375	0	0	145	0
Design Horizon Year	2030	0	0	0	0	0	0	0	409	0	0	159	0
Development Generation		17		3				2	-5			-1	4
With Development	2020	17	0	3	0	0	0	2	370	0	0	144	4
With Development	2030	17	0	3	0	0	0	2	404	0	0	158	4

Study Year: 2018 Growth Rate: 0.87

Time Period: Weekday PM Peak Hour of the Street Intersection: Site Driveway - Gettysburg Pike

		Sit	e Driveway	EB		WB		Get	tysburg Pike	NB	Get	tysburg Pike	SB
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Study Year	2018								212			319	
Opening Year	2020	0	0	0	0	0	0	0	216	0	0	325	0
Design Horizon Year	2030	0	0	0	0	0	0	0	235	0	0	354	0
Development Generation	n	9		3				3	-3			-6	18
With Development	2020	9	0	3	0	0	0	3	213	0	0	319	18
With Development	2030	9	0	3	0	0	0	3	232	0	0	348	18

Trip Generation Worksheets

Single-Family Detached Housing

(210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies:

159

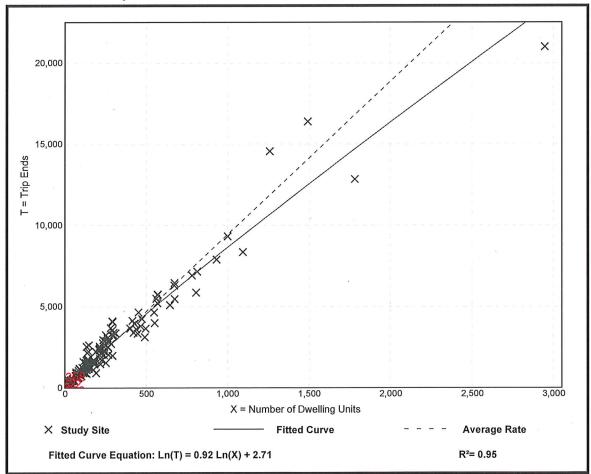
Avg. Num. of Dwelling Units:

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.44	4.81 - 19.39	2.10

Data Plot and Equation



Trip Generation Manual, 10th Edition • Institute of Transportation Engineers

Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

General Urban/Suburban Setting/Location:

Number of Studies: 173

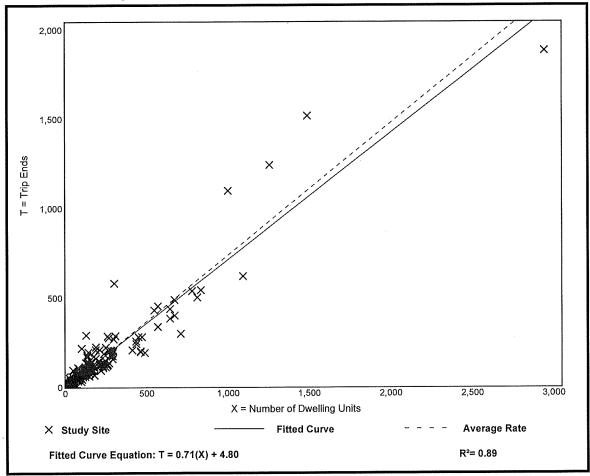
219 Avg. Num. of Dwelling Units:

Directional Distribution: 25% entering, 75% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.74	0.33 - 2.27	0.27

Data Plot and Equation



Trip Generation Manual, 10th Edition • Institute of Transportation Engineers

Single-Family Detached Housing

(210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

AM Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies:

157

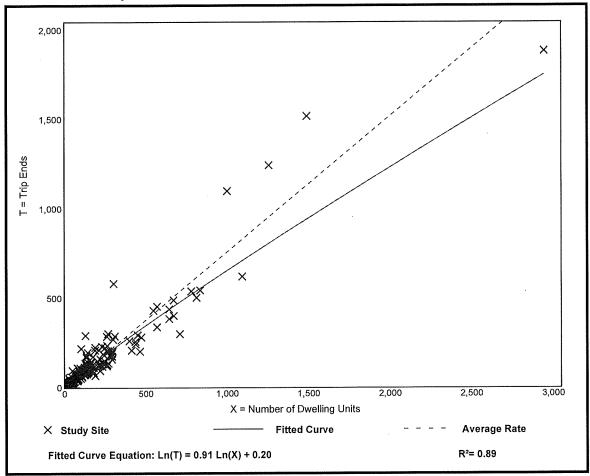
Avg. Num. of Dwelling Units: 231

26% entering, 74% exiting Directional Distribution:

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.76	0.36 - 2.27	0.26

Data Plot and Equation



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Single-Family Detached Housing

(210)

Vehicle Trip Ends vs:

Dwelling Units

On a:

Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location:

General Urban/Suburban

Number of Studies:

Avg. Num. of Dwelling Units: 242

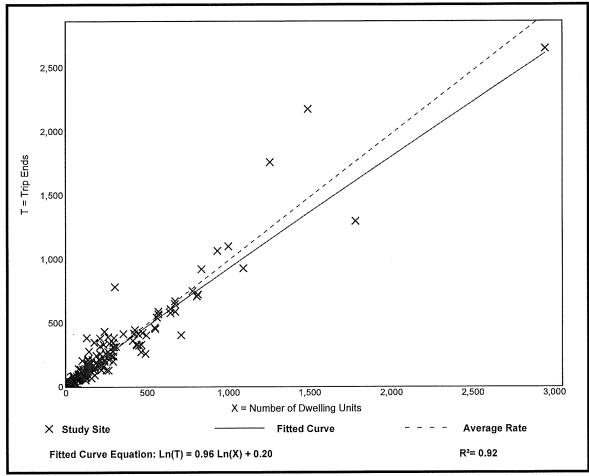
63% entering, 37% exiting Directional Distribution:

190

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.99	0.44 - 2.98	0.31

Data Plot and Equation



Trip Generation Manual, 10th Edition • Institute of Transportation Engineers

Single-Family Detached Housing

(210)

Vehicle Trip Ends vs: Dwelling Units

On a:

Weekday,

PM Peak Hour of Generator

Setting/Location:

General Urban/Suburban

Number of Studies:

165

Avg. Num. of Dwelling Units: 217

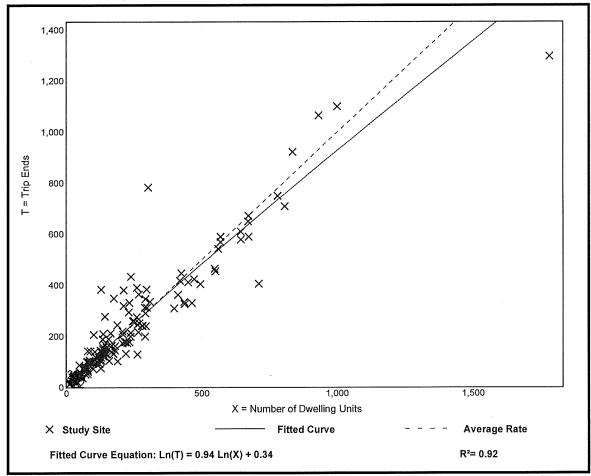
Directional Distribution:

64% entering, 36% exiting

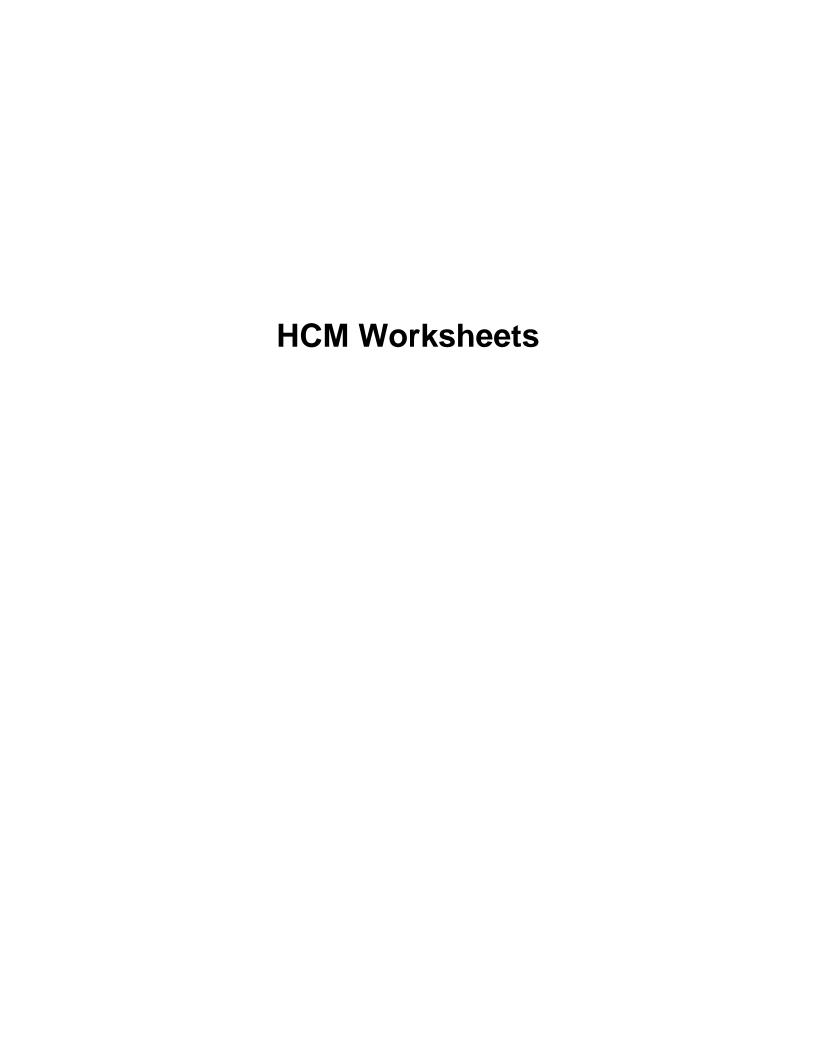
Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
1.00	0.49 - 2.98	0.31

Data Plot and Equation



Trip Generation Manual, 10th Edition • Institute of Transportation Engineers



2018 Baseline Scenario AM Peak Hour

1. Octtysburg i ikc	<u> </u>	i itout	4				
	٦	•	4	†	ļ	4	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*1	7		4	†	7	
Volume (vph)	136	31	39	233	80	63	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	11	11	10	10	
Grade (%)	6%			-2%	-2%		
Storage Length (ft)	210	0	0			170	
Storage Lanes	1	1	0			1	
Taper Length (ft)	200		25				
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		0.850				0.850	
Flt Protected	0.950			0.993			
Satd. Flow (prot)	1717	1521	0	1806	1722	1493	
Flt Permitted	0.950			0.993			
Satd. Flow (perm)	1717	1521	0	1806	1722	1493	
Link Speed (mph)	25			35	35		
Link Distance (ft)	571			711	637		
Travel Time (s)	15.6			13.9	12.4		
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80	
Heavy Vehicles (%)	2%	3%	2%	2%	4%	2%	
Adj. Flow (vph)	170	39	49	291	100	79	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	170	39	0	340	100	79	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08	
Turning Speed (mph)	15	9	15			9	
Sign Control	Stop			Free	Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza				IC	U Level	of Service	e A
Analysis Period (min) 15							

