

2509 Mill Road Townhomes *Upper Allen Township, Cumberland County, PA*

For Submission To: *Upper Allen Township*

2509 MILL ROAD TOWNHOMES TRANSPORTATION IMPACT STUDY

FOR SUBMISSION TO:

Upper Allen Township, Cumberland County, PA

Prepared For:

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2509 Mill Road

Mechanicsburg, PA 17055

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EXECUTIVE SUMMARY

The purpose of this study is to examine the potential traffic impact associated with the proposed 2509 Mill Road Townhomes on the roadway network in Upper Allen Township, Cumberland County, PA. Based on this evaluation, the following conclusions were reached:

- 1. The study area intersections included in this TIS are as follows:
 - » West Lisburn Road (SR 2004) & Mill Road;
 - » Mill Road & Proposed Site Driveway;
 - Mill Road & Wingert Drive;
 - » Mill Road & Sholly Drive.
- 2. The project site is located on the eastern side of Mill Road, between West Lisburn Road (SR 2004) and Wingert Drive. The proposed site will consist of 14 single-family attached dwelling units.
- 3. Access to the site is proposed via one (1) full-movement driveway to Mill Road. The current driveway serving the property will be removed in conjunction with construction of the proposed development.
- 4. Available sight distances at the proposed driveway exceed applicable PennDOT safe stopping distance and desirable stopping distance criteria.
- 5. Upon full build-out, the proposed development is expected to generate 2 new vehicle-trips during the weekday A.M. peak hour and 4 new vehicle-trips during the weekday P.M. peak hour.
- 6. Under the 2026 and 2036 projected conditions, all approaches and turning movements at the site driveway intersections with the external roadway network will operate at LOS A or better during weekday A.M. and P.M. peak hours.
- 7. All overall intersection levels of service (ILOS) will operate at an acceptable <u>ILOS C or better</u> during the 2026 and 2036 projected condition scenarios. All levels of service at the study area intersection comply with the requirements outlined in PennDOT's TIS Guidelines and Upper Allen Township SALDO standards.
- 8. Traffic Planning and Design Inc. (TPD) recommends the following roadway improvements as outlined at the study area intersections:

Mill Road & Proposed Site Driveway

- » Design the site access as a full-movement with one ingress and one egress lane.
- » Install a "Stop" sign, R1-1, 30"x30" on the egress access approach;
- » Provide and perpetually maintain required sight distances.

As part of the Township's land development process, the applicant will coordinate and fund the implementation of the recommended roadway improvements. The improvements will be designed and constructed in compliance with ADA requirements to the extent feasible where applicable, unless otherwise directed or approved by the Township.

Levels of Service (LOS) for the study area intersections have been summarized in matrix form. Table I
details the overall intersection LOS for each study area intersection.

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TABLE I

OVERALL INTERSECTION LEVEL OF SERVICE SUMMARY (DELAY IN SECONDS)

Intersection	Peak	Eviction	Opening Year 2026			Design Year 2036			Meets LOS
intersection	Hour	Existing	Base	Projected	Projected ¹	Base	Projected	Projected ¹	Requirements?
West Lisburn Road	AM	B (11.4)	B (11.7)	B (11.8)		B (12.4)	B (12.4)		Yes
(SR 2004) & Mill Road	PM	C (15.8)	C (16.5)	C (16.7)		C (19.0)	C (19.2)		Yes
Mill Road & Proposed Site Driveway	AM	A (0.1)	A (0.1)	A (0.2)		A (0.1)	A (0.2)		Yes
	PM	A (0.0)	A (0.0)	A (0.1)		A (0.0)	A (0.1)		Yes
Adill Dood St Minner Drive	AM	A (0.1)	A (0.1)	A (0.1)	-	A (0.1)	A (0.1)		Yes
Mill Road & Wingert Drive	PM	A (0.1)	A (0.1)	A (0.1)	-1	A (0.1)	A (0.1)		Yes
	AM	A (1.1)	A (1.1)	A (1.1)		A (1.2)	A (1.2)		Yes
Mill Road & Sholly Drive	PM	A (0.3)	A (0.3)	A (0.3)		A (0.3)	A (0.3)		Yes

Base = No-Build scenario Projected = Build scenario

Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

1 = Projected conditions with implementation of recommended improvements

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INTRODUCTION

Traffic Planning and Design, Inc. (TPD) has completed a Transportation Impact Study (TIS) for the proposed 2509 Mill Road Townhomes in Upper Allen Township, Cumberland County, Pennsylvania. The project site is located on the eastern side of Mill Road, between West Lisburn Road (SR 2004) and Wingert Drive, as shown in **Figure 1**. The land use context of the site and surrounding area is defined as Suburban Neighborhood in PennDOT's Publication 10X (DM-1X), Appendix B. As shown in **Figure 2**, the proposed site will consist of fourteen (14) single-family attached dwelling units.

This report has been prepared in accordance with PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's Publication 282, Appendix A, dated September 2022.

Site Access Locations

Access to the site is proposed via one (1) full-movement driveway to Mill Road. The current driveway serving the property will be removed in conjunction with construction of the proposed development.

EXISTING ROADWAY NETWORK

A field review of the existing roadway system in the study area was conducted. The existing roadway characteristics within the study area are summarized in **Table 1**. Study area photographs are provided in **Appendix A**.

TABLE 1
ROADWAY CHARACTERISTICS WITHIN STUDY AREA

Roadway	Ownership	Functional Classification/ Roadway Type	Predominant Directional Orientation	Average Daily Traffic ¹	Posted Speed Limit
West Lisburn Road	State (SR 2004)	Minor Arterial	East/West	3,571	35 mph
Mill Road	Township	Local Road	North/South		25 mph
Wingert Drive	Township	Local Road	East/West		25 mph
Sholly Drive	Township	Local Road	East/West		25 mph

¹Obtained from PennDOT TIRe website

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TABLE 2
EXISTING INTERSECTION CONTROLS, LANE WIDTHS, SHOULDER WIDTHS, AND APPROACH GRADES

Intersection	Control	Lane Configuration	Lane Width	Shoulder Width	Approach Grade
		EB L/T/R	11′	0'	-1%
	Cton	WB L/T/R	10′	4'	2%
West Lisburn Road (SR 2004) & Mill Road	Stop	NB L/T/R	11′	0'	3%
(3K 2004) & IVIIII KOdu		SB L/T	11′	- 1	20/
	Yield	SB R	11′	2'	-2%
NATION AND ADDRESS OF THE PROPERTY OF THE PROP	Stop	WB L/R	10′	0'	6%
Mill Road & Proposed Site Driveway	Free	NB T/R	11′	0'	3%
Driveway		SB L/T	11′	0'	-3%
	Class	EB L/T/R	12′	0'	1%
AA'II Daad QuAA' aaad Da'	Stop	WB L/T/R	13′	0'	-3%
Mill Road & Wingert Drive	F	NB L/T/R	11′	0'	6%
	Free	SB L/T/R	12′	0'	-4%
	Stop	EB L/R	16′	0'	-1%
Mill Road & Sholly Drive	F	NB L/T	11′	0'	-1%
	Free	SB T/R	11′	0'	1%

Land Use Context

In Publication 10X (Design Manual Part IX, Appendix B), there is guidance pertaining to defining the land use context(s) for a given area. Based upon review of this information, the land uses surrounding the proposed site best fits the **Suburban Neighborhood** designation, as described below:

Suburban Neighborhood, "Predominantly low-density residential communities with houses typically arranged along a curvilinear system of streets with limited connectivity to regional road networks. Neighborhoods can include community facilities (schools, churches, recreation) and some small businesses or offices."

The project site and adjacent area is zoned R-2 (Medium Density Residential); the majority of existing adjacent uses are residential in nature.

Roadway Type

In PennDOT Publication 10X (DM-1X), Appendix B, there is guidance pertaining to defining the transportation context(s) for a given area. Comparing the existing condition roadway characteristics to the various options presented in Publication 10X, the study area roadways best fit the following categories, as described below:

Community Arterial, traffic volumes of 5,000 to 25,000 vehicles per day, intersection spacing of 300 to 1,320 feet, a desired operating speed of 25-55 mph, and a description as follows: "often classified as Minor Arterial in traditional classification but may include road segments classified as Principal Arterial."

» West Lisburn Road (SR 2004).

Local Road, traffic volumes of <3,000 vehicles per day, intersection spacing of 000 to 660 feet, a desired operating speed of 20-30 mph.

- » Mill Road;
- » Wingert Drive;
- » Sholly Drive.

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Bicycle and Pedestrian Facilities

Based on observations during field visits at the study area intersections, paved shoulders currently accommodate pedestrian and bicycle traffic in the vicinity of the proposed development. Sidewalks will be provided throughout the internal roadways on-site and along the site frontage of Mill Road. The proposed development will not adversely impact any existing bicycle or pedestrian facility.

Upon review of the Mechanicsburg Area School District (MASH) current school bus routes, stops occur within the study area along Mill Road at 2500 Mill Road, 2509 Mill Road (Proposed Site Location), Mill Road/Sholly Road and Mill Road/Wingert Drive. The Mill Road corridor does not currently have sidewalks; however, the proposed development will be providing sidewalks along the Mill Road site frontage and throughout the site to accommodate students. The sidewalks will provide an adequate refuge area for students to be picked up/dropped off at the 2509 Mill Road driveway intersection.

Mass Transit Facilities

Mass transit is not available in the vicinity of the proposed development.

EXISTING TRAFFIC CONDITIONS

Manual Turning Movement Counts

Manual traffic counts were conducted on 15-minute intervals during the weekday morning (6:00 to 9:00 A.M.) and weekday evening (3:00 to 6:00 P.M.) peak periods. Data pertaining to heavy vehicles, pedestrians and transit vehicles were observed during the manual counts. Peak hours and count dates for the study area intersections are identified in **Table 3**.

TABLE 3
MANUAL TRAFFIC COUNT INFORMATION

Intersection	Date of Traffic Counts	Time Period	Intersection Peak Hour ¹
West Lisburn Road		Weekday A.M.	7:15 to 8:15 A.M.
(SR 2004) & Mill Road Mill Road & Proposed Site Driveway		Weekday P.M.	4:45 to 5:45 P.M.
	Thursday, May 4, 2023	Weekday A.M.	7:15 to 8:15 A.M.
		Weekday P.M.	4:45 to 5:45 P.M.
Mill Dood 9: Wingout Drive		Weekday A.M.	7:15 to 8:15 A.M.
Mill Road & Wingert Drive		Weekday P.M.	4:45 to 5:45 P.M.
Mill David Or Chall Dir		Weekday A.M.	7:15 to 8:15 A.M.
Mill Road & Sholly Drive		Weekday P.M.	4:45 to 5:45 P.M.

Peak Hour consists of the four consecutive 15-minute intervals where the highest traffic volumes occur.

Existing condition traffic volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 3**. Traffic count data sheets are provided in **Appendix B**.