Synchro 8 Report Arborview

Intersection						
Int Delay, s/veh	4.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	136	31	39	233	80	63
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-		170
Veh in Median Storage, #		-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	3	2	2	4	2
Mvmt Flow	170	39	49	291	100	79
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	489	100	100	0	-	0
Stage 1	100	100	100	-	-	-
Stage 2	389	-	-	-	-	-
Critical Hdwy	7.1	6.2	4.3	_		_
Critical Hdwy Stg 1	7.1	-	-	-		-
Critical Hdwy Stg 2	6.62	-		-		-
Follow-up Hdwy	3	3.1	3	-	_	-
Pot Cap-1 Maneuver	557	1020	1110	-	-	-
Stage 1	1027	-	-	-	-	-
Stage 2	687	-	-	-		-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	527	1020	1110	-	-	-
Mov Cap-2 Maneuver	527	-	-	-	-	-
Stage 1	1027	-	-	-	-	-
Stage 2	651	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	13.9		1.2		0	
HCM LOS	В		1.2		0	
nom Loo						
Minor Lane/Major Mvmt	NBL	NBT EBLn1	EBLn2 SBT	SBR		
Capacity (veh/h)	1110		1020 -	- JUIX		
HCM Lane V/C Ratio	0.044	- 0.323		-		
HCM Control Delay (s)	8.4	0.323	8.7 -	-		
HCM Lane LOS	0.4 A	A C	A -	-		
HCM 95th %tile Q(veh)	0.1	- 1.4	0.1 -	-		
HOW 75th 75the Q(Veh)	0.1	- 1.4	0.1	-		

2020 Opening Year No Build Scenario AM Peak Hour

							_			•
	•	•	•	†	↓	4				
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR				
Lane Configurations	ሻ	7		4	†	7				
Volume (vph)	138	32	40	237	81	64				
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900				
Lane Width (ft)	12	12	11	11	10	10				
Grade (%)	6%			-2%	-2%					
Storage Length (ft)	210	0	0			170				
Storage Lanes	1	1	0			1				
Taper Length (ft)	200		25							
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00				
Frt		0.850				0.850				
Flt Protected	0.950			0.993						
Satd. Flow (prot)	1717	1521	0	1806	1722	1493				
Flt Permitted	0.950			0.993						
Satd. Flow (perm)	1717	1521	0	1806	1722	1493				
Link Speed (mph)	25			35	35					
Link Distance (ft)	571			711	637					
Travel Time (s)	15.6			13.9	12.4					
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80				
Heavy Vehicles (%)	2%	3%	2%	2%	4%	2%				
Adj. Flow (vph)	173	40	50	296	101	80				
Shared Lane Traffic (%)										
Lane Group Flow (vph)	172	40	0	346	101	80				
Enter Blocked Intersection	No	No	No	No	No	No				
Lane Alignment	Left	Right	Left	Left	Left	Right				
Median Width(ft)	12			0	0	-				
Link Offset(ft)	0			0	0					
Crosswalk Width(ft)	16			16	16					
Two way Left Turn Lane										
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08				
Turning Speed (mph)	15	9	15			9				
Sign Control	Stop			Free	Free					
Intersection Summary										
	Other									
Control Type: Unsignalized	Olliel									
Intersection Capacity Utiliza	tion 25 70/			IC	'III ovol i	of Service	0 /	١	\	١
Analysis Period (min) 15	11011 33.7 70			IC	O LEVEL	UI SEIVILE	,c F	١	1	1
Analysis Fehlu (IIIII) 13										

Arborview (No-Build Scenario) AM Peak Hour Street

Intersection						
Int Delay, s/veh	4.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	138	32	40	237	81	64
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-	-	170
Veh in Median Storage, #		-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	3	2	2	4	2
Mvmt Flow	172	40	50	296	101	80
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	497	101	101	0	iviajorz	0
Stage 1	101	- 101	101	-	-	-
Stage 2	396	-	_	_		_
Critical Hdwy	7.1	6.2	4.3	_		_
Critical Hdwy Stg 1	7.1	-	-	-		-
Critical Hdwy Stg 2	6.62	_	_	_		_
Follow-up Hdwy	3	3.1	3	-		-
Pot Cap-1 Maneuver	550	1019	1109	-		-
Stage 1	1025	-	-	-	_	-
Stage 2	680	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	520	1019	1109	-		-
Mov Cap-2 Maneuver	520	-	-	-	-	-
Stage 1	1025	-	-	-	-	-
Stage 2	643	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	14.1		1.2		0	
HCM LOS	В		1.2			
Minor Lane/Major Mvmt	NBL	NBT EBLn1 E	EBLn2 SBT	SBR		
Capacity (veh/h)	1109		1019 -	- JUIX		
HCM Lane V/C Ratio	0.045	- 0.332		-		
HCM Control Delay (s)	8.4	0.332	8.7 -	-		
HCM Lane LOS	0.4 A	A C	A -	-		
HCM 95th %tile Q(veh)	0.1	- 1.4	0.1 -	-		
TION 7501 70010 Q(VCII)	0.1	- 1.4	0.1 -	-		

2020 Opening Year Build Scenario AM Peak Hour

						,
	•	•	1	†	¥	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	۲	7		4	†	7
Volume (vph)	133	32	40	239	84	63
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.993		
Satd. Flow (prot)	1717	1521	0	1806	1722	1493
Flt Permitted	0.950			0.993		
Satd. Flow (perm)	1717	1521	0	1806	1722	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	2%	3%	2%	2%	4%	2%
Adj. Flow (vph)	166	40	50	299	105	79
Shared Lane Traffic (%)						
Lane Group Flow (vph)	166	40	0	349	105	79
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
	Other					
Control Type: Unsignalized	Otrici					
Intersection Capacity Utiliza	tion 35 5%			IC	III evel (of Service
Analysis Period (min) 15	11011 00.070			IC.	O LCVCI (or octation
Analysis i Gliou (Illili) 13						

Arborview (Build Scenario) AM Peak Hour Street

Intersection						
Int Delay, s/veh	4.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	133	32	40	239	84	63
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-		170
Veh in Median Storage, #		-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	3	2	2	4	2
Mvmt Flow	166	40	50	299	105	79
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	504	105	105	0	-	0
Stage 1	105	105	103	-	-	-
Stage 2	399	-	-	-	-	-
Critical Hdwy	7.1	6.2	4.3	_		_
Critical Hdwy Stg 1	7.1	-	-	-		-
Critical Hdwy Stg 2	6.62	-		-		-
Follow-up Hdwy	3	3.1	3	-	_	-
Pot Cap-1 Maneuver	544	1014	1106	-	-	-
Stage 1	1019	-	-	-	-	-
Stage 2	677	-	-	-		-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	515	1014	1106	-	-	-
Mov Cap-2 Maneuver	515	-	-	-	-	-
Stage 1	1019	-	-	-	-	-
Stage 2	640	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	14		1.2		0	
HCM LOS	В		1.2		U	
Minor Lane/Major Mvmt	NBL	NBT EBLn1	EBLn2 SBT	SBR		
Capacity (veh/h)	1106		1014 -	- 201		
HCM Lane V/C Ratio	0.045	- 0.323		-		
HCM Control Delay (s)	8.4	0.323	8.7 -	-		
HCM Lane LOS	A	A C	A -	-		
HCM 95th %tile Q(veh)	0.1	- 1.4	0.1 -	-		
1101VI 73111 701116 (VCII)	0.1	- 1.4	0.1 -	-		

	٠	•	•	†	ţ	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N/F			4	4î	
Volume (vph)	17	3	2	370	144	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	10	10	10	10
Grade (%)	5%			3%	-3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.981				0.997	
Flt Protected	0.959					
Satd. Flow (prot)	1709	0	0	1712	1726	0
Flt Permitted	0.959					
Satd. Flow (perm)	1709	0	0	1712	1726	0
Link Speed (mph)	25			35	35	
Link Distance (ft)	446			637	4124	
Travel Time (s)	12.2			12.4	80.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	4%	2%
Adj. Flow (vph)	18	3	2	402	157	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	21	0	0	404	161	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.03	1.03	1.11	1.11	1.07	1.07
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized
Intersection Capacity Utilization 31.1%
Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0.4					
·						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	17	3	2	370	144	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	5	-	-	3	-3	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	4	2
Mvmt Flow	18	3	2	402	157	4
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	566	159	161	0	-	0
Stage 1	159	,	-	-	-	-
Stage 2	407	-	-	-	-	-
Critical Hdwy	7.1	6.2	4.3	-	-	-
Critical Hdwy Stg 1	7.1	-	-	-	-	-
Critical Hdwy Stg 2	6.42	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	493	945	1058	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	685	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	492	945	1058	-	-	-
Mov Cap-2 Maneuver	492	-	-	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	684	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12.1		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR			
Capacity (veh/h)	1058	- 530				
HCM Lane V/C Ratio	0.002	- 0.041				
HCM Control Delay (s)	8.4	0.041				
HCM Lane LOS	Α	A B				
HCM 95th %tile Q(veh)	0	- 0.1				
/ 5 / 5 6 (* 6)	3	0.1				

2030 Opening Year No Build Scenario AM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	۲	7		€	†	7
Volume (vph)	151	34	43	259	89	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.993		
Satd. Flow (prot)	1717	1521	0	1806	1722	1493
Flt Permitted	0.950			0.993		
Satd. Flow (perm)	1717	1521	0	1806	1722	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	2%	3%	2%	2%	4%	2%
Adj. Flow (vph)	189	43	54	324	111	88
Shared Lane Traffic (%)						
Lane Group Flow (vph)	189	42	0	378	111	88
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12	3		0	0	3
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
ŭ						
Intersection Summary						
JI	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	ition 37.7%			IC	U Level	ot Service
Analysis Period (min) 15						