Average Daily Traffic

The existing Average Daily Traffic (ADT) volumes are based data obtained from PennDOT's Traffic Information Repository (TIRe) website. The traffic volume map contained on the PennDOT TIRe website was

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reviewed in <u>June 2023</u> to determine the Average Daily Traffic (ADT) for a typical weekday along the Statemaintained roadways in the vicinity of the proposed site.

BASE (NO-BUILD) CONDITIONS

Annual Background Growth

A background growth factor for the roadways in the study area was developed based on growth factors for August 2022 to July 2023 obtained from the PennDOT Bureau of Planning and Research (BPR). The PennDOT BPR suggests using a background growth trend factor of 0.59% per year in Cumberland County for urban non-interstate roadways. As such, the background growth factor was applied annually to the existing volumes to determine the 2026 and 2036 base condition traffic volumes.

Nearby Proposed Developments

Base (no-build) traffic conditions were calculated to include traffic volumes from proposed developments, which, though not operating under existing conditions, may be operating by the opening year (2026) and design year (2036) of the proposed development. The following nearby planned developments were specifically included in this study:

151 – 237 Gettysburg Pike Development is a proposed development located in the northeast and southeast quadrant of South Market Street (SR 0114) & Gettysburg Pike. Trip distributions for this development were developed based on data provided by the township which was completed by *Alpha Consulting Engineers Inc.* The distribution of this development is illustrated in **Figure 4.**

The additional traffic volumes due to background growth and background developments were added to the existing traffic data to produce 2026 and 2036 base (no-build) condition traffic volumes. Base condition volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 5** for the 2026 opening year conditions and **Figure 6** for the 2036 design year conditions. Background developments are provided in **Appendix D.**

SCHEDULED ROADWAY IMPROVEMENTS

Programmed Improvements

Based on a review of the Pennsylvania Transportation Improvement Program (TIP) there are no programmed roadway improvements in the vicinity of the proposed site.

PROPOSED SITE ACCESS

Access to the site is proposed via one (1) full-movement driveway to Mill Road. The current driveway serving the property will be removed in conjunction with construction of the proposed development.

Sight Distance Analysis

In general, recommended safe sight distances depend upon the posted speed limit and roadway grades. Desirable sight distances as noted in PennDOT's Chapter 441 were evaluated at each access location. In addition, measured sight distances were compared to PennDOT's safe stopping sight distance standard, which is calculated by the following equation:

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$SSSD = 1.47VT + V^2/[30(f\pm g)]$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Vehicle Speed

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements

g = Percent of Roadway Grade Divided by 100

Table 4 show the measured, desirable (DES), acceptable (SSSD), and required sight distances at the site driveways for vehicles entering and exiting the site.

TABLE 4
SIGHT DISTANCE ANALYSIS
SITE DRIVEWAY TO MILL ROAD

	N	5		Sight Distances (feet)			
	Direction	Speed	Grade ¹	DES	SSSD	EXIST	
Exiting	To the left	25 mph	3%	250	143	300+	
Movements	To the right	25 mph	-3%	195	151	300+	
Entering Left	Approaching same direction	25 mph	-3%		151	300+	
Turns	Approaching opposite direction	25 mph	3%	190	143	300+	

DES = PennDOT Desirable Sight Distance SSSD = PennDOT Acceptable Sight Distance 1 = Roadway Grade Approaching Driveway EXIST = Existing (measured) Sight Distance

As shown in **Table 4** above, the measured sight distances at the site driveways exceed applicable DES and SSSD criteria. Sight distances were evaluated at the proposed site driveway intersection as applicable per Township Ordinance requirements (PennDOT's SSSD) and also compared to PennDOT's Desirable sight distance requirements, which are more stringent than SSSD for passenger vehicles and single-unit trucks (school bus).

TRIP GENERATION

The trip generation rates for the proposed development were obtained from the manual *Trip Generation*, 11th Edition, an Institute of Transportation Engineers (ITE) Informational Report. The statistics in *Trip Generation* are empirical data based on more than 4,800 trip generation studies. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

For the proposed residential development, Land Use Code 215 (Single-Family Attached Housing) from *Trip Generation* was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour. **Table 5** shows the rates/equations and directional percentages for the analyzed time periods.

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TABLE 5
ITE TRIP GENERATION DATA

Land Use	ITE#	Time Period	Equations/Rates	Entering %	Exiting %
Single-Family		Average Weekday	T = 7.62(X) - 50.48	50%	50%
Attached	215	Weekday A.M. Peak Hour	T = 0.52(X) - 5.70	25%	75%
Housing		Weekday P.M. Peak Hour	T = 0.60(X) - 3.93	59%	41%

T = number of site-generated vehicular trips X = independent variable (dwelling units)

The calculated trip generation for the proposed development is shown in **Table 6**.

TABLE 6
TRIP GENERATION SUMMARY

Time Desired	2509 Mill Road Townhomes					
Time Period	Total	Enter	Exit			
Average Weekday	56	28	28			
A.M. Peak Hour	2	0	2			
P.M. Peak Hour	4	3	1			

Based on the trip generation analysis summarized in **Table 6**, the proposed development will generate approximately 2 new trips during the weekday A.M. peak hour, and 4 new trips during the weekday P.M. peak hour at full build-out.

TRIP DISTRIBUTION

The distribution of trips generated by the proposed development was based on the local road network, the existing traffic patterns, and the proposed use of the site. The new trips for the proposed development were distributed to the local roadway network based on the percentages shown in **Table 7**. The distribution of site-generated trips are shown in **Figure 7**.

TABLE 7
TRIP DISTRIBUTION PERCENTAGES

Assignment (To/From)	via	Distribution Percentage
To/From East	West Lisburn Road (SR 2004)	35%
To/From West	West Lisburn Road (SR 2004)	50%
To/From North	Mill Road	4%
To/From South	Mill Road	11%

The assignment of site-generated trips for the proposed development during the weekday A.M. and P.M. peak hours are shown in **Figure 8**.

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PROJECTED (BUILD) CONDITION TRAFFIC VOLUMES

The site-generated trips for the proposed development were added to the 2026 and 2036 base (no-build) condition traffic volumes to develop 2026 and 2036 projected (build) condition traffic volumes.

Projected condition traffic volumes for the opening year of 2026 for the weekday A.M. and P.M. peak hours are shown in **Figure 9**. Projected condition traffic volumes for the design year of 2036 for the weekday A.M. and P.M. peak hours are shown in **Figure 10**. Traffic volume development worksheets are contained in **Appendix C.**

LEVELS OF SERVICE FOR AN INTERSECTION

For analysis of intersections, level of service is defined in terms of delay, which is a measure of driver discomfort and frustration, fuel consumption, and lost travel time. LOS criteria is stated in terms of control delay per vehicle for a one-hour analysis period. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The criteria are shown in **Table 8**. Delay, as it relates to level of service, is a complex measure and is dependent upon a number of variables. For signalized intersections, these variables include the quality of vehicle progression, the cycle length, the green time ratio, and the volume/capacity ratio for the lane group in question. For unsignalized intersections, delay is related to the availability of gaps in the flow of traffic on the major street and the driver's discretion in selecting an appropriate gap for a particular movement from the minor street (straight across, left or right turn).

TABLE 8

LEVEL OF SERVICE CRITERIA

UNSIGNALIZED AND SIGNALIZED INTERSECTIONS¹

Level of Service	Control Delay Per Vehicle (Seconds)					
Level of Service	Signalized	Unsignalized				
А	< 10	< 10				
В	> 10 and < 20	> 10 and < 15				
С	> 20 and < 35	> 15 and < 25				
D	> 35 and < 55	> 25 and < 35				
E	> 55 and < 80	> 35 and < 50				
F	> 80 or v/c > 1.0	> 50 or v/c > 1.0				

¹Obtained from Exhibits 19-8 and 20-2 of the Transportation Research Board's Highway Capacity Manual 6th Edition

CAPACITY ANALYSIS METHODOLOGY

Capacity analyses were conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the *Highway Capacity Manual* (HCM) 6th Edition using *Synchro 11* software, a Trafficware product.

The following conditions were analyzed, as applicable:

- » Existing conditions;
- » 2026 Base conditions (Build-out year without development);
- » 2026 Projected conditions (Build-out year with development);

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- » 2036 Base conditions (Design year without development);
- » 2036 Projected conditions (Design year with development).

The following items should be noted with respect to the capacity analyses:

The Pennsylvania default values for the suburban land use context contained in Chapter 10 of PennDOT's Publication 46 were utilized for the base critical headway and base follow-up headways at unsignalized intersections.

In addition, capacity analyses were conducted at the proposed site driveway intersections under the 2026 and 2036 projected conditions. The capacity analysis worksheets are included in **Appendix E**.

PennDOT's Transportation Impact Study Guidelines outlined in PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's Publication 282, Appendix A, dated September 2022 contain the following criteria regarding levels of service:

- That if evaluation of the With Development Horizon Year Scenario to the Without Development Horizon Year Scenario indicates that the overall intersection level of service has dropped, the applicant will be required to mitigate the level of service if the increase in overall intersection delay is greater than 10-seconds. If the overall intersection delay increase is less than or equal to 10-seconds, mitigation of the intersection will not be required.
- » That for mitigation scenarios, applicants are expected to mitigate the overall intersection LOS to the original Without Development LOS; the 10-second delay variance is not applied to mitigation scenarios. Applicants may be required to address available storage and queue lengths at critical movements or approaches even if the overall LOS requirements are met.
- » The Guidelines state that if signalization is the preferred alternative for mitigation, overall intersection LOS C in rural areas and LOS D in urban areas is acceptable.
- The Guidelines states new signalized or unsignalized intersection established to serve as access to the development shall be designed to operate at minimum LOS C for rural areas, and minimum LOS D for urban areas.

LEVELS OF SERVICE IN THE STUDY AREA

Level of service (LOS) matrices for the study area intersections are shown in **Tables 9 and 10** for the weekday A.M. and weekday P.M. peak hours.

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TABLE 9
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY

				Weekd	lay A.M. Pea	ak Hour		
Intersection	Movement	Existing	Ор	ening Yea	r 2026	D	esign Year	2036
		Condition	Base	Projected	Projected ¹	Base	Projected	Projected ¹
	EB L/T/R	В	В	В		В	В	
	WB L/T/R	В	В	В		В	В	
West Lisburn Road	NB L/T/R	В	В	В		В	В	
(SR 2004) & Mill Road	SB L/T	В	В	В		В	В	
	SB R	Α	Α	Α		Α	Α	
	ILOS	B (11.4)	B (11.7)	B (11.8)		B (12.4)	B (12.4)	
MATH Door of Or	WB L/R	Α	Α	Α		Α	Α	
Mill Road &	SB L/T	Α	Α	Α		Α	Α	
Proposed Site Driveway	ILOS	A (0.1)	A (0.1)	A (0.2)		A (0.1)	A (0.2)	
	EB L/T/R	Α	Α	Α		Α	Α	
	WB L/T/R	Α	Α	Α		Α	Α	
Mill Road & Wingert Drive	NB L/T/R	Α	Α	Α		Α	Α	
	SB L/T/R	Α	Α	Α		Α	Α	
	ILOS	A (0.1)	A (0.1)	A (0.1)		A (0.1)	A (0.1)	
Mill Road & Sholly Drive	EB L/R	Α	Α	Α		Α	Α	
	NB L/T	Α	Α	Α		Α	Α	
	ILOS	A (1.1)	A (1.1)	A (1.1)		A (1.2)	A (1.2)	

Base = No-Build scenario;

Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

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¹= Projected conditions with implementation of recommended improvements

TABLE 10
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY

		Weekday P.M. Peak Hour						
Intersection	Movement	Existing	Opening Year 2026			Design Year 2036		
		Condition	Base	Projected	Projected ¹	Base	Projected	Projected ¹
West Lisburn Road (SR 2004) & Mill Road	EB L/T/R	С	C	C		C	C	
	WB L/T/R	С	C	C		C	C	
	NB L/T/R	В	В	В		В	В	
	SB L/T/R	В	В	В		В	В	
	SB R	Α	Α	Α		В	В	
	ILOS	C (15.8)	C (16.5)	C (16.7)		C (19.0)	C (19.2)	
Mill Road & Private Driveway/Proposed Site Driveway	WB L/R	Α	Α	Α		Α	Α	
	SB L/T	Α	Α	Α		Α	Α	
	ILOS	A (0.0)	A (0.0)	A (0.1)		A (0.0)	A (0.1)	
Mill Road & Wingert Drive	EB L/T/R	Α	Α	Α		Α	Α	
	WB L/T/R	Α	Α	Α		Α	Α	
	NB L/T/R	Α	Α	Α		Α	Α	
	SB L/T/R	Α	Α	Α		Α	Α	
	ILOS	A (0.1)	A (0.1)	A (0.1)		A (0.1)	A (0.1)	
Mill Road & Sholly Drive	EB L/R	Α	Α	Α		Α	Α	
	NB L/T	Α	Α	Α		Α	Α	
	ILOS	A (0.3)	A (0.3)	A (0.3)		A (0.3)	A (0.3)	

Base = No-Build scenario;

Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

¹= Projected conditions with implementation of recommended improvements

As shown in **Tables 9 and 10** under 2026 and 2036 projected conditions with the development of the proposed site, the study area intersections will operate at the same overall intersection level of service (ILOS) as under 2026 and 2036 base conditions, during the weekday A.M. and P.M. peak hours.

All approaches and turning movements at the site driveway intersections will operate at **LOS A or better** under 2026 and 2036 Projected Conditions during the weekday A.M. and P.M. peak hours.

All levels of service at the study area intersection comply with the requirement outlined in PennDOT's TIS Guidelines and Upper Allen Township standards.

95TH PERCENTILE QUEUE ANALYSIS

Queue analyses were conducted at the study area intersections using *Synchro 11* software and reporting the HCM 6th Edition 95th percentile queue lengths. The queue analysis results are summarized in **Table 11** for the analyzed peak hours.

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TABLE 11
95TH PERCENTILE QUEUE ANALYSIS

		203	36 Base Cond	itions	2036 Projected Conditions			
Intersection	Lane Group	Existing Storage	95th Percentile Queue Length (ft)		Proposed Storage	95th Percentile Queue Length (ft)		
		Length ¹	A.M.	P.M.	Length	A.M.	P.M.	
West Lisburn Road (SR 2004) & Mill Road	EB L/T/R	400	73	153	Same	73	155	
	WB L/T/R	500+	58	150	Same	58	153	
	NB L/T/R	100	18	30	Same	18	30	
	SB L/T	250	0	0	Same	0	0	
	SB R	175	8	8	Same	8	8	
Mill Road & Proposed Site Driveway	WB L/R	200	0	0	Same	0	0	
	SB L/T	175	0	0	Same	0	0	
Mill Road & Wingert Drive	EB L/T/R	75	0	0	Same	0	0	
	WB L/T/R	350	0	0	Same	0	0	
	NB L/T/R	275	0	0	Same	0	0	
	SB L/T/R	125	0	0	Same	0	0	
Mill Road & Sholly Drive	EB L/R	150	3	3	Same	3	3	
	NB L/T	500+	0	0	Same	0	0	

^{1 =} Existing Storage Length determined by the nearest public intersection.

As shown in **Table 11**, adequate queue storage will be provided for the turn lanes in 2036 with construction and full build-out of the proposed development. Queue analysis worksheets are included with the capacity analysis worksheets provided in **Appendix E**.

AUXILIARY TURN LANE ANALYSIS

Methodology

TPD evaluated auxiliary turn lane warrants at the site access intersections. The warrant analysis methodology contained within Chapter 11 of PennDOT's *Publication 46*, Section 11.17 was utilized for this evaluation.

Findings

Table 12 summarizes the results of the auxiliary turn lane analysis at the site access intersections.

TABLE 12
AUXILIARY TURN LANE ANALYSIS SUMMARY

Intersection	Auxiliary Lane	Warrant Satisfied?	Required Lane Length	Proposed Lane Length
Mill Road & Proposed Site Driveway	SB Left-Turn Lane	No		
	NB Right-Turn Lane	No		

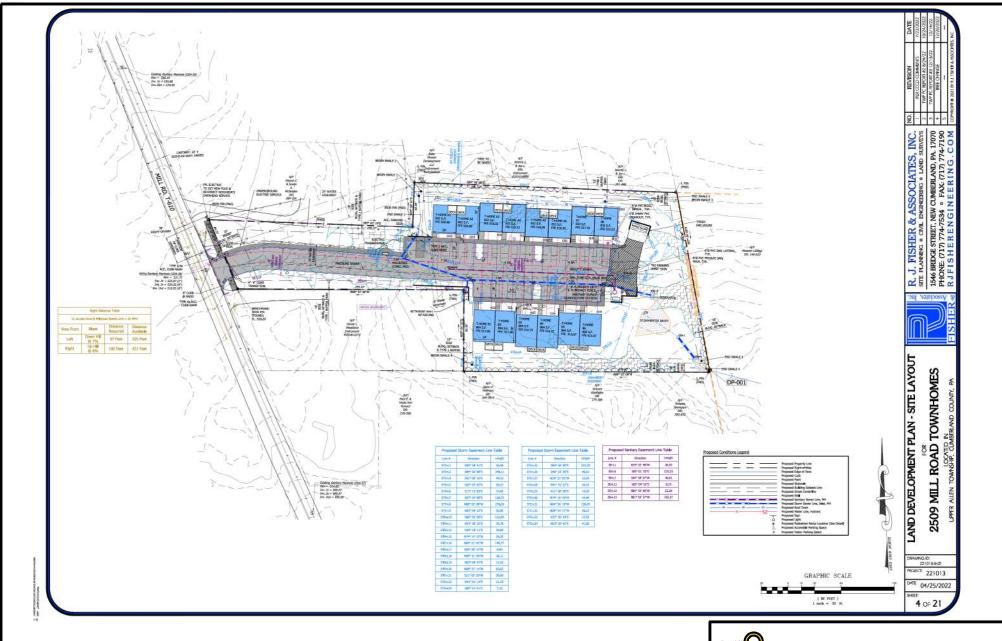
The calculations for the auxiliary turn lane warrants are included in **Appendix F**.

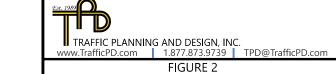
RECOMMENDATIONS AND CONCLUSIONS

The recommendations and conclusions for this Transportation Impact Study are identified in the Executive Summary.