Arborview (No-Build Scenario) AM Peak Hour Street

Intersection							
Int Delay, s/veh	5						
·							
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	151	34	43	259	89	70	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	210	0	-	-	-	170	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	6	-	-	-2	-2	-	
Peak Hour Factor	80	80	80	80	80	80	
Heavy Vehicles, %	2	3	2	2	4	2	
Mvmt Flow	189	42	54	324	111	88	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	542	111	111	0	iviajorz	0	
Stage 1	111	- 111	- 111	-	-	-	
Stage 2	431	-	-	-	-	-	
Critical Hdwy	7.1	6.2	4.3		-	-	
Critical Hdwy Stg 1	7.1	0.2	4.3	-	-	-	
Critical Hdwy Stg 2	6.62	-	-	_	-	-	
Follow-up Hdwy	3	3.1	3	-	-	-	
Pot Cap-1 Maneuver	512	1006	1100	_	_	_	
Stage 1	1009	-	-	_		-	
Stage 2	647	_	_	_	_	_	
Platoon blocked, %	0-17			_	_	-	
Mov Cap-1 Maneuver	481	1006	1100	_		_	
Mov Cap-2 Maneuver	481	-	-	-		-	
Stage 1	1009	-	-		-	-	
Stage 2	608	-	-	-	_	-	
Approach	EB		NB		SB		
HCM Control Delay, s	15.6		1.2		0		
HCM LOS	C		1,2		U		
HOW LOO							
Minor Lano/Major Mumt	NBL	NBT EBLn1 EBLn2	SBT	SBR			
Minor Lane/Major Mvmt							
Capacity (veh/h)	1100	- 481 1006	-	-			
HCM Control Dolay (s)	0.049	- 0.392 0.042	-	-			
HCM Control Delay (s) HCM Lane LOS	8.4	0 17.2 8.7	-	-			
	A	A C A	-	-			
HCM 95th %tile Q(veh)	0.2	- 1.8 0.1	-	-			

2030 Opening Year Build Scenario AM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	٦	ř		4	†	ř
Volume (vph)	146	34	43	261	92	69
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.993		
Satd. Flow (prot)	1717	1521	0	1806	1722	1493
Flt Permitted	0.950			0.993		
Satd. Flow (perm)	1717	1521	0	1806	1722	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Heavy Vehicles (%)	2%	3%	2%	2%	4%	2%
Adj. Flow (vph)	183	43	54	326	115	86
Shared Lane Traffic (%)						
Lane Group Flow (vph)	182	42	0	380	115	86
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
	Other					
Control Type: Unsignalized	- ****					
Intersection Capacity Utiliza	tion 37.5%			IC	U Level	of Service
Analysis Period (min) 15				10	3 23 01 1	2. 23. 1100

Arborview (Build Scenario) AM Peak Hour Street

Intersection						
Int Delay, s/veh	4.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	146	34	43	261	92	69
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-	-	170
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	3	2	2	4	2
Mvmt Flow	182	42	54	326	115	86
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	549	115	115	0	-	0
Stage 1	115	-	-	-	-	-
Stage 2	434	-	-	-	-	-
Critical Hdwy	7.1	6.2	4.3	-	-	-
Critical Hdwy Stg 1	7.1	-	-	-	-	-
Critical Hdwy Stg 2	6.62	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	506	1001	1097	-	-	-
Stage 1	1003	-	-	-	-	-
Stage 2	644	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	476	1001	1097	-	-	-
Mov Cap-2 Maneuver	476	-	-	-	-	-
Stage 1	1003	-	-	-	-	-
Stage 2	605	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	15.6		1.2		0	
HCM LOS	С					
Minor Lane/Major Mvmt	NBL	NBT EBLn1 I	EBLn2 SBT	SBR		
Capacity (veh/h)	1097		1001 -	- JUIC		
HCM Lane V/C Ratio	0.049	- 0.383		-		
HCM Control Delay (s)	8.5	0.303	8.8 -	-		
HCM Lane LOS	0.5 A	A C	A -	-		
HCM 95th %tile Q(veh)	0.2	- 1.8	0.1 -	_		
/ 541 / 5410 (2(1011)	0.2	1.0	J			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	f	
Volume (vph)	17	3	2	404	158	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	10	10	10	10
Grade (%)	5%			3%	-3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.981				0.997	
Flt Protected	0.959					
Satd. Flow (prot)	1709	0	0	1712	1726	0
Flt Permitted	0.959					
Satd. Flow (perm)	1709	0	0	1712	1726	0
Link Speed (mph)	25			35	35	
Link Distance (ft)	446			637	4124	
Travel Time (s)	12.2			12.4	80.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	4%	2%
Adj. Flow (vph)	18	3	2	439	172	4
Shared Lane Traffic (%)						
Lane Group Flow (vph)	21	0	0	441	176	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.03	1.03	1.11	1.11	1.07	1.07
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Control Type: Unsignalized Intersection Capacity Utilization 32.9% Analysis Period (min) 15

ICU Level of Service A

Intersection							
Int Delay, s/veh 0	0.4						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	17	3	2	404	158	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	5	-	-	3	-3	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	4	2	
Mvmt Flow	18	3	2	439	172	4	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	617	174	176	0	-	0	
Stage 1	174	- 1/1	-	-	-	-	
Stage 2	443	_	-	-		-	
Critical Hdwy	7.1	6.2	4.3	_		_	
Critical Hdwy Stg 1	7.1	-	-	-		-	
Critical Hdwy Stg 2	6.42	-	_	-			
Follow-up Hdwy	3	3.1	3	-	_	-	
Pot Cap-1 Maneuver	455	927	1046	-		-	
Stage 1	915	-	-	-	_	-	
Stage 2	651		-	-	-	-	
Platoon blocked, %				-	_	-	
Mov Cap-1 Maneuver	454	927	1046	-		-	
Mov Cap-2 Maneuver	454	-	-	-	-	-	
Stage 1	915	-	-	-		-	
Stage 2	649	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	12.7		0		0		
HCM LOS	В						
200							
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	1046	- 492					
HCM Lane V/C Ratio	0.002	- 0.044					
HCM Control Delay (s)	8.4	0.044					
HCM Lane LOS	Α	A B					
HCM 95th %tile Q(veh)	0	- 0.1					
	0	0.1					

2018 Baseline Scenario PM Peak Hour

1: Gettysburg Pike	& Fishe	er Road	<u> </u>				(Baseline Scenario) PM Peak Hour Street
	۶	•	•	†	↓	✓	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	7	7		4	†	7	
Volume (vph)	75	38	34	137	192	127	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	11	11	10	10	
Grade (%)	6%			-2%	-2%		
Storage Length (ft)	210	0	0			170	
Storage Lanes	1	1	0			1	
Taper Length (ft)	200		25				
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		0.850				0.850	
Flt Protected	0.950			0.990			
Satd. Flow (prot)	1717	1536	0	1800	1756	1493	
Flt Permitted	0.950			0.990			
Satd. Flow (perm)	1717	1536	0	1800	1756	1493	
Link Speed (mph)	25			35	35		
Link Distance (ft)	571			711	637		
Travel Time (s)	15.6			13.9	12.4		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	79	40	36	144	202	134	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	79	40	0	180	202	134	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08	
Turning Speed (mph)	15	9	15			9	
Sign Control	Stop			Free	Free		
Intersection Summary							
	Othor						

Area Type:

Other

Control Type: Unsignalized

Intersection Capacity Utilization 33.4%

ICU Level of Service A

Analysis Period (min) 15

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	75	38	34	137	192	127
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-	-	170
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	79	40	36	144	202	134
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	418	202	202	0	-	0
Stage 1	202	-	-	-		-
Stage 2	216	-	-	-	-	-
Critical Hdwy	7.1	6.2	4.3	-	-	-
Critical Hdwy Stg 1	7.1	-	-	-	-	-
Critical Hdwy Stg 2	6.62	-	-	-		-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	623	893	1024	-	-	-
Stage 1	875	-	-	-	-	-
Stage 2	881	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	599	893	1024	-	-	-
Mov Cap-2 Maneuver	599	-	-	-	-	-
Stage 1	875	-	-	-	-	-
Stage 2	848	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11		1.7		0	
HCM LOS	В					
Minor Lane/Major Mvmt	NBL	NBT EBLn1 E	EBLn2 SBT	SBR		
Capacity (veh/h)	1024	- 599	893 -	-		
HCM Lane V/C Ratio	0.035	- 0.132		_		
HCM Control Delay (s)	8.6	0.132	9.2 -	_		
HCM Lane LOS	A	A B	Α -	-		
HCM 95th %tile Q(veh)	0.1	- 0.5	0.1 -	-		
	0.1	0.0	J			

2020 Opening Year No Build Scenario PM Peak Hour

	•	•	1	†	¥	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7		4	†	7
Volume (vph)	76	39	35	139	195	129
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.990		
Satd. Flow (prot)	1717	1536	0	1800	1756	1493
Flt Permitted	0.950			0.990		
Satd. Flow (perm)	1717	1536	0	1800	1756	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	80	41	37	146	205	136
Shared Lane Traffic (%)			<u>. </u>			.00
Lane Group Flow (vph)	80	41	0	183	205	136
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12		20.0	0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane	10			10	10	
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	15	1.00	1.00	9
Sign Control	Stop	,	10	Free	Free	,
	Этор			1100	1100	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized Intersection Capacity Utilization 33.7%

ICU Level of Service A

Analysis Period (min) 15

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(No-Build Scenario) PM Peak	Hour Street

Intersection							
Int Delay, s/veh 2	2.6						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	76	39	35	139	195	129	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	210	0	-	-	-	170	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	6	-	-	-2	-2	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	80	41	37	146	205	136	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	425	205	205	0	-	0	
Stage 1	205	-	-	-		-	
Stage 2	220	-	-	-		_	
Critical Hdwy	7.1	6.2	4.3	-		_	
Critical Hdwy Stg 1	7.1	-	-	-	-	-	
Critical Hdwy Stg 2	6.62	-	-	-	-	-	
Follow-up Hdwy	3	3.1	3	-	-	-	
Pot Cap-1 Maneuver	616	890	1022	-	-	-	
Stage 1	871	-	-	-	-	-	
Stage 2	876	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	592	890	1022	-		-	
Mov Cap-2 Maneuver	592	-	-	-	-	-	
Stage 1	871	-	-	-		-	
Stage 2	842	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	11.1		1.7		0		
HCM LOS	В		1.7		U		
HOW LOS							
Minor Lane/Major Mvmt	NBL	NBT EBLn1 E	BLn2 SBT	SBR			
Capacity (veh/h)	1022	- 592	000				
HCM Lane V/C Ratio	0.036	- 592 - 0.135 (-			
HCM Control Delay (s)	8.7	0.135 (0.046 - 9.2 -	-			
HCM Lane LOS	6.7 A	0 12 A B	Δ.	-			
				-			
HCM 95th %tile Q(veh)	0.1	- 0.5	0.1 -	-			

2020 Opening Year Build Scenario PM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ኻ	7		4	†	7
Volume (vph)	73	39	35	142	198	123
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.990		
Satd. Flow (prot)	1717	1536	0	1800	1756	1493
Flt Permitted	0.950			0.990		
Satd. Flow (perm)	1717	1536	0	1800	1756	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	77	41	37	149	208	129
Shared Lane Traffic (%)						
Lane Group Flow (vph)	77	41	0	186	208	129
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizat	ion 33.9%			IC	CU Level	of Service
Analysis Daried (win) 15						2. 00. 1100