Page 12— www.TrafficPD.com

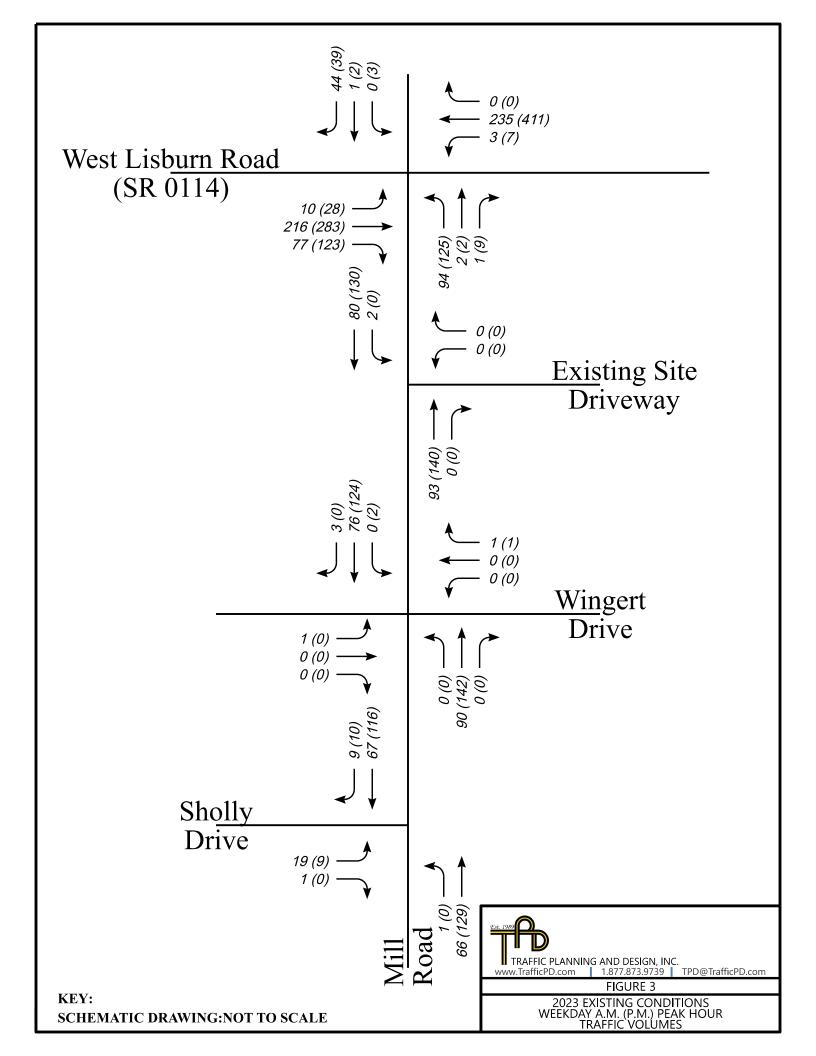




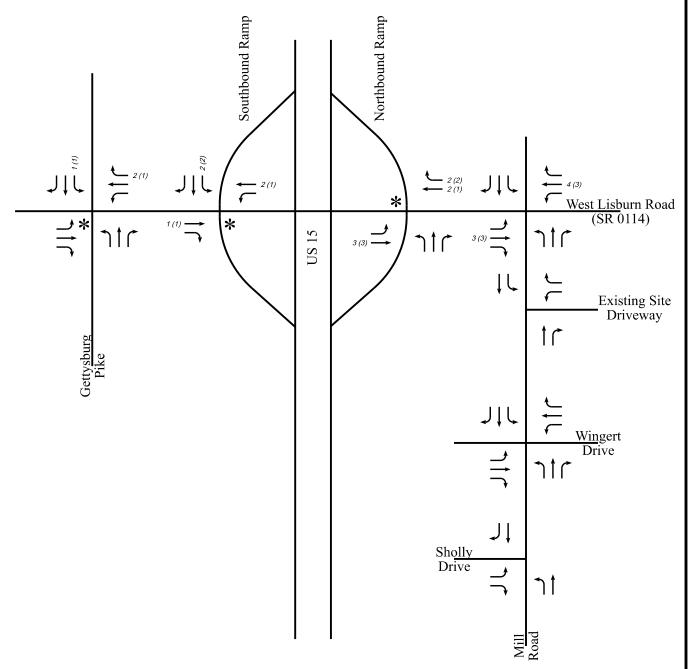


KEY: SCHEMATIC DRAWING:NOT TO SCALE

SITE PLAN



151 - 237 Gettysburg Pike Development

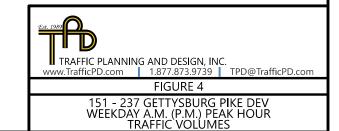


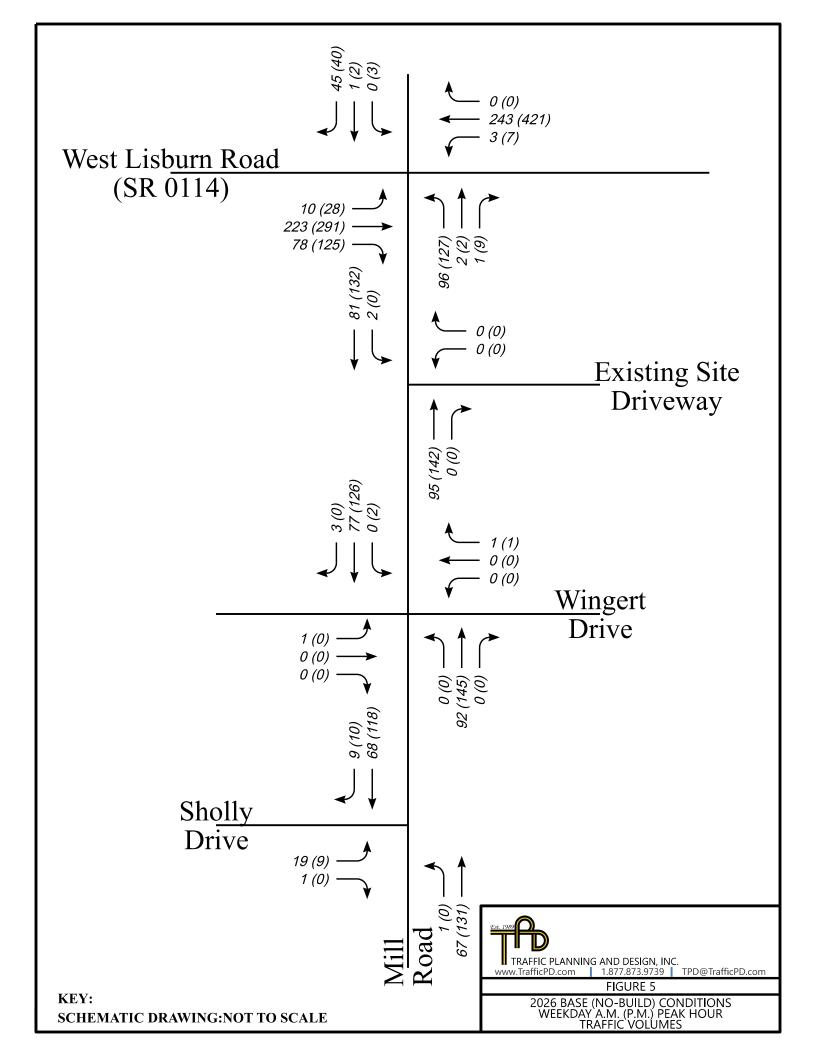
TPD incorporated 5% of the new trips coming to/from the south along Gettysburg Pike to come to/from east on Lisburn Road.

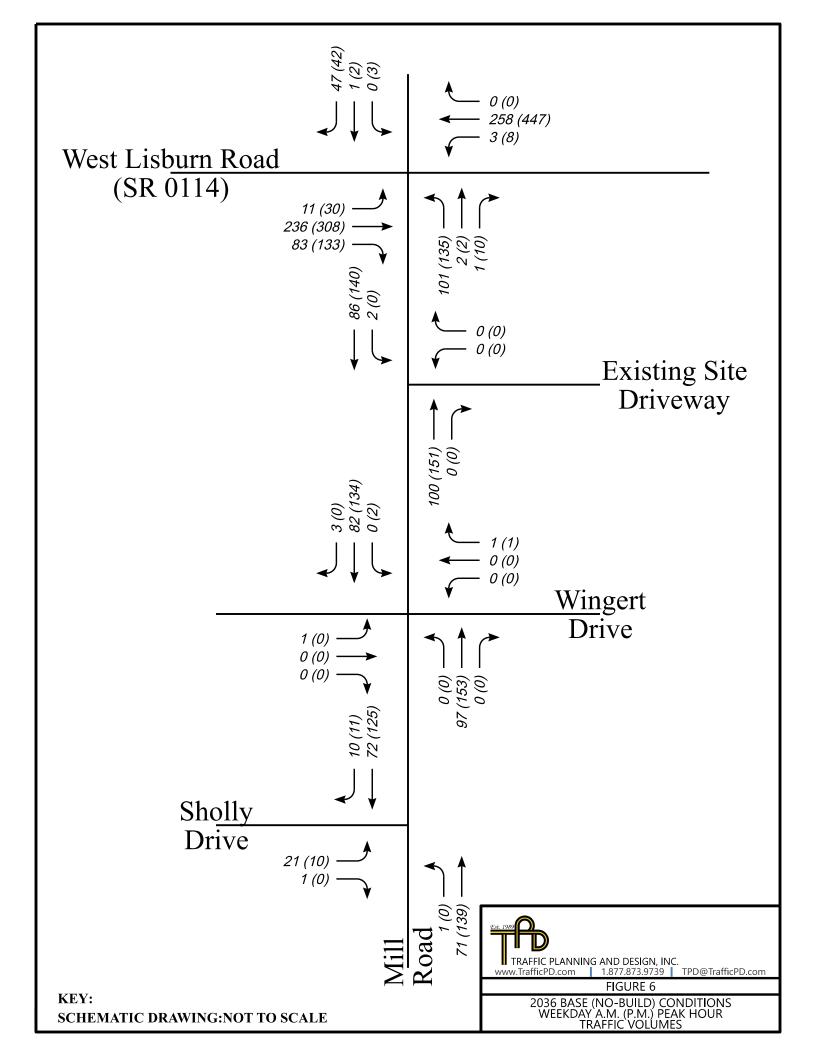
TPD also incorporated 5% of the new trips coming to/from the south along US-15 to come to/from east on Lisburn Road.

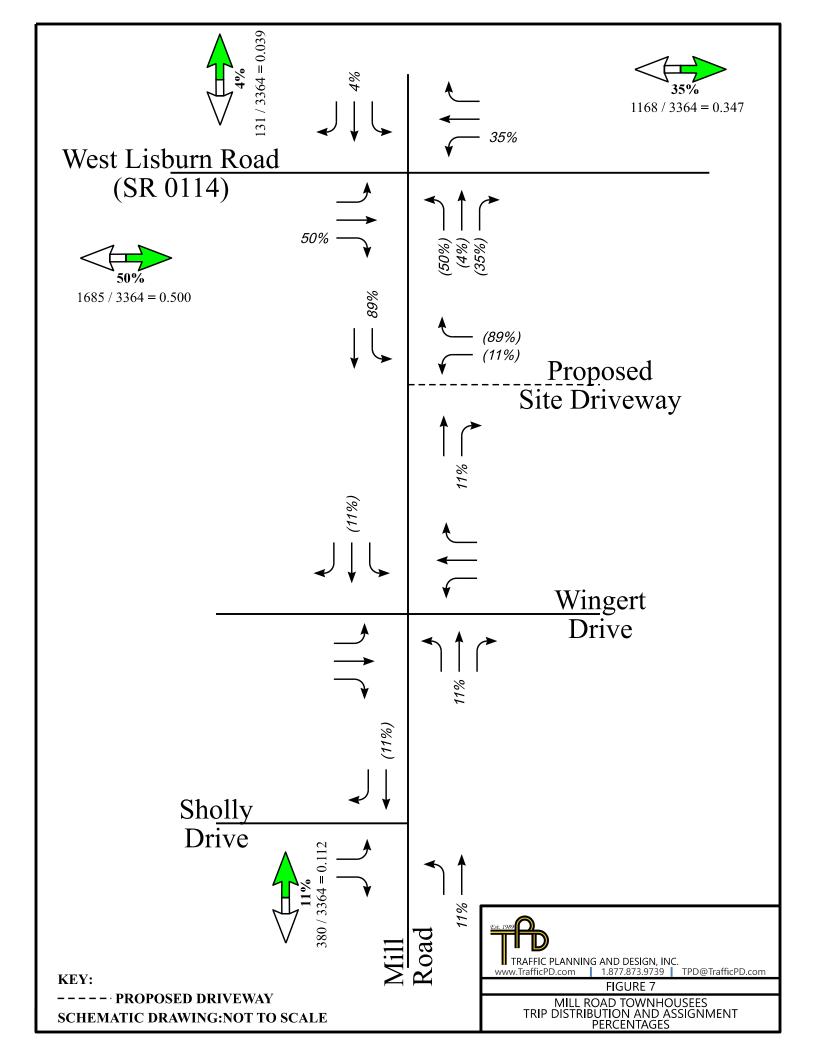
KEY:

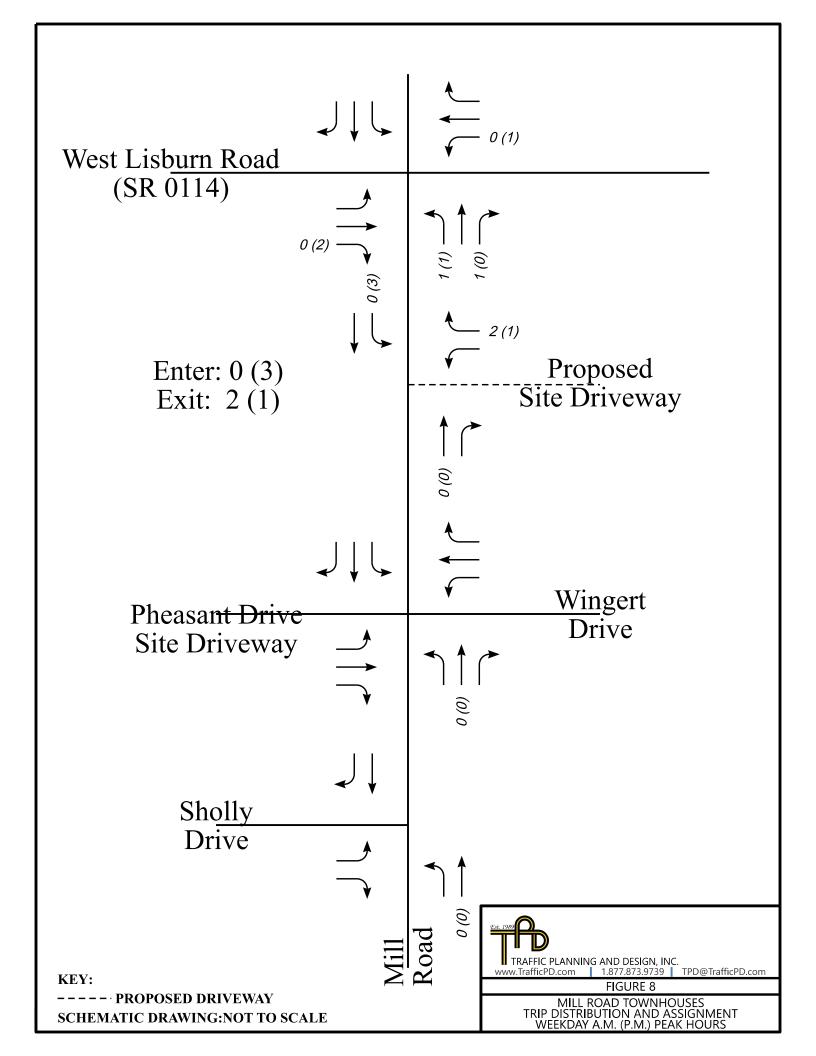
SCHEMATIC DRAWING:NOT TO SCALE
* INTERSECTION FOR ILLISTRATION PURPOSES ONLY

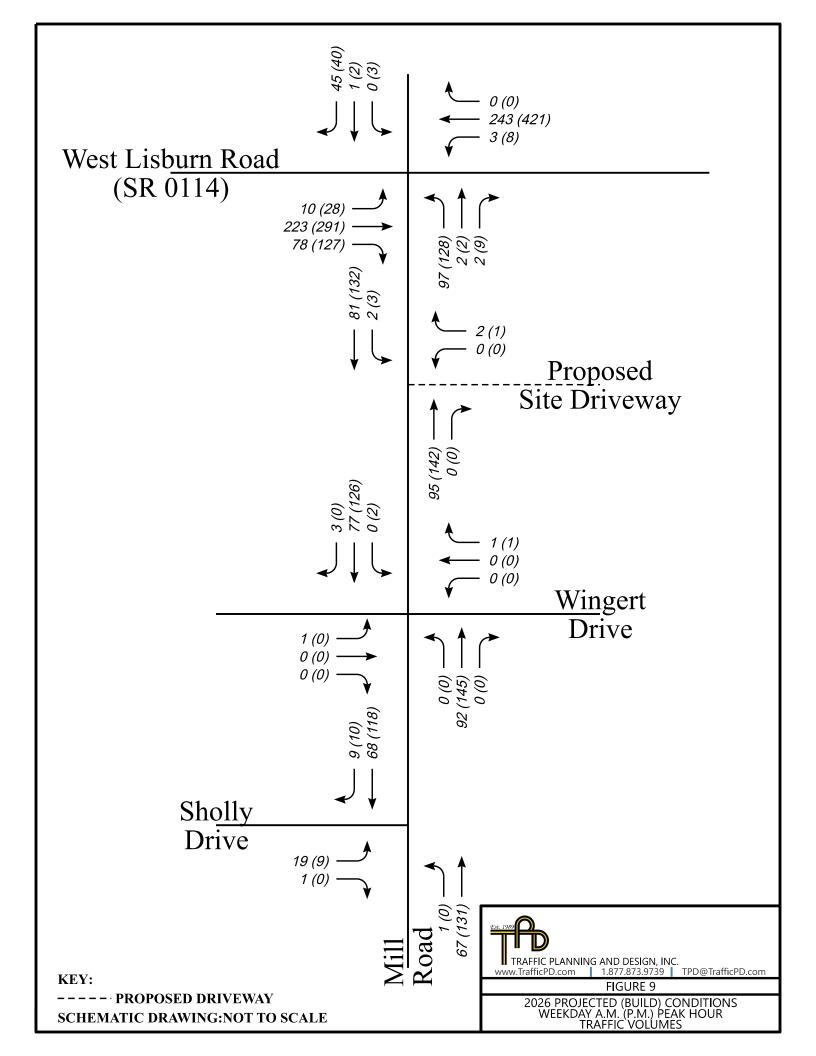


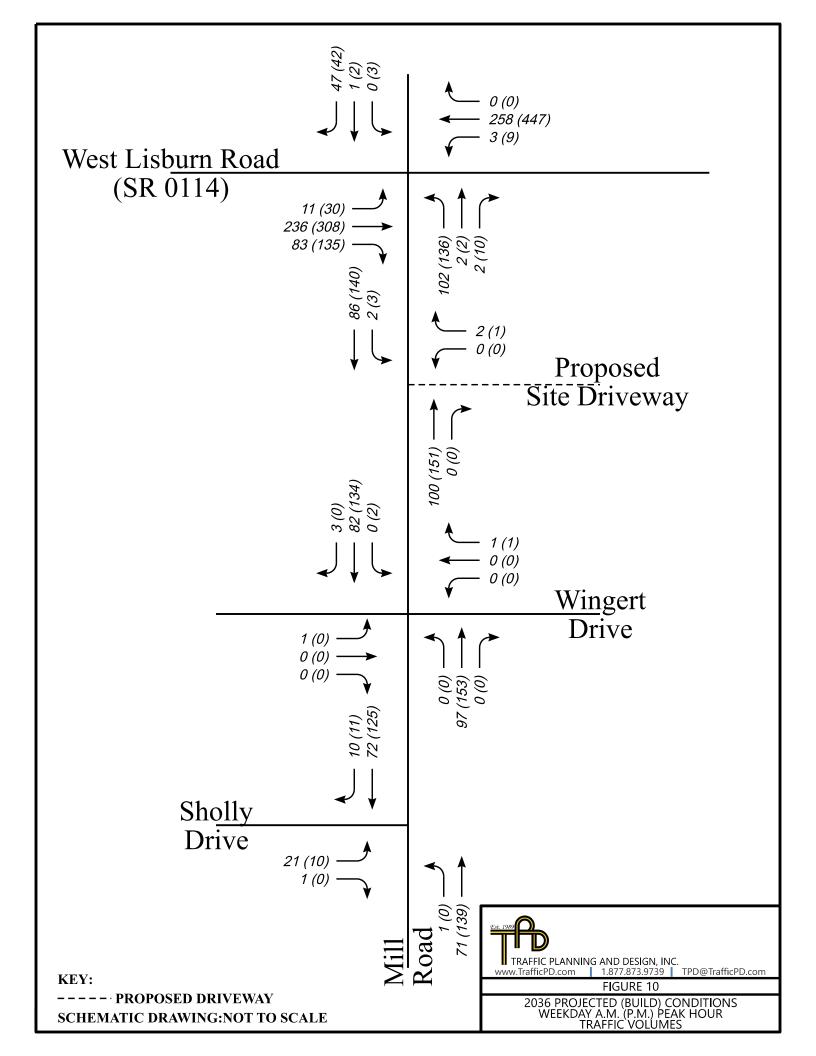












APPENDIX A

STUDY AREA PHOTOGRAPHS & M-950S FORM

Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (1) Mill Road x W Lisburn Road



Direction / Road: NB Mill Road Approach / Departure: Approach

> Distance: 50 feet



Direction / Road: NB Mill Road Approach / Departure: Approach

Distance:

200 Feet

 $\textit{Job \#:} \quad \textit{RJFA.00006} \quad \textit{Date Taken:} \quad \textit{05/09/2023} \quad \textit{Intersection Of:} \quad \textit{(1) Mill Road x W Lisburn Road }$



Direction / Road: SB Mill Road
Approach / Departure: Approach

Distance: 50 feet



Direction / Road: SB Mill Road

Approach / Departure: Approach

Distance: 200 Feet

Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (1) Mill Road x W Lisburn Road



Direction / Road: EB W Lisburn Road SR 2004

Approach / Departure: Approach
Distance: 50 feet



Direction / Road: EB W Lisburn Road SR 2004

Approach / Departure: Approach

Distance: 200 Feet

Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (1) Mill Road x W Lisburn Road



WB W Lisburn Road SR 2004 Direction / Road:

Approach / Departure: Approach

Distance: 50 feet



Direction / Road: WB W Lisburn Road SR 2004

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road:

Center Driveway - Looking Out

Approach / Departure:

Distance:



Direction / Road: Center Driveway – Looking In

Approach / Departure:

Distance:



Direction / Road: Center Driveway – Looking Right

Approach / Departure:

Distance:



Approach / Departure:

Distance:

Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (3) Mill Road x Wingert Drive





Direction / Road: NB Mill Road Approach / Departure: Approach

Distance: 50 feet



NB Mill Road **Direction / Road:**

Approach / Departure: Approach

Distance: 200 Feet Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (3) Mill Road x Wingert Drive



Direction / Road: SB Mill Road Approach / Departure: Approach

> Distance: 50 feet



Direction / Road: SB Mill Road

Approach / Departure: Approach

> Distance: 200 Feet

Job #: RJFA.00006 Date Taken: 05/09/2023 Intersection Of: (3) Mill Road x Wingert Drive



Direction / Road: EB Private Drive Approach / Departure: Approach

Distance:

50 feet



Direction / Road: EB Private Drive

Approach / Departure: Approach



Direction / Road: WB Wingert Drive

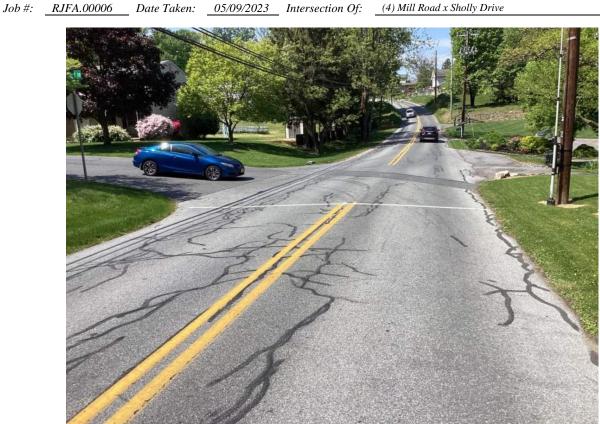
Approach / Departure: Approach

Distance: 50 feet



Direction / Road: WB Wingert Drive

Approach / Departure: Approach



Direction / Road: NB Mill Road Approach / Departure: Approach

Distance: 50 feet



Direction / Road: NB Mill Road

Approach / Departure: Approach



Direction / Road: SB Mill Road Approach / Departure: Approach Distance: 50 feet



SB Mill Road **Direction / Road:**

Approach / Departure: Approach

 $\textit{Job \#:} \quad \textit{RJFA.00006} \quad \textit{Date Taken:} \quad \textit{05/09/2023} \quad \textit{Intersection Of:} \quad \textit{(4) Mill Road x Sholly Drive}$



Direction / Road: EB Sholly Drive

Approach / Departure: Approach Distance: 50 feet



Direction / Road: EB Sholly Drive

Approach / Departure: Approach

DRIVEWAY SIGHT DISTANCE MEASUREMENTS

(FOR LOCAL ROADS, USE PENNDOT PUB 70)