Analysis Period (min) 15

Intersection							
Int Delay, s/veh	2.5						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	73	39	35	142	198	123	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	210	0	-	-	-	170	
Veh in Median Storage, #		-	-	0	0	-	
Grade, %	6	-	-	-2	-2	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	77	41	37	149	208	129	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	431	208	208	0	iviajoi z	0	
Stage 1	208	200	200	-	_	-	
Stage 2	223	-	-	_	_	_	
Critical Hdwy	7.1	6.2	4.3	_	<u>-</u>	_	
Critical Hdwy Stg 1	7.1	-	-	_	_	_	
Critical Hdwy Stg 2	6.62	-	-	-	_	-	
Follow-up Hdwy	3	3.1	3	-	-	-	
Pot Cap-1 Maneuver	610	887	1019	-	-	-	
Stage 1	867	-	-	-	-	-	
Stage 2	873	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	586	887	1019	-	-	-	
Mov Cap-2 Maneuver	586	-	-	-	-	-	
Stage 1	867	-	-	-	-	-	
Stage 2	838	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	11.1		1.7		0		
HCM LOS	В						
Minor Lane/Major Mvmt	NBL	NBT EBLn1 E	EBLn2 SBT	SBR			
Capacity (veh/h)	1019	- 586					
HCM Lane V/C Ratio	0.036	- 0.131		-			
HCM Control Delay (s)	8.7	0.131	9.3 -	-			
HCM Lane LOS	0.7 A	A B	A -	-			
HCM 95th %tile Q(veh)	0.1	- 0.5	0.1 -	-			
How /our /oure Q(veri)	0.1	- 0.5	0.1	=			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M.			4	₽	
Volume (vph)	9	3	3	213	319	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	10	10	10	10
Grade (%)	5%			3%	-3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.969				0.993	
Flt Protected	0.963			0.999		
Satd. Flow (prot)	1695	0	0	1711	1752	0
Flt Permitted	0.963			0.999		
Satd. Flow (perm)	1695	0	0	1711	1752	0
Link Speed (mph)	25			35	35	
Link Distance (ft)	446			637	4124	
Travel Time (s)	12.2			12.4	80.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	10	3	3	232	347	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	235	367	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.03	1.03	1.11	1.11	1.07	1.07
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 27.9% Analysis Period (min) 15 ICU Level of Service A

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	. NBT	SBT	SBR
Vol, veh/h	9	3	3	213	319	18
Conflicting Peds, #/hr	0	0	0		0	0
Sign Control	Stop	Stop	Free		Free	Free
RT Channelized	-	None				None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	5	-		. 3	-3	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	. 2	2	2
Mvmt Flow	10	3	3	232	347	20
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	595	357	366		- Wajorz	0
Stage 1	357	- 337	300		_	-
Stage 2	238	-			_	_
Critical Hdwy	7.1	6.2	4.3		-	-
Critical Hdwy Stg 1	7.1	-	-			-
Critical Hdwy Stg 2	6.42	-		_		-
Follow-up Hdwy	3	3.1	3	-	_	-
Pot Cap-1 Maneuver	471	729	899		-	-
Stage 1	686	-	-		-	-
Stage 2	865	-			-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	469	729	899	-	-	-
Mov Cap-2 Maneuver	469	-	-	-	-	-
Stage 1	686	-	-	-	-	-
Stage 2	862	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	12.2		0.1		0	
HCM LOS	В		J. 1		Ü	
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR			
Capacity (veh/h)	899					
HCM Lane V/C Ratio	0.004	- 515				
HCM Control Delay (s)	9	0.025				
HCM Lane LOS	A	0 12.2 A B				
HCM 95th %tile Q(veh)	0	- 0.1				
HOW FOUT WILLE (VEII)	U	- U. I				

2030 Opening Year No Build Scenario PM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	7		4	†	7	
Volume (vph)	83	42	38	152	213	141	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	11	11	10	10	
Grade (%)	6%			-2%	-2%		
Storage Length (ft)	210	0	0			170	
Storage Lanes	1	1	0			1	
Taper Length (ft)	200		25				
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		0.850				0.850	
Flt Protected	0.950			0.990			
Satd. Flow (prot)	1717	1536	0	1800	1756	1493	
Flt Permitted	0.950			0.990			
Satd. Flow (perm)	1717	1536	0	1800	1756	1493	
Link Speed (mph)	25			35	35		
Link Distance (ft)	571			711	637		
Travel Time (s)	15.6			13.9	12.4		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	87	44	40	160	224	148	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	87	44	0	200	224	148	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08	
Turning Speed (mph)	15	9	15			9	
Sign Control	Stop			Free	Free		
Intersection Summary							
Jr -	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 35.9%			IC	U Level	of Service	e A

Analysis Period (min) 15

Intersection							
Int Delay, s/veh	2.7						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	83	42	38	152	213	141	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	210	0	-	-	-	170	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	6	-	-	-2	-2	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	87	44	40	160	224	148	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	464	224	224	0	IVIQ 012	0	
Stage 1	224	-	-	-	_	-	
Stage 2	240	_	-	-	_	-	
Critical Hdwy	7.1	6.2	4.3	_	_	_	
Critical Hdwy Stg 1	7.1	-	-	-	_	_	
Critical Hdwy Stg 2	6.62	-	_	_	_	_	
Follow-up Hdwy	3	3.1	3	_	_	_	
Pot Cap-1 Maneuver	579	868	1007	_	<u>.</u>	_	
Stage 1	846	-	-	-	_	_	
Stage 2	852	_		_	_	_	
Platoon blocked, %	002			_	_	_	
Mov Cap-1 Maneuver	554	868	1007	_	_	_	
Mov Cap-2 Maneuver	554	-	-	-	-	-	
Stage 1	846	-	_	_	-	-	
Stage 2	815	-	-	-	_	-	
Approach	EB		NB		SB		
HCM Control Delay, s	11.6		1.7		0		
HCM LOS	В		1.7		0		
10.11 200							
Minor Lane/Major Mvmt	NBL	NBT EBLn1 EB	3Ln2 SBT	SBR			
Capacity (veh/h)	1007	- 554	868 -	- JUIK			
HCM Lane V/C Ratio	0.04	- 0.158 0		-			
HCM Control Delay (s)	8.7	0.138 0	9.4 -	-			
HCM Lane LOS	Α.7	A B	A -	-			
HCM 95th %tile Q(veh)	0.1	- 0.6	0.2	-			
HOW FOUT MILE Q(VEH)	0.1	- 0.0	0.2 -	-			

2030 Opening Year Build Scenario PM Peak Hour

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7		र्स	†	7
Volume (vph)	80	42	38	155	216	135
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	11	11	10	10
Grade (%)	6%			-2%	-2%	
Storage Length (ft)	210	0	0			170
Storage Lanes	1	1	0			1
Taper Length (ft)	200		25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850				0.850
Flt Protected	0.950			0.990		
Satd. Flow (prot)	1717	1536	0	1800	1756	1493
Flt Permitted	0.950			0.990		
Satd. Flow (perm)	1717	1536	0	1800	1756	1493
Link Speed (mph)	25			35	35	
Link Distance (ft)	571			711	637	
Travel Time (s)	15.6			13.9	12.4	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	84	44	40	163	227	142
Shared Lane Traffic (%)						
Lane Group Flow (vph)	84	44	0	203	227	142
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.04	1.04	1.03	1.03	1.08	1.08
Turning Speed (mph)	15	9	1.03	1.00	1.00	9
Sign Control	Stop	,	10	Free	Free	,
	Otop			1700	1700	
Intersection Summary						
31	Other					
Control Type: Unsignalized						

ICU Level of Service A

Intersection Capacity Utilization 36.1%
Analysis Period (min) 15

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Vol, veh/h	80	42	38	155	216	135
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	210	0	-	-	-	170
Veh in Median Storage, #		-	-	0	0	-
Grade, %	6	-	-	-2	-2	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	84	44	40	163	227	142
Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	470	227	227	0	-	0
Stage 1	227	-	-	-	-	-
Stage 2	243	-	-	-		-
Critical Hdwy	7.1	6.2	4.3	-		-
Critical Hdwy Stg 1	7.1	-	-	-	_	-
Critical Hdwy Stg 2	6.62	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	574	865	1004	-	-	-
Stage 1	842	-	-	-	-	-
Stage 2	848	-	-	-		-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	549	865	1004	-	-	-
Mov Cap-2 Maneuver	549	-	-	-	-	-
Stage 1	842	-	-	-	-	-
Stage 2	811	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11.6		1.7		0	
HCM LOS	В					
Minor Lane/Major Mvmt	NBL	NBT EBLn1	EBLn2 SBT	SBR		
Capacity (veh/h)	1004	- 549	865 -	-		
HCM Lane V/C Ratio	0.04	- 0.153		_		
HCM Control Delay (s)	8.7	0 12.7	9.4 -	_		
HCM Lane LOS	A	A B	Α -	-		
HCM 95th %tile Q(veh)	0.1	- 0.5	0.2 -	_		
	0.1	0.0	U.L			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ની	f	
Volume (vph)	9	3	3	292	348	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	10	10	10	10
Grade (%)	5%			3%	-3%	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.969				0.993	
Flt Protected	0.963					
Satd. Flow (prot)	1695	0	0	1712	1752	0
Flt Permitted	0.963					
Satd. Flow (perm)	1695	0	0	1712	1752	0
Link Speed (mph)	25			35	35	
Link Distance (ft)	446			637	4124	
Travel Time (s)	12.2			12.4	80.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	10	3	3	317	378	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	320	398	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	12			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.03	1.03	1.11	1.11	1.07	1.07
Turning Speed (mph)	15	9	15			9
Sign Control	Stop			Free	Free	
Intersection Summary						
	OIL					

Area Type: Control Type: Unsignalized Other

Intersection Capacity Utilization 29.4% Analysis Period (min) 15

ICU Level of Service A

ntersection							
Int Delay, s/veh 0.	.3						
		555				000	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	9	3	3	292	348	18	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-		-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	5	-	-	3	-3	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	10	3	3	317	378	20	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	712	388	398	0	-	0	
Stage 1	388	-	-	-	-	-	
Stage 2	324	-	-	-	-	-	
Critical Hdwy	7.1	6.2	4.3	-	-	-	
Critical Hdwy Stg 1	7.1	-	-	-	-	-	
Critical Hdwy Stg 2	6.42	-	-	-	-	-	
Follow-up Hdwy	3	3.1	3	-		-	
Pot Cap-1 Maneuver	391	700	877	-	-	-	
Stage 1	653	-	-	-	-	-	
Stage 2	768	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	389	700	877	-	-	-	
Mov Cap-2 Maneuver	389	-	-	-	-	-	
Stage 1	653	-	-	-	-	-	
Stage 2	765	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	13.5		0.1		0		
HCM LOS	В		0.1				
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	877	- 438					
HCM Lane V/C Ratio	0.004	- 0.03					
HCM Control Delay (s)	9.1	0.05					
HCM Lane LOS	Α	A B					
HCM 95th %tile Q(veh)	0	- 0.1					

Turn Lane Warrant Worksheets

Right Turn Lane 2020 Opening Year Build Scenario

Turn Lane Warrant and Length Analysis Workbook

			DI LOCA	ATION AN					
	Mur	nicipality:	Upper A	llen Twp		Analysis I	Date:	2/28/	2018
		County:		nd County		Conducte		MI	
PennDOT	Engineering		8	8		Checke			
					Age	ency/Company N		ALPH	A CEI
Intersection & Ap	proach De	scription: Prop	osed Site Dr	riveway 1/ Get	tysburg Pike -	Southbound Adv	ancing		
	Analys	is Period:	2020	Build		Number o	f Approacl	h Lanes:	1
	-	ign Hour:	AM Pea	ak Hour		Undivided or			Undivided
ı	ntersection	າ Control:	Unsign	nalized					
Posted	l Speed Lim	of Terrain:		B5 Iling		Left or Right-Tu	rn Lana Ar		pe of Analysis ight Turn Lane
	Турс о	T Terrain.		VOLUME	CALCIIIA		IIII Laile Ai	ialysis:	ight rum Lunc
				eft Turn Lane		liculations			
Movemen	Left	Include? Yes	Volume	% Trucks 0.0%	PCEV N/A			dvancing Volu	MAC NI/A
Advancing	Through	res		0.0%	N/A N/A			avancing volu Opposing Volu	
Auvaileing	Right	Yes	0	0.0%	N/A			Jpposing voit Left Turn Volt	
	Left	Yes	0	0.0%	N/A		'		IV/A
Opposing	Through	-	-	0.0%	N/A				
- 1-1 0	Right	Yes		0.0%	N/A	% Left	Turns in A	dvancing Volu	ıme: N/A
			Riç	ght Turn Lan	e Volume Ca	alculations			
Movemen	ıt	Include?	Volume	% Trucks	PCEV				
	Left	No	0	0.0%	N/A				
Advancing	Through	-	144	4.0%	153			dvancing Volu	
	Right	-	4	0.0%	4		Ri	ight Turn Volu	ıme: 4
			TUR	N LANE W	VARRANT	FINDINGS			
Le	ft Turn La	ne Warrant F	indings			Righ	Turn Laı	ne Warrant I	indings
Applicable \	Warrant Fi	igure:	N/A			Applicable W	arrant Fig	ure: Fig	ure 9
	Warrant I	Met?:	N/A			v	/arrant M	et?:	No
			TURN	LANE LEI	NGTH CAL	CULATIONS			
	ntersection	Control:	Unsignalize	ed					
ı		ing Land	4						
I Design Hour Volu	ime of Turn	ung Lane:							
Design Hour Volu	ıme of Turn Per Hour (A		60						
Design Hour Volu Cycles F		ssumed):			Average #	of Vehicles/Cycl	e:	N/A	
Design Hour Volu Cycles F	Per Hour (A	ssumed):	60	PennDOT Pub			e:	N/A]
Design Hour Volu Cycles F	Per Hour (A	ssumed):	60		lication 46, Ex	hibit 11-6 ed (MPH)]
Design Hour Volu Cycles F	Per Hour (A Per Hour (If	ssumed):	60	PennDOT Publ	lication 46, Ex	hibit 11-6 ed (MPH) 40-45		N/A :0-60	
Design Hour Volu Cycles F	Per Hour (A Per Hour (If	ssumed): f Known):	60	25-35	lication 46, Ex Spec	hibit 11-6 ed (MPH) 40-45 mand Volume	5	0-60	
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type c	Assumed): f Known):	60 0	25-35 Low	Specification 46, Ex	hibit 11-6 ed (MPH) 40-45 mand Volume Low	5 High	0-60 Low	
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	ssumed): f Known):	60	25-35	lication 46, Ex Spec	hibit 11-6 ed (MPH) 40-45 mand Volume	5	0-60	
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A	Turn Der High B or C	hibit 11-6 ed (MPH) 40-45 mand Volume Low B or C B	High B or C B or C	0-60 Low B or C B	Feet
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A	Turn Der High B or C	hibit 11-6 ed (MPH) 40-45 mand Volume Low B or C B Length, Condition	High B or C B or C	Low B or C B	Feet Feet
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A	Turn Der High B or C	hibit 11-6 ed (MPH) 40-45 mand Volume Low B or C B B Length, Condition	High B or C B or C	Low B or C B N/A N/A	Feet
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A Right Turn La	Turn Dei High B or C C ane Storage I	hibit 11-6 ed (MPH) 40-45 Mand Volume Bor C B Length, Condition Condition	High B or C B or C on A: on B: on C:	Low B or C B N/A N/A N/A	Feet Feet
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A Right Turn La	Turn Dei High B or C C ane Storage I	hibit 11-6 ed (MPH) 40-45 mand Volume Low B or C B B Length, Condition	High B or C B or C on A: on B: on C: agth:	Low B or C B N/A N/A N/A N/A	Feet Feet Feet
Design Hour Volu Cycles F	Per Hour (A Per Hour (If Type o	Assumed): If Known): Of Traffic Contro	High A	25-35 Low A A Right Turn La	Turn Dei High B or C C ane Storage I	hibit 11-6 ed (MPH) 40-45 Mand Volume Bor C B Length, Condition Condition	High B or C B or C on A: on B: on C: agth:	Low B or C B N/A N/A N/A	Feet Feet Feet



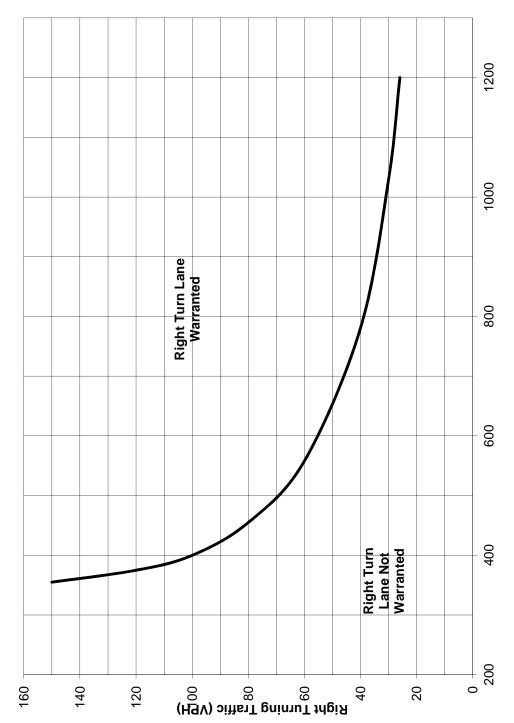


Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

Advancing Volume including Right Turns (VPH)

Turn Lane Warrant and Length Analysis Workbook

	Mu	nicipality:	Upper A	Allen Twp		Analysis I	Date:	2/28/2	2018
		County:		ind County		Conducte		ME	
PennDOT	Engineerin	g District:		8		Checke	d By:		
					Α	gency/Company N	ame:	ALPHA	A CEI
Intersection & A	pproach De	scription: Prop	osed Site D	riveway 1 / G	ettysburg Pik	e - Southbound Adv	/ancing		
	Analys	sis Period:	2020) Build		Number o	of Approach	n Lanes:	1
		sign Hour:		ak Hour		Undivided or	Divided H	ighway:	Undivided
	Intersection d Speed Lim			nalized 35				-	
Poste	-	of Terrain:		lling		Left or Right-Tu	ırn Lane Ar		pe of Analysis ght Turn Lane
				VOLUMI	E CALCUL	ATIONS			
			Lo	eft Turn Laı	ne Volume C	Calculations			
Moveme	nt	Include?	Volume	% Trucks	PCEV]			
	Left	Yes		0.0%	N/A			dvancing Volu	
Advancing	Through	- Van	0	0.0%	N/A			Opposing Volu	
	Right Left	Yes Yes	0	0.0%	N/A N/A	+	'	Left Turn Volu	me: N/A
Opposing	Through	-		0.0%	N/A				
	Right	Yes		0.0%	N/A	% Left	Turns in A	dvancing Volu	me: N/A
			Ri	ght Turn La	ne Volume	Calculations			
Moveme	nt	Include?	Volume	% Trucks	PCEV]			
	Left	No	0	0.0%	N/A				247
Advancing	Through Right	-	319 18	2.0% 0.0%	329 18	+		dvancing Volu ight Turn Volu	
								6	
				KN LANE	WARRAN	T FINDINGS			
Le	ft Turn La	ane Warrant F	indings			Right	t Turn Lai	ne Warrant F	indings
Applicable	Warrant F	igure:	N/A]		Applicable W	arrant Fig	ure: Figu	ure 9
	Warrant	Met?:	N/A			W	/arrant M	et?: N	No
			TURN	I LANE LE	NGTH CA	LCULATIONS			
	Intersection	n Control:	Unsignalize	ed					
Design Hour Vol			18						
	Per Hour (A		60		A	# of Mahialas /Coss		NI/A	1
Cycles	Per Hour (I	r known):	0		Average	# of Vehicles/Cycl	e:	N/A	J
				PennDOT Pu	blication 46, I				1
				25-35	Sp	eed (MPH) 40-45	5	0-60	-
	Type	of Traffic Contro	ı		Turn D	emand Volume			
			High	Low	High	Low	High	Low	
		Signalized Unsignalized	A	A	B or C	B or C	B or C	B or C	-
		7.10.B.10.12.00				1			_ ■
				Right Turn	Lane Storage	Length, Condition	on A:	N/A	Feet
						Condition	on B:	N/A	Feet
						Condition	on C:	N/A	Feet
				Require	d Right Turn	Lane Storage Ler	ngth:	N/A	Feet
				-		-			-
							Δdditic	nal Findings	•
							Additio	nal Findings: N/A	



2/28/2018 2020 Build PM SBRT02

Right Turn Lane Warranted 347, 18 Right Turn Lane Not Warranted Right Turning Traffic (VPH)

Advancing Volume including Right Turns (VPH)