	(1 011	LOCAL HOADS, COL I	ENNBOTT OB TO)	
APPLICANT_Mil	l Road Residenti	al Development	APPLICATION NO. RJ	FA.00006
S.R	SEG	OFFSET	LEGAL SPEED LIMIT_	25 mph
MEASURED BY_	TPD		DATE _05/09/2023	
FOR DEPARTME	ENT USE ONLY: Sa	afe-Running Speed	85th Percentile Speed	
Mill Road	& Proposed Acc	ess		
A				
		4	GRADE3 _% 3.50']
	3.50'	300+' GRADE 3 %		
DISTANCE FSD= 143		Sight Line	DRIVER'S EYE 10' DISTANCE REQUIRED FSD= 151'	EDGE OF TRAVEL LANE
DES= 250		I TH OF BOADWAY ALONG WHI	DES= 195' ICH A DRIVER AT A DRIVEWAY LOCA	TION
			E APPROACHING ON THE ROADWAY.	
B——		CL ← 45'		
			300+¹ GRADE3 %	
			3.50' Sight Line	3.50
========	=======================================		:==========	
			DISTANCE REQUIRED FSD= 151'	
	INUOUSLY SEE THE	REAR OF A VEHICLE WHICH	HICH A DRIVER ON THE ROADWAY C IS LOCATED IN THE DRIVER'S TRAV LEFT TURN INTO A DRIVEWAY.	
		CL←	- 35 ' →	
		Sight Line	3.50'	=========
[3.50 [†]])	GRADE	00+'		
		, į	DISTANCE REQUESTS FSD= 143'	JIRED

THE MAXIMUM LENGTH OF ROADWAY ALONG WHICH A DRIVER OF A VEHICLE INTENDING TO MAKE A LEFT TURN INTO A DRIVEWAY CAN CONTINUOUSLY SEE A VEHICLE APPROACHING FROM THE OPPOSITE DIRECTION.

DES= 190'

APPENDIX BTRAFFIC COUNT DATA PRINTOUTS



Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Mill Road x W Lisburn Road SR 2004 Site Code: Start Date: 05/04/2023 Page No: 1