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

Right Turn Lane 2030 Opening Year Build Scenario

Turn Lane Warrant and Length Analysis Workbook

STUDY LOCATION AND ANALYSIS INFORMATION Upper Allen Twp 2/28/2018 Municipality **Analysis Date:** County **Cumberland County** Conducted By MEA PennDOT Engineering District: 8 Checked By ALPHA CEI Agency/Company Name: Intersection & Approach Description: Proposed Site Driveway 1/ Gettysburg Pike - Southbound Advancing 2030 Build **Analysis Period Number of Approach Lanes: Design Hour** AM Peak Hour Undivided or Divided Highway: Undivided Intersection Control Unsignalized Posted Speed Limit (MPH) Type of Analysis 35 Type of Terrain: Rolling Right Turn Lane Left or Right-Turn Lane Analysis?: **VOLUME CALCULATIONS** Left Turn Lane Volume Calculations Include? Volume % Trucks **PCEV** Movement Left Yes 0.0% N/A **Advancing Volume:** N/A Advancing 0.0% N/A N/A Through **Opposing Volume:** Right Yes 0 0.0% N/A Left Turn Volume: N/A 0 0.0% N/A Left Yes Opposing Through 0.0% N/A 0.0% N/A Right Yes N/A % Left Turns in Advancing Volume: **Right Turn Lane Volume Calculations** Include? Volume % Trucks **PCFV** Movement Left 0.0% N/A Nο 0 158 4.0% **Advancing Volume:** Advancing Through 168 172 Right 0.0% Right Turn Volume: 4 **TURN LANE WARRANT FINDINGS Left Turn Lane Warrant Findings Right Turn Lane Warrant Findings** N/A **Applicable Warrant Figure: Applicable Warrant Figure:** Figure 9 Warrant Met?: N/A Warrant Met?: No **TURN LANE LENGTH CALCULATIONS** Unsignalized Intersection Control: **Design Hour Volume of Turning Lane** Cycles Per Hour (Assumed): 60 Cycles Per Hour (If Known): 0 Average # of Vehicles/Cycle: N/A PennDOT Publication 46, Exhibit 11-6 Speed (MPH) 50-60 25-35 40-45 Type of Traffic Control **Turn Demand Volume** High High Low High Low Low Signalized Α Α B or C B or C B or C B or C Unsignalized B or C Right Turn Lane Storage Length, Condition A N/A Feet **Condition B** N/A Feet N/A **Condition C** Feet N/A Required Right Turn Lane Storage Length: Feet **Additional Findings:** Additional Comments / Justifications:



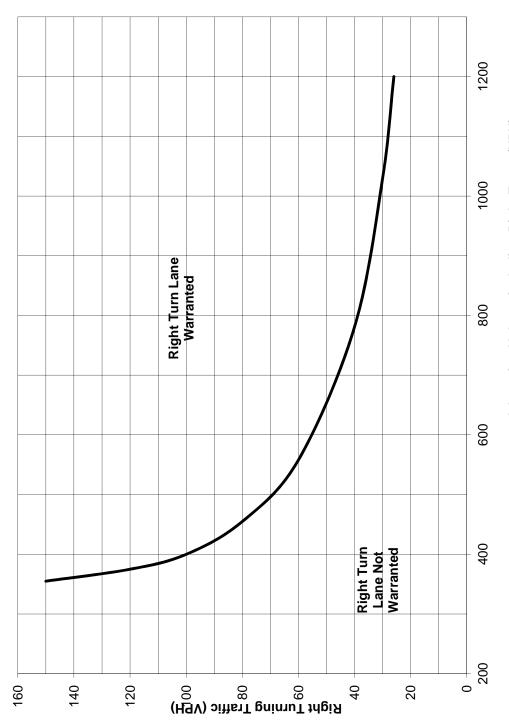


Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

Advancing Volume including Right Turns (VPH)

Turn Lane Warrant and Length Analysis Workbook

STUDY LOCATION AND ANALYSIS INFORMATION Upper Allen Twp 2/28/2018 Municipality **Analysis Date:** County **Cumberland County** Conducted By MEA PennDOT Engineering District: 8 Checked By ALPHA CEI Agency/Company Name: Intersection & Approach Description: Proposed Site Driveway 1 / Gettysburg Pike - Southbound Advancing 2030 Build **Analysis Period Number of Approach Lanes: Design Hour** PM Peak Hour Undivided or Divided Highway: Undivided Intersection Control Unsignalized Posted Speed Limit (MPH) Type of Analysis 35 Type of Terrain: Rolling Right Turn Lane Left or Right-Turn Lane Analysis?: **VOLUME CALCULATIONS** Left Turn Lane Volume Calculations Include? Volume % Trucks **PCEV** Movement Left Yes 0.0% N/A **Advancing Volume:** N/A Advancing 0.0% N/A N/A Through **Opposing Volume:** Right Yes 0 0.0% N/A Left Turn Volume: N/A 0 0.0% N/A Left Yes Opposing Through 0.0% N/A 0.0% N/A Right Yes N/A % Left Turns in Advancing Volume: **Right Turn Lane Volume Calculations** Include? Volume % Trucks PCEV Movement Left 0.0% N/A Nο 0 2.0% **Advancing Volume:** Advancing Through 348 359 377 Right 18 0.0% 18 Right Turn Volume: 18 **TURN LANE WARRANT FINDINGS Left Turn Lane Warrant Findings Right Turn Lane Warrant Findings** N/A **Applicable Warrant Figure: Applicable Warrant Figure:** Figure 9 Warrant Met?: N/A Warrant Met?: No **TURN LANE LENGTH CALCULATIONS** Unsignalized Intersection Control: **Design Hour Volume of Turning Lane** Cycles Per Hour (Assumed): 60 Cycles Per Hour (If Known): 0 Average # of Vehicles/Cycle: N/A PennDOT Publication 46, Exhibit 11-6 Speed (MPH) 50-60 25-35 40-45 Type of Traffic Control **Turn Demand Volume** High High Low High Low Low Signalized Α Α B or C B or C B or C B or C Unsignalized B or C Right Turn Lane Storage Length, Condition A N/A Feet **Condition B** N/A Feet N/A **Condition C** Feet N/A Required Right Turn Lane Storage Length: Feet **Additional Findings:** Additional Comments / Justifications:



Volume Data Point

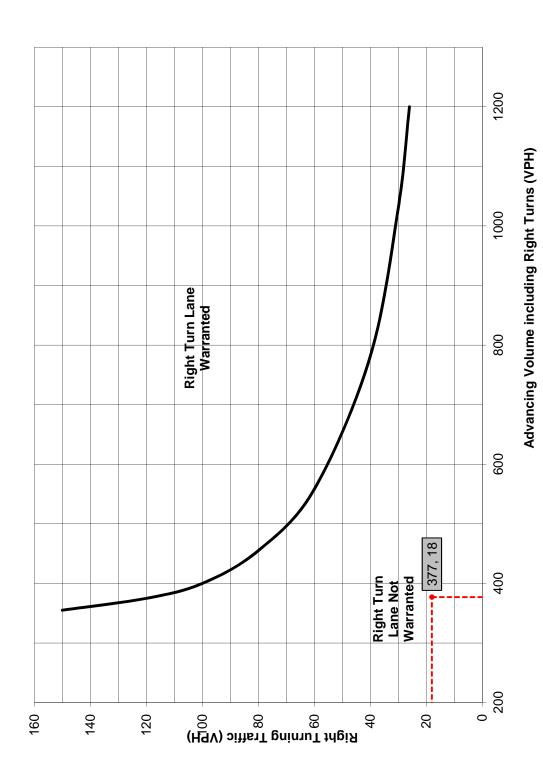


Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

Left Turn Lane 2020 Opening Year Build Scenario

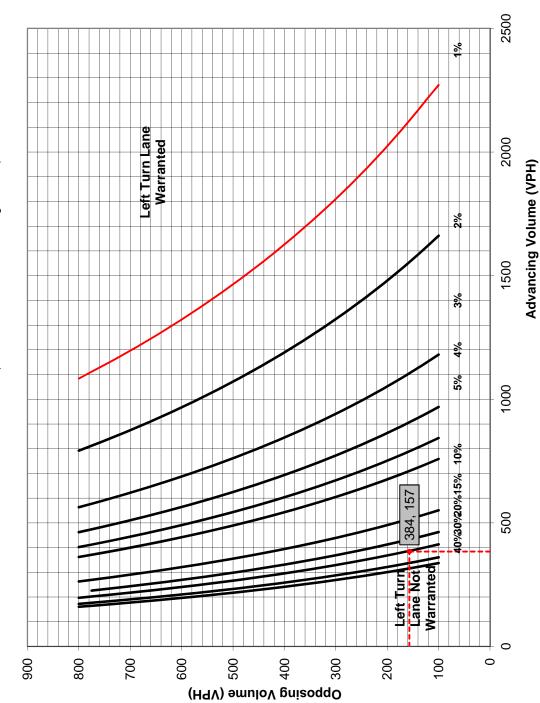
Turn Lane Warrant and Length Analysis Workbook

	Mu	nicipality:	Upper A	llen Twp		Analysis D	Date:	2/28/2	2018
		County:		nd County		Conducted		ME	
PennDO1	Γ Engineerin	g District:		8		Checke			
					Α	gency/Company Na	ame:	ALPHA	A CEI
Intersection & A	pproach De	scription: Prop	osed Site D	riveway 1 / G	ettysburg Pike	e - Northbound Adv	rancing		
	Analys	sis Period:	2020	Build		Number o	f Approacl	n Lanes:	1
		sign Hour:		ak Hour		Undivided or	Divided H	ighway:	Undivided
Dt-	Intersection d Speed Lim			nalized 85					and Ameliania
Poste	•	of Terrain:		lling		Left or Right-Tu	rn Lane Ar		pe of Analysis eft Turn Lane
				VOLUME	CALCUL	ATIONS			
			L	eft Turn Lar	ne Volume C	alculations			
Moveme	ent	Include?	Volume	% Trucks	PCEV]			
	Left	Yes	2	0.0%	2			dvancing Volu	
Advancing	Through	- V	370	2.0%	382			Opposing Volu	
	Right Left	Yes Yes	0	0.0%	0	1		Left Turn Volu	me: 2
Opposing	Through	-	144	4.0%	153				
	Right	Yes	4	0.0%	4	% Left	Turns in A	dvancing Volu	me: 0.52%
			Ri	ght Turn La	ne Volume	Calculations			
Moveme	nt	Include?	Volume	% Trucks	PCEV]			
	Left	No		0.0%	N/A				/.
Advancing	Through Right	-		0.0%	N/A N/A	-		dvancing Volu ight Turn Volu	
	18			1 1 1		1		Sitt Turn Void	
			TUR	IN LANE	WARRAN	T FINDINGS			
L	eft Turn La	ane Warrant F	indings			Right	Turn Laı	ne Warrant F	indings
Applicable	Warrant F	igure: Fig	ure 1	[Applicable Wa	arrant Fig	ure: N	/A
	Warrant	Met?:	No			W	arrant M	et?: N	/A
			TURN	I LANE LE	NGTH CA	LCULATIONS			
	Intersection	n Control:	Unsignalize	ed					
Design Hour Vol	ume of Turr	ning Lane:	2						
-	Per Hour (A		60		_				7
Cycles	Per Hour (I	f Known):	0		Average	# of Vehicles/Cycle	9:	N/A	J
•				PennDOT Pu	blication 46, E				-
•				25-35	Sp	eed (MPH) 40-45	-	0-60	
•	Type	of Traffic Contro	1	23-33	Turn D	emand Volume		0-60	
•	·ype		High	Low	High	Low	High	Low	
	Туре			Α	B or C	B or C	B or C	B or C	
•		Signalized	A				B or C	В	
·		Signalized Jnsignalized	A A	A	С	, b			- -
·				А	С	Length, Conditio	n A:	N/A	Feet
·				А	С			N/A N/A	Feet Feet
•				А	С	Length, Conditio	n B:	N/A	Feet
				Left Turn	C Lane Storage	Length, Conditio Conditio	on B:	N/A N/A	Feet Feet
				Left Turn	C Lane Storage	Length, Conditio	on B: on C: gth:	N/A N/A N/A	Feet Feet Feet
				Left Turn	C Lane Storage	Length, Conditio Conditio	on B: on C: gth:	N/A N/A	Feet Feet Feet



2/28/2018 2020 Build AM NBLT02

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections) (L = % Left Turns in Advancing Volume)



Volume Data Point

—0.5%

Turn Lane Warrant and Length Analysis Workbook

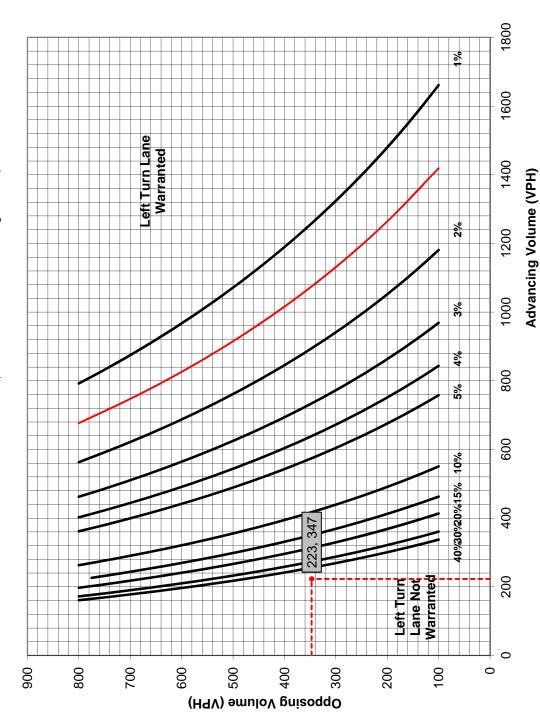
	Mu	nicipality:		Upper A	llen Twp		1	Analysis D	ate:	2/28/	2018	
		County:	C		nd Count	:у		Conducted		_, _, _,		
PennDOT I	Engineerin	g District:		8	3			Checked				
							Ag	ency/Company Na	ime:	ALPH	A CEI	
Intersection & Ap	proach De	scription:	ropos	ed Site Dr	riveway 1	/ Getty	ysburg Pike	- Northbound Adv	ancing			
	Analys	is Period:		2020	Build		1	Number o	f Approach	n Lanes:	1	
	Des	sign Hour:		PM Pea	ak Hour			Undivided or			Undivide	ed
	ntersectio				nalized		_					
Posted	Speed Lim Type o	of Terrain:			5 ling			Left or Right-Tu	rn Lane Ar		pe of Ana eft Turn L	
					VOLU	ME C	ALCULA	TIONS				
				Le	eft Turn	Lane \	Volume Ca	alculations				
Movemen	t	Include?	V	olume	% Truc	ks	PCEV					
	Left	Yes		3	0.0%		3			dvancing Volu		223
Advancing	Through	- Yes		0	2.0% 0.0%		220 0			Opposing Volu Left Turn Volu		347
	Right Left	Yes		0	0.0%		0			Leit Turn volt	inie.	3
Opposing	Through	-		319	2.0%		329					
	Right	Yes		18	0.0%	Š .	18	% Left	Turns in A	dvancing Volu	ıme:	1.35%
				Riç	ght Turn	Lane	Volume C	alculations				
Movemen		Include?	٧	olume	% Truc		PCEV					
Advancing	Left	No -			0.0%		N/A N/A		Δ.	duancina Valu		N/A
Advancing	Through Right	-			0.0%		N/A N/A			dvancing Volu ight Turn Volu		N/A N/A
		I.		TUD	N. I. A. N.	IE 14//		FINDINGS				·
					IN LAIN	IE VV	ARKANI	FINDINGS				
Lef	t Turn La	ne Warrai	nt Find	dings				Right	Turn La	ne Warrant I	Findings	_
Applicable V	Varrant F	igure:	Figur	e 1				Applicable Wa	rrant Fig	ure: N	I/A	
	Warrant	Met?:	No	ס				W	arrant M	et?:	I/A	
				TURN	LANE	LENG	GTH CA	LCULATIONS				
	ntersectio	_	Ur	nsignalize	d							
Design Hour Volu		_		3 60								
-	Per Hour (<i>A</i> Per Hour (I	f Known):		0			Average	# of Vehicles/Cycle	:	N/A		
,	•	· <u>_</u>					_	-		,		
					PennDOT	Publica	ation 46, Ex	ed (MPH)				
	Turne	of Tueffic Co.	-4		25-35			40-45	5	0-60		
	Туре	of Traffic Co	11101				1	mand Volume		1		
		Signalized		High A		A A	High B or C	Low B or C	High B or C	Low B or C	-	
	L	Jnsignalized		A		A	C	В	B or C	В	<u> </u>	
					Loft Tu	ırn I an	o Storago	Langth Canditio	n A.	NI/A	Foot	
					Leit IU	ıı ıı Lan	e storage	Length, Conditio		N/A	Feet	
								Conditio		N/A	Feet	
								Conditio	n C:	N/A	Feet	
					Rec	quired	Left Turn I	Lane Storage Len	gth:	N/A	Feet	
									Δdditic	nal Findings		
									Auditio	mai i iiiaiiiga	٠.	
									Additio	N/A		



2/28/2018 2020 Build PM NBLT02

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections) (L = % Left Turns in Advancing Volume)





Volume Data Point

—1.3%

Left Turn Lane 2030 Opening Year Build Scenario

Turn Lane Warrant and Length Analysis Workbook

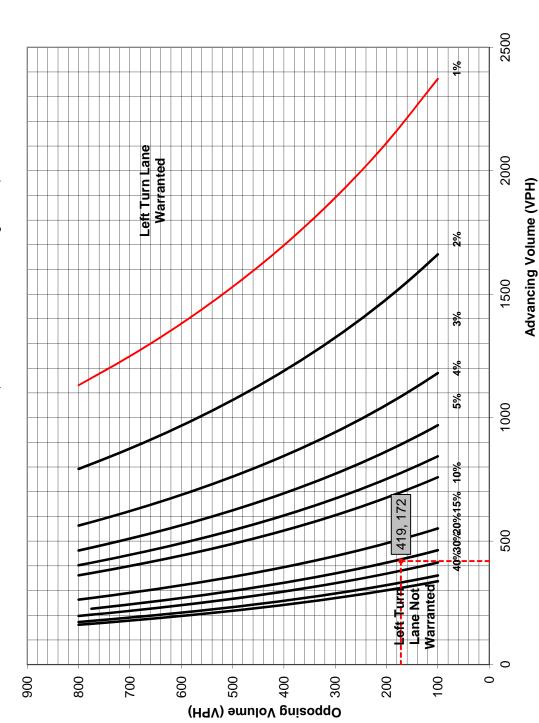
			DI LOCA	ATION AN	_ ,			
	Mur	nicipality:	Upper A	llen Twp		Analysis Da	te: 2/28/	/2018
		County:		nd County		Conducted I		EA
PennDOT	Engineering	g District:	8	8		Checked I	Ву:	
					Age	ency/Company Nan	ne: ALPH	A CEI
Intersection & Ap	proach De	scription: Prop	osed Site Dr	riveway 1 / Ge	ttysburg Pike -	Northbound Adva	ncing	
	Analys	is Period:	2030	Build		Number of A	Approach Lanes:	1
		ign Hour:		ak Hour		Undivided or D	ivided Highway:	Undivided
	Intersection			nalized				
Posted	l Speed Lim Type o	f Terrain:		B5 Iling		Left or Right-Turn		/pe of Analysis Left Turn Lane
				VOLUME	CALCULA			
				eft Turn Lane				
Movemer	nt	Include?	Volume	% Trucks	PCEV			
	Left	Yes	2	0.0%	2		Advancing Vol	ume: 419
Advancing	Through	-	404	2.0%	417		Opposing Volu	
	Right	Yes	0	0.0%	0		Left Turn Vol	ume: 2
	Left	Yes	0	0.0%	0			
Opposing	Through Right	- Yes	158 4	4.0% 0.0%	168 4	% Left Tu	ırns in Advancing Voli	ume: 0.48%
	Mgm	103	•	ght Turn Lan	•		ariis iii Advancing voi	unie. 0.4070
Movemer	nt I	Include?	Volume	% Trucks	PCEV			
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4/9/2018 2030 Build AM NBLT02.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections) (L = % Left Turns in Advancing Volume)





Volume Data Point

-0.5%

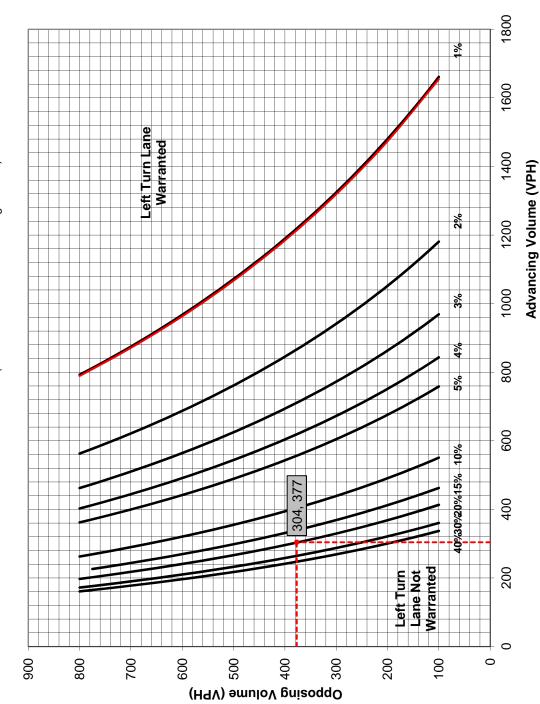
Turn Lane Warrant and Length Analysis Workbook

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4/9/2018 2030 Build PM NBLT02.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections) (L = % Left Turns in Advancing Volume)



Volume Data Point

1.0%

Correspondence

Mark Allen

From:

Mark Allen

Sent:

Tuesday, February 20, 2018 10:10 AM

To: Cc: 'Jennifer Boyer'
'Wheeler, Jason'

Subject:

Arborview TIS Scope

Attachments:

2-7-18 Exhibit 11x17 (1).pdf

Jennifer Boyer

Community Development Director/Planner

Upper Allen Township

Jennifer,

ALPHA Consulting Engineers is preparing an application on behalf of the developer to create 22 single family lots along the northern side of Gettysburg Pike just east of Fisher Road in Upper Allen Township. Please see attached (very preliminary) concept plan. We have prepared a TIS scope (included below) for TPD's/Township Traffic Engineer review, comment, and concurrence. Thank You.