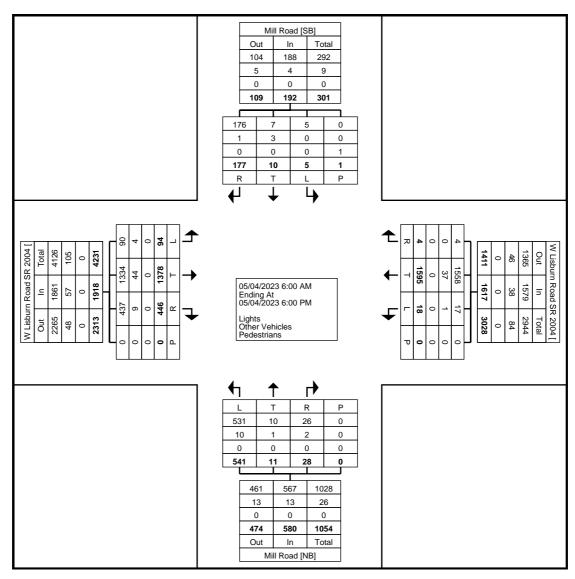
Turning Movement Data

Name								I	urnii	ng IV	lovei	men	t Dai	a								
Sear Time			W Lisbu	rn Road	SR 2004										t t				Mill Road	t t		
Company Comp			E	astboun	d			V	Vestbour	ıd			N	orthbour	nd			S	outhbour	nd		
6:30 AM 6:30 A	Start Time	Left	Thru	Right	Peds		Left	Thru	Right	Peds		Left	Thru	Right	Peds		Left	Thru	Right	Peds		
6:46 AM 0 29 1 0 30 0 31 0 0 31 11 0 0 0 1 11 0 0 0 9 0 9 10 9 1	6:00 AM	3	15	1	0	19	0	24	0	0	24	7	0	0	0	7	0	0	1	0	1	51
Houry Total	6:15 AM	3	24	4	0	31	0	20	0	0	20	7	0	0	0	7	0	1	4	0	5	63
Hourly Total	6:30 AM	0	29	1	0	30	0	31	0	0	31	11	0	0	0	11	0	0	9	0	9	81
7:00 AM	6:45 AM	0	36	9	0	45	0	36	0	0	36	13	0	0	0	13	0	0	10	0	10	104
7:16 AM 7:30 A	Hourly Total	6	104	15	0	125	0	111	0	0	111	38	0	0	0	38	0	1	24	0	25	299
7:30 AM 7:30 AM 7:30 AM 7:45 AM 4	7:00 AM	5	27	4	0	36	0	53	0	0	53	21	1	1	0	23	0	0	16	0	16	128
T-45 AM	7:15 AM	1	45	12	0	58	2	55	0	0	57	36	1	0	0	37	0	0	. 8	0	8	160
Hourly Total	7:30 AM	3	50	25	0	78	0	72	0	0	72	22	0	0	0	22	0	0	16	0	16	188
8:00 AM	7:45 AM	4	62	25	0	91	1	65	0	0	66	20	1	0	0	21	0	0	12	1	12	190
8:15 AM	Hourly Total	13	184	66	0	263	3	245	0	0	248	99	3	1	0	103	0	0	52	1	52	666
8:30 AM 8:45 A	8:00 AM	2	59	15	0	76	0	43	0	0	43	16	0	1	0	17	0	1	8	0	9	145
Best	8:15 AM	3	57	16	0	76	0	51	0	0	51	14	0	2	0	16	0	0	5	0	5	148
Hourly Total 9 220 61 0 290 2 176 0 0 178 74 0 5 0 79 0 1 19 0 20 567 ***BREAK***	8:30 AM	2	51	17	0	70	1	49	0	0	50	22	0	1	0	23	0	0	3	0	3	146
**************************************	8:45 AM	2	53	13	0	68	1	33	0	0	34	22	0	1	0	23	0	0	3	0	3	128
3:00 PM	Hourly Total	9	220	61	0	290	2	176	0	0	178	74	0	5	0	79	0	1	19	0	20	567
3:15 PM	*** BREAK ***	-			-	-	-	-		-		-		-	-		-	-			-	-
3:30 PM	3:00 PM	2	71	20	0	93	1	74	2	0	77	21	1	0	0	22	0	2	4	0	6	198
Book	3:15 PM	5	84	28	0	117	0	70	0	0	70	23	0	1	0	24	0	0	3	0	3	214
Hourly Total 18 323 92 0 433 1 309 3 0 313 100 1 4 0 105 0 4 15 0 19 870 4:00 PM 2 73 29 0 104 1 95 1 0 97 26 2 2 0 30 0 0 6 0 6 237 4:15 FM 7 70 29 0 106 0 103 0 0 103 26 1 5 0 32 0 0 5 0 5 246 4:30 PM 3 71 21 0 95 3 76 0 0 79 31 2 1 0 34 2 1 6 0 9 15 246 Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 1 10 0 1 1 275 5:15 FM 7 80 38 0 125 1 98 0 0 99 34 1 2 2 0 37 0 1 9 0 10 271 5:30 PM 7 62 32 0 101 1 94 0 0 95 34 1 1 0 36 0 0 8 0 8 240 5:45 FM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 0 23 0 1 11 0 1 0 12 173 Hourly Total 26 249 118 0 393 7 386 0 0 0 333 121 2 9 0 132 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28 0 580 5 10 177 1 192 4307 Approach% 4.9 71.8 23.3 1.1 98.6 0.2 93.3 1.9 4.8 2.6 5.2 92.2 Total% 2.2 32.0 10.4 - 44.5 0.4 37.0 0.1 - 37.5 12.6 0.3 0.7 - 13.5 0.1 0.2 4.1 - 4.5 Total% 95.7 96.8 98.0 - 97.0 94.4 97.7 100.0 - 97.6 98.2 90.9 92.9 - 97.8 100.0 70.0 99.4 - 97.9 97.4 Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Pedestrians 0	3:30 PM	7	92	27	0	126	0	76	1	0	77	24	0	3	0	27	0	1	6	0	7	237
4:00 PM 2 73 29 0 104 1 95 1 0 97 26 2 2 0 30 0 0 6 0 6 237 4:15 PM 7 70 29 0 106 0 103 0 0 103 26 1 5 0 32 0 0 5 0 5 246 4:30 PM 3 71 21 0 95 3 76 0 0 79 31 2 1 0 34 2 1 6 0 9 217 4:45 PM 10 84 15 0 109 1 94 0 0 95 26 0 1 0 27 3 0 12 15 246 Hourly Total 22 298 94 0 4125 0 0 129 31 0	3:45 PM	4	76	17	0	97	0	89	0	0	89	32	0	0	0	32	0	1	2	0	3	221
4:15 PM 7 70 29 0 106 0 103 0 0 103 26 1 5 0 32 0 0 5 0 5 246 4:30 PM 3 71 21 0 95 3 76 0 0 79 31 2 1 0 34 2 1 6 0 9 217 4:45 PM 10 84 15 0 109 1 94 0 0 95 26 0 1 0 27 3 0 12 0 15 246 Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 10 0 11 <td>Hourly Total</td> <td>18</td> <td>323</td> <td>92</td> <td>0</td> <td>433</td> <td>1</td> <td>309</td> <td>3</td> <td>0</td> <td>313</td> <td>100</td> <td>1</td> <td>4</td> <td>0</td> <td>105</td> <td>0</td> <td>4</td> <td>15</td> <td>0</td> <td>19</td> <td>870</td>	Hourly Total	18	323	92	0	433	1	309	3	0	313	100	1	4	0	105	0	4	15	0	19	870
4:30 PM 3 71 21 0 95 3 76 0 0 79 31 2 1 0 34 2 1 6 0 9 217 4:45 PM 10 84 15 0 109 1 94 0 0 95 26 0 1 0 27 3 0 12 0 15 246 Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 10 0 11 10 0 11 29 0 132 9 0 10 22 0 37 0 1 9 0 10 22 0 36 0 0 8 <	4:00 PM	2	73	29	0	104	1	95	1	0	97	26	2	2	0	30	0	0	6	0	6	237
4:45 PM 10 84 15 0 109 1 94 0 0 95 26 0 1 0 27 3 0 12 0 15 246 Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 10 0 11 275 5:15 PM 7 80 38 0 125 1 98 0 0 99 34 1 2 0 37 0 1 9 0 10 271 1 28 240 11 2 0 37 0 1 9 0 10 2 0 1 0 36 0 0 8 240	4:15 PM	7	70	29	0	106	0	103	0	0	103	26	1	5	0	32	0	0	. 5	0	5	246
Hourly Total 22 298 94 0 414 5 368 1 0 374 109 5 9 0 123 5 1 29 0 35 946 5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 10 0 11 275 5:15 PM 7 80 38 0 125 1 98 0 0 99 34 1 2 0 37 0 1 9 0 10 271 5:30 PM 7 62 32 0 101 1 94 0 0 95 34 1 1 0 36 0 0 8 0 8 240 5:45 PM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 23 0 1 11 0 12 173 Hourly Total 26 249 118 0 393 7 386 0 0 3393 121 2 9 0 132 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28 0 580 5 10 177 1 192 4307 Approach 4.9 71.8 23.3 1.1 98.6 0.2 93.3 1.9 4.8 2.6 5.2 92.2 1.1 1 98.6 0.2 93.3 1.9 4.8 2.6 5.2 92.2 1.1 1 98.6 0.2 1.1 1578 1 10 26 - 567 5 7 176 - 188 4195 Mights 95.7 96.8 98.0 - 97.0 94.4 97.7 100.0 - 97.6 98.2 90.9 92.9 - 97.8 100.0 70.0 99.4 - 97.9 97.4 Other Vehicles 4 44 9 - 57 1 37 0 - 38 10 1 2 2 - 13 0 3 0 0 6 - 2.1 2.6 September 1	4:30 PM	3	71	21	0	95	3	76	0	0	79	31	2	1	0	34	2	1	6	0	9	217
5:00 PM 4 57 38 0 99 4 125 0 0 129 31 0 5 0 36 0 1 10 0 11 275 5:15 PM 7 80 38 0 125 1 98 0 0 99 34 1 2 0 37 0 1 9 0 10 271 5:30 PM 7 62 32 0 101 1 94 0 0 95 34 1 1 0 36 0 0 8 240 5:45 PM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28	4:45 PM	10	84	15	0	109	1	94	0	0	95	26	0	1	0	27	3	0	12	0	15	246
5:15 PM 7 80 38 0 125 1 98 0 0 99 34 1 2 0 37 0 1 9 0 10 271 5:30 PM 7 62 32 0 101 1 94 0 0 95 34 1 1 0 36 0 0 8 240 5:45 PM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 23 0 1 11 0 12 173 Hourly Total 26 249 118 0 393 7 386 0 0 393 121 2 9 0 132 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 <t< td=""><td>Hourly Total</td><td>22</td><td>298</td><td>94</td><td>0</td><td>414</td><td>5</td><td>368</td><td>1</td><td>0</td><td>374</td><td>109</td><td>5</td><td>9</td><td>0</td><td>123</td><td>5</td><td>1</td><td>29</td><td>0</td><td>35</td><td>946</td></t<>	Hourly Total	22	298	94	0	414	5	368	1	0	374	109	5	9	0	123	5	1	29	0	35	946
5:30 PM 7 62 32 0 101 1 94 0 0 95 34 1 1 0 36 0 0 8 0 8 240 5:45 PM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 23 0 1 11 0 12 173 Hourly Total 26 249 118 0 393 7 386 0 0 393 121 2 9 0 132 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28 0 580 5 10 177 1 192 4307 Approach % 4.9 71.8 23.3 - - 1.1 98.6 <t< td=""><td>5:00 PM</td><td>4</td><td>57</td><td>38</td><td>0</td><td>99</td><td>4</td><td>125</td><td>0</td><td>0</td><td>129</td><td>31</td><td>0</td><td>5</td><td>0</td><td>36</td><td>0</td><td>1</td><td>10</td><td>0</td><td>11</td><td>275</td></t<>	5:00 PM	4	57	38	0	99	4	125	0	0	129	31	0	5	0	36	0	1	10	0	11	275
5:45 PM 8 50 10 0 68 1 69 0 0 70 22 0 1 0 23 0 1 11 0 12 173 Hourly Total 26 249 118 0 393 7 386 0 0 393 121 2 9 0 132 0 3 38 0 41 959 Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28 0 580 5 10 177 1 192 4307 Approach % 4.9 71.8 23.3 - - 1.1 98.6 0.2 - - 93.3 1.9 4.8 - - 2.6 5.2 92.2 - - - 1 192 4307 Total % 2.2 32.0 10.4 <t< td=""><td>5:15 PM</td><td>7</td><td>80</td><td>38</td><td>0</td><td>125</td><td>1</td><td>98</td><td>0</td><td>0</td><td>99</td><td>34</td><td>1</td><td>2</td><td>0</td><td>37</td><td>0</td><td>1</td><td>9</td><td>0</td><td>10</td><td>271</td></t<>	5:15 PM	7	80	38	0	125	1	98	0	0	99	34	1	2	0	37	0	1	9	0	10	271
Hourly Total 26 249 118 0 393 7 386 0 0 393 121 2 9 0 132 0 3 38 0 41 959	5:30 PM	7	62	32	0	101	1	94	0	0	95	34	1	1	0	36	0	0	. 8	0	8	240
Grand Total 94 1378 446 0 1918 18 1595 4 0 1617 541 11 28 0 580 5 10 177 1 192 4307 Approach % 4.9 71.8 23.3 - - 1.1 98.6 0.2 - - 93.3 1.9 4.8 - - 2.6 5.2 92.2 - - - Total % 2.2 32.0 10.4 - 44.5 0.4 37.0 0.1 - 37.5 12.6 0.3 0.7 - 13.5 0.1 0.2 4.1 - 4.5 - Lights 90 1334 437 - 1861 17 1558 4 - 1579 531 10 26 - 567 5 7 176 - 188 4195 % Lights 95.7 96.8 98.0 - 97.0 <td>5:45 PM</td> <td>8</td> <td>50</td> <td>10</td> <td>0</td> <td>68</td> <td>1</td> <td>69</td> <td>0</td> <td>0</td> <td>70</td> <td>22</td> <td>0</td> <td>1</td> <td>0</td> <td>23</td> <td>0</td> <td>1</td> <td>11</td> <td>0</td> <td>12</td> <td>173</td>	5:45 PM	8	50	10	0	68	1	69	0	0	70	22	0	1	0	23	0	1	11	0	12	173
Approach % 4.9 71.8 23.3 - - 1.1 98.6 0.2 - - 93.3 1.9 4.8 - - 2.6 5.2 92.2 -	Hourly Total	26	249	118	0	393	7	386	0	0	393	121	2	9	0	132	0	3	38	0	41	959
Total % 2.2 32.0 10.4 - 44.5 0.4 37.0 0.1 - 37.5 12.6 0.3 0.7 - 13.5 0.1 0.2 4.1 - 4.5 - Lights 90 1334 437 - 1861 17 1558 4 - 1579 531 10 26 - 567 5 7 176 - 188 4195 % Lights 95.7 96.8 98.0 - 97.0 94.4 97.7 100.0 - 97.6 98.2 90.9 92.9 - 97.8 100.0 70.0 99.4 - 97.9 97.4 Other Vehicles 4 44 9 - 57 1 37 0 - 38 10 1 2 - 13 0 3 1 - 4 112 % Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Pedestrians 0 - 0 0 - 0 0 1 - 1	Grand Total	94	1378	446	0	1918	18	1595	4	0	1617	541	11	28	0	580	5	10	177	1	192	4307
Lights 90 1334 437 - 1861 17 1558 4 - 1579 531 10 26 - 567 5 7 176 - 188 4195 % Lights 95.7 96.8 98.0 - 97.0 94.4 97.7 100.0 - 97.6 98.2 90.9 92.9 - 97.8 100.0 70.0 99.4 - 97.9 97.4 Other Vehicles 4 44 9 - 57 1 37 0 - 38 10 1 2 - 13 0 3 1 - 4 112 % Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Workington - - - - -	Approach %	4.9	71.8	23.3	-	-	1.1	98.6	0.2	-	_	93.3	1.9	4.8	-	-	2.6	5.2	92.2	-	-	-
% Lights 95.7 96.8 98.0 - 97.0 94.4 97.7 100.0 - 97.6 98.2 90.9 92.9 - 97.8 100.0 70.0 99.4 - 97.9 97.4 Other Vehicles 4 44 9 - 57 1 37 0 - 38 10 1 2 - 13 0 3 1 - 4 112 % Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Pedestrians - - 0 - - 0 - - - 0 - - - 0 - - - - 0 - - - 0 - - 2.1 2.6	Total %	2.2	32.0	10.4	-	44.5	0.4	37.0	0.1	-	37.5	12.6	0.3	0.7	-	13.5	0.1	0.2	4.1	-	4.5	-
Other Vehicles 4 44 9 - 57 1 37 0 - 38 10 1 2 - 13 0 3 1 - 4 112 % Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Pedestrians - - - 0 - - - - 0 - - - - 1 - <td>Lights</td> <td>90</td> <td>1334</td> <td>437</td> <td>-</td> <td>1861</td> <td>17</td> <td>1558</td> <td>4</td> <td>-</td> <td>1579</td> <td>531</td> <td>10</td> <td>26</td> <td>-</td> <td>567</td> <td>5</td> <td>7</td> <td>176</td> <td>_</td> <td>188</td> <td>4195</td>	Lights	90	1334	437	-	1861	17	1558	4	-	1579	531	10	26	-	567	5	7	176	_	188	4195
% Other Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.6 Pedestrians - - - 0 - - - 0 - - - 1 - - 1 - - - 1 - <td>% Lights</td> <td>95.7</td> <td>96.8</td> <td>98.0</td> <td>_</td> <td>97.0</td> <td>94.4</td> <td>97.7</td> <td>100.0</td> <td>-</td> <td>97.6</td> <td>98.2</td> <td>90.9</td> <td>92.9</td> <td>-</td> <td>97.8</td> <td>100.0</td> <td>70.0</td> <td>99.4</td> <td>_</td> <td>97.9</td> <td>97.4</td>	% Lights	95.7	96.8	98.0	_	97.0	94.4	97.7	100.0	-	97.6	98.2	90.9	92.9	-	97.8	100.0	70.0	99.4	_	97.9	97.4
Vehicles 4.3 3.2 2.0 - 3.0 5.6 2.3 0.0 - 2.4 1.8 9.1 7.1 - 2.2 0.0 30.0 0.6 - 2.1 2.5 Pedestrians - - - 0 - - - 0 - - - - - 1 - -	Other Vehicles	4	44	9	-	57	1	37	0	-	38	10	1	2	-	13	0	3	1	-	4	112
		4.3	3.2	2.0	-	3.0	5.6	2.3	0.0	-	2.4	1.8	9.1	7.1	-	2.2	0.0	30.0	0.6	-	2.1	2.6
% Pedestrians - <	Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-	-		1	-	-
	% Pedestrians	-		-	-	-	-			-	-	-	-	-	-	-	-	-		100.0	-	-



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Count Name: (1) AM/PM Mill Road x W Lisburn Road SR 2004 Site Code: Start Date: 05/04/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (1) AM/PM Mill Road x W Lisburn Road SR 2004 Site Code: Start Date: 05/04/2023 Page No: 3

Turning Movement Peak Hour Data (7:15 AM)