Mark Allen PLS, PE

ALPHA CONSULTING ENGINEERS, INC.

115 LIMEKILN ROAD P.O. BOX 'G' NEW CUMBERLAND, PA. 17070 OFFICE 717-770-2500 FAX 717-770-2400 mallen@alphacei.com

Proposed TIS Scope

A. Study Area

The study area will include the proposed site driveway along Gettysburg Pike, and the adjacent un-signalized intersection of Gettysburg Pike and Fisher Road.

B. Study Periods

The study shall include traffic analysis for the following time periods that occur while school is in session-

- AM Peak Hour of the adjacent street
- PM Peak Hour of the adjacent street

A current year analysis (2018) along with future opening year (2020) analysis will be provided. Future year analysis shall be based on current published PennDOT growth rate values for Cumberland County.

C. Data Collection

Vehicular traffic volume data will be collected while school is in session via:

- Manual Turn Movement Counts at the adjacent un-signalized intersection of Gettysburg
 Pike and Fisher Road during the following time periods to establish peak traffic hours:
 - Weekday AM Peak 6:00AM 9:00AM
 - Weekday PM Peak 3:00PM 7:00PM

D. Trip Generation

Traffic generated by the proposed development shall be estimated per current published Institute of Transportation Engineers (ITE) Trip Generation Manual 10th Edition, Land Use Code 210, Single Family Detached Housing using 22 units as the independent variable.

E. Assignment

Generated traffic shall be assigned to the study area based on the current traffic distribution along Gettysburg Pike

F. Included Analysis

- a) Capacity Analysis per 2010 HCM using Synchro 8, all study area intersections, Substituting Pennsylvania Suburban Context default values for:
- <u>Un- Signalized Intersections</u>:
- Base Critical Headways -
- Base Follow up Headways
- b) Queue Analysis using 95th percentile queues from Synchro methodology, for all intersections.
- c) Turn Lane Warrant Analysis per PennDOT Pub 46, Chapter 11, Site Driveways Only, The lengths of any proposed turn lanes will be sized in accordance with Pub 46, Chapter 11. The need for lengthening any existing turn lanes will be determined in accordance with the Queue Analysis.
- d) Sight Distance Analysis per PA Code 67 CH 441, Site Driveways Only.





DATE: March 19, 2018

TO: Upper Allen Township Planning Commission

Wayne Willey, Chair

FROM: Jennifer M. Boyer, AICP

Community Development Director/Planner

Zachary R. Gulden, MPA Planning Technician

RE: Plan Name: Arborview

Plan Type: Preliminary / Final Subdivision / Land Development

UAT File No.: 18-03-01

Property Parcel ID: 42-29-2456-001 & 001A Property Address: 418 Gettysburg Pike

Zoning District: Medium-Density Residential (R-2)

The Applicant's proposal is to subdivide Lots 1, 2, and 3 into 26 separate Lots. The proposed project is for the development of 22 single-family detached homes, which will be located on Lots 3 through 24. Two private open spaces lots will be provided within the development. The existing single-family homes on Lots 1 and 2 will remain. The total tract acreage is 14.8, with 12.7 acres being developed. The development will create an additional 1,307 linear feet of new public streets on Arborview Drive and Coventry Drive. The development will be served with public water and sewer. The proposed use of the subject property is consistent with the Upper Allen Township Zoning Ordinance and Comprehensive Plan.

The Applicant is requesting the following deferrals:

1. Defer the requirements of Section 220-15.B (11) to reconstruct existing streets abutting the subdivision / land development to the widths specified in the Township of Upper Allen Subdivision and land Development Ordinance.

Staff Comment: The existing ROW on Gettysburg Pike is 36 feet with a 23 foot-wide cartway. Collector roads are required to be 60 feet wide with 24 foot-wide cartways. Given the additional ROW from the Applicant, as well as dedicated ROW from residents on the other side, Gettysburg Pike will increase to 60 feet of ROW. The existing cartway would remain. Staff could support the deferral of the road widening until such time as deemed necessary.

Arborview P/F Sub/LD UAT File No. 18-03-01 PC Meeting: March 26, 2018

shade trees shall be added to the plan in accordance with Section 220-26.B(1) of the Codified Ordinances of Upper Allen Township.

3. Section 220-10.B(2)(c) of codified ordinances requires pipe sizes and location of valves to be shown for the proposed water distribution system.

TRAFFIC IMPACT STUDY

TPD concurs with the recommendations and conclusions outlined in the TIS submission; it should be noted that the above comments are not anticipated to impact the recommendations or conclusions in the study and the Applicant and/or Township may consider a waiver of the 10-year horizon analysis.

- 4. Future traffic projections should consider a ten-year growth period beyond the construction of the proposed development. Therefore, assuming an opening year of 2020, the future analysis year should be 2030. If the Applicant is not proposing to analyze a 10-year horizon, a modification may be discussed with the Township, in accordance with Section 220-11.F(2)(c)(2).
- 5. There appears to be typographical errors on page 15 in reference to total trips generated by the site. In addition, Figures 4 (trip distribution %'s) and 5A must be reviewed and verified for consistency with the volume development worksheet.
- 6. The minimum length of a vertical curve shall be 100 feet in accordance with Section 220-15.D(2)(c). While the curves less than 100 feet meet the required K values, the length should be increased or else a modification to this section should be requested.
- 7. All proposed public areas should be designed in accordance with applicable federal and state standards. Plans should be constructed to comply with the following standards in accordance with Section 220-16.B(1):
 - a. PennDOT Design Manual 2, Chapter 6
 - b. PennDOT Standards for Roadway Construction, Publication 72M, RC-67M.
 - c. U.S. Access Board, Public Right of Way Accessibility Guidelines (PROWAG) and ADA Accessibility Guidelines for Buildings and Facilities (ADAAG).
- 8. A crossing/ADA ramp shall be provided on the Coventry Drive approach to Arborview Drive.
- 9. A detail for an Alternate 4A Curb Ramp was provided on Sheet 12 of 15. The Applicant should verify whether or not this detail is appropriate for each of the proposed curb ramps. In addition, the proposed curb ramp at the Gettysburg Pike intersection should be realigned to provide a crossing of Arborview Drive that runs parallel to the Gettysburg Pike.



April 9, 2018

Jennifer M. Boyer, AICP, Community Development Director/Planner Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

RE: Arborview Preliminary / Final Subdivision & Land Development Plan Your File No. 18-03-01

Dear Jennifer,

Please find below in **bold type** our responses to the written review comments received for the above referenced project.

Township Memo from Jennifer Boyer and Zachary Gulden, dated March 19, 2018

1. Section 220-15.E(5) states that intersections along collector streets shall be at least 800 feet apart. The proposed intersection of Arborview Drive and Fisher Road are less than 600 feet apart. Since a modification of this section has not been requested, please describe why the new road cannot be positioned at the 800-foot distance. Is there any way to align Arborview Drive with the proposed intersection north of the site for the proposed Terraces at Shepherdstown development?

A modification is now requested; please see the enclosed Township form for more information

2. Section 220-28.C states that properties within 1,000 feet of a municipal historic district may be subject to additional requirements/restrictions, including buffer zones and screening, as may be imposed by the governing body. The Applicant should discuss how the new development will not negatively impact the continued protection/preservation of the Shepherdstown Historic District.

All proposed construction is located outside of the historic district. All proposed development is located well downhill of the historic district, so it will not visually or spatially be part of the village cluster. There are existing trees to remain that will shield proposed lot #2.

SUBDIVISION, LAND DEVELOPMENT & ZONING

 Gettysburg Pike is a collector roadway. The cartway edge at the intersection with Gettysburg Pike shall be 50 feet in accordance with Section 220-15.E(7) of the Codified Ordinances of Upper Allen Township.

The curb radii have been increased to 50 feet; please see sheet #3.

2. The Stormwater Management Buffer and Screening information on Sheet 6 indicates that a type 3 buffer yard requires 1 shade tree per 100 linear feet. The requirement for shade trees in a type 3 buffer yard is 1 tree per 30 linear feet; therefore, additional shade trees shall be added to the plan in accordance with Section 220-26.B(1) of the Codified Ordinances of Upper Allen Township.

Upper Allen Township April 9, 2018 Page 2

On sheet #5, the text has been changed from 100 feet to 30 feet, calculations have been revised, and additional trees are now shown.

3. Section 220-10.B(2)(c) of codified ordinances requires pipe sizes and location of valves to be shown for the proposed water distribution system.

Water main pipe sizes and valves are now shown on sheet #4.

TRAFFIC IMPACT STUDY

TPD concurs with the recommendations and conclusions outlined in the TIS submission; it should be noted that the above comments are not anticipated to impact the recommendations or conclusions in the study and the Applicant and/or Township may consider a waiver of the 10- year horizon analysis.

4. Future traffic projections should consider a ten-year growth period beyond the construction of the proposed development. Therefore, assuming an opening year of 2020, the future analysis year should be 2030. If the Applicant is not proposing to analyze a 10-year horizon, a modification may be discussed with the Township, in accordance with Section 220-11.F(2)(c)(2).

A revised study is enclosed with the ten-year growth projections.

5. There appears to be typographical errors on page 15 in reference to total trips generated by the site. In addition, Figures 4 (trip distribution %'s) and 5A must be reviewed and verified for consistency with the volume development worksheet.

The errors have been corrected in the enclosed revised study.

6. The minimum length of a vertical curve shall be 100 feet in accordance with Section 220-15.D(2)(c). While the curves less than 100 feet meet the required K values, the length should be increased or else a modification to this section should be requested.

A modification is now requested with this resubmission; please see the enclosed Township form.

- 7. All proposed public areas should be designed in accordance with applicable federal and state standards. Plans should be constructed to comply with the following standards in accordance with Section 220-16.B(1):
- a. PennDOT Design Manual 2, Chapter 6
- b. PennDOT Standards for Roadway Construction, Publication 721M, RC-67M.
- c. U.S. Access Board, Public Right of Way Accessibility Guidelines (PROWAG) and ADA Accessibility Guidelines for Buildings and Facilities (ADAAG).

ADA ramps are provided at locations that meet the above criteria.

Upper Allen Township April 9, 2018 Page 13

This concludes our responses. Please contact me or Tom Scully with any further comments. Thank you.

Very Truly Yours,

John K. Murphy, P.E., P.L.S.