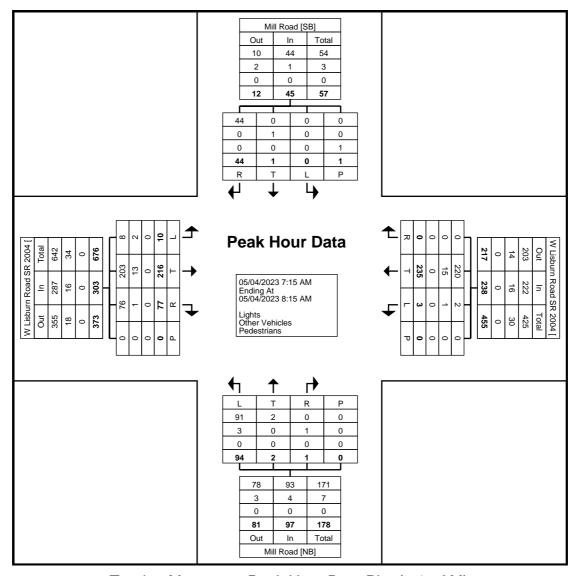
		W Lisbu	rn Road	SR 2004			W Lisbu	rn Road	SR 2004			I	Mill Road	l	,		1	Mill Road	i		
		E	astboun	d			V	/estboun	d			N	orthbour	ıd			S	outhbour	nd		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
7:15 AM	1	45	12	0	58	2	55	0	0	57	36	1	0	0	37	0	0	8	0	8	160
7:30 AM	3	50	25	0	78	0	72	0	0	72	22	0	0	0	22	0	0	16	0	16	188
7:45 AM	4	62	25	0	91	1	65	0	0	66	20	1	0	0	21	0	0	12	1	12	190
8:00 AM	2	59	15	0	76	0	43	0	0	43	16	0	1	0	17	0	1	8	0	9	145
Total	10	216	77	0	303	3	235	0	0	238	94	2	1	0	97	0	1	44	1	45	683
Approach %	3.3	71.3	25.4	-	-	1.3	98.7	0.0	-	-	96.9	2.1	1.0	-	-	0.0	2.2	97.8	-	-	-
Total %	1.5	31.6	11.3	-	44.4	0.4	34.4	0.0	-	34.8	13.8	0.3	0.1	-	14.2	0.0	0.1	6.4	-	6.6	-
PHF	0.625	0.871	0.770	-	0.832	0.375	0.816	0.000	-	0.826	0.653	0.500	0.250	-	0.655	0.000	0.250	0.688	-	0.703	0.899
Lights	8	203	76	-	287	2	220	0	-	222	91	2	0	-	93	0	0	44	-	44	646
% Lights	80.0	94.0	98.7	-	94.7	66.7	93.6	-	-	93.3	96.8	100.0	0.0	-	95.9	•	0.0	100.0	-	97.8	94.6
Other Vehicles	2	13	1	-	16	1	15	0	-	16	3	0	1	-	4	0	1	0	-	1	37
% Other Vehicles	20.0	6.0	1.3	-	5.3	33.3	6.4	-	-	6.7	3.2	0.0	100.0	-	4.1	-	100.0	0.0	-	2.2	5.4
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-	-	-	1	-	-
% Pedestrians	-	_	-	-	-	-	-	-	-	_	-	_	_	-		-	_	_	100.0	-	-



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Count Name: (1) AM/PM Mill Road x W Lisburn Road SR

2004 Site Code: Start Date: 05/04/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:15 AM)



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Count Name: (1) AM/PM Mill Road x W Lisburn Road SR 2004 Site Code: Start Date: 05/04/2023 Page No: 5

Turning Movement Peak Hour Data (4:45 PM)

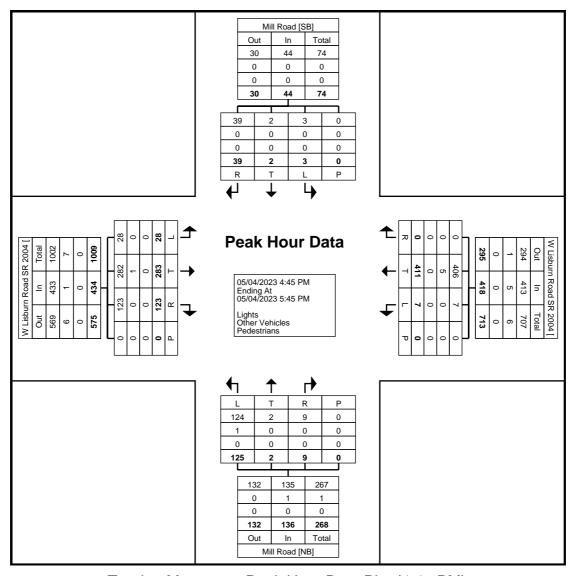
			rn Road					rn Road					Mill Road		,			Mill Road			
Start Time	Left	Thru	astboun Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	orthbour Right	Peds	App. Total	Left	Thru	outhbour Right	Peds	App. Total	Int. Total
4:45 PM	10	84	15	0	109	1	94	0	0	95	26	0	1	0	27	3	0	12	0	15	246
5:00 PM	4	57	38	0	99	4	125	0	0	129	31	0	5	0	36	0	1	10	0	11	275
5:15 PM	7	80	38	0	125	1	98	0	0	99	34	1	2	0	37	0	1	9	0	10	271
5:30 PM	7	62	32	0	101	1	94	0	0	95	34	1	1	0	36	0	0	8	0	8	240
Total	28	283	123	0	434	7	411	0	0	418	125	2	9	0	136	3	2	39	0	44	1032
Approach %	6.5	65.2	28.3	-	-	1.7	98.3	0.0	-		91.9	1.5	6.6	-		6.8	4.5	88.6	-		-
Total %	2.7	27.4	11.9	-	42.1	0.7	39.8	0.0	-	40.5	12.1	0.2	0.9	-	13.2	0.3	0.2	3.8	-	4.3	-
PHF	0.700	0.842	0.809	-	0.868	0.438	0.822	0.000	-	0.810	0.919	0.500	0.450	-	0.919	0.250	0.500	0.813	-	0.733	0.938
Lights	28	282	123	-	433	7	406	0	-	413	124	2	9	-	135	3	2	39	-	44	1025
% Lights	100.0	99.6	100.0	-	99.8	100.0	98.8	-	-	98.8	99.2	100.0	100.0	-	99.3	100.0	100.0	100.0	-	100.0	99.3
Other Vehicles	0	1	0	-	1	0	5	0	-	5	1	0	0	-	1	0	0	0	-	0	7
% Other Vehicles	0.0	0.4	0.0	-	0.2	0.0	1.2	-	-	1.2	0.8	0.0	0.0	-	0.7	0.0	0.0	0.0	-	0.0	0.7
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: (1) AM/PM Mill Road x W Lisburn Road SR

2004 Site Code: Start Date: 05/04/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:45 PM)



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 1

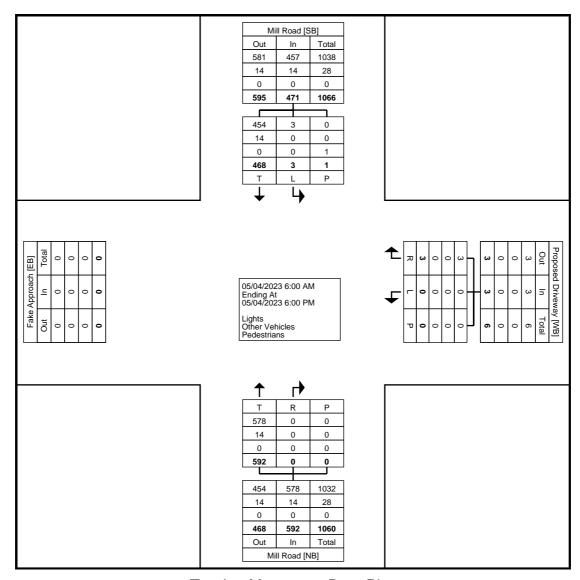
Turning Movement Data

				ΙŲ	ming i	Vlovem	IEIIL D	ala ˌ					1
		Proposed	Driveway			Mill	Road			Mill I	Road		
Start Time		Westb	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Int. Total
6:00 AM	0	0	0	0	7	0	0	7	0	1	0	1	8
6:15 AM	0	0	0	0	8	0	0	. 8	0	6	0	6	14
6:30 AM	0	0	0	0	10	0	0	10	0	0	0	0	10
6:45 AM	0	0	0	0	18	0	0	18	0	9	0	9	27
Hourly Total	0	0	0	0	43	0	0	43	0	16	0	16	59
7:00 AM	0	2	0	2	21	0	0	21	0	6	0	6	29
7:15 AM	0	0	0	0	31	0	0	31	0	13	0	13	44
7:30 AM	0	0	0	0	22	0	0	22	1	25	0	26	48
7:45 AM	0	0	0	0	23	0	0	23	1	24	0	25	48
Hourly Total	0	2	0	2	97	0	0	97	2	68	0	70	169
8:00 AM	0	0	0	0	17	0	0	17	0	18	0	18	35
8:15 AM	0	0	0	0	18	0	0	18	0	13	0	13	31
8:30 AM	0	0	0	0	18	0	0	18	0	19	0	19	37
8:45 AM	0	0	0	0	24	0	0	24	0	15	0	15	39
Hourly Total	0	0	0	0	77	0	0	77	0	65	0	65	142
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	0	0	0	0	23	0	0	23	0	24	0	24	47
3:15 PM	0	0	0	0	26	0	0	26	0	27	1	27	53
3:30 PM	0	0	0	0	32	0	0	32	0	27	0	27	59
3:45 PM	0	0	0	0	33	0	0	33	0	17	0	17	50
Hourly Total	0	0	0	0	114	0	0	114	0	95	1	95	209
4:00 PM	0	1	0	1	25	0	0	25	0	33	0	33	59
4:15 PM	0	0	0	0	34	0	0	34	0	30	0	30	64
4:30 PM	0	0	0	0	36	0	0	36	1	20	0	21	57
4:45 PM	0	0	0	0	26	0	0	26	0	19	0	19	45
Hourly Total	0	1	0	1	121	0	0	121	1	102	0	103	225
5:00 PM	0	0	0	0	35	0	0	35	0	42	0	42	77
5:15 PM	0	0	0	0	46	0	0	46	0	35	0	35	81
5:30 PM	0	0	0	0	33	0	0	33	0	34	0	34	67
5:45 PM	0	0	0	0	26	0	0	26	0	11	0	11	37
Hourly Total	0	0	0	0	140	0	0	140	0	122	0	122	262
Grand Total	0	3	0	3	592	0	0	592	3	468	1	471	1066
Approach %	0.0	100.0	-	_	100.0	0.0		-	0.6	99.4	-	_	-
Total %	0.0	0.3	-	0.3	55.5	0.0	-	55.5	0.3	43.9	-	44.2	-
Lights	0	3	-	3	578	0	-	578	3	454	_	457	1038
% Lights	-	100.0	-	100.0	97.6	-	-	97.6	100.0	97.0		97.0	97.4
Other Vehicles	0	0	-	0	14	0		14	0	14		14	28
% Other Vehicles	-	0.0	-	0.0	2.4	-	-	2.4	0.0	3.0	-	3.0	2.6
Pedestrians	-	-	0	-	-		0	-	-	-	1	-	-
% Pedestrians	-		-	_			-		-		100.0	_	_



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 3

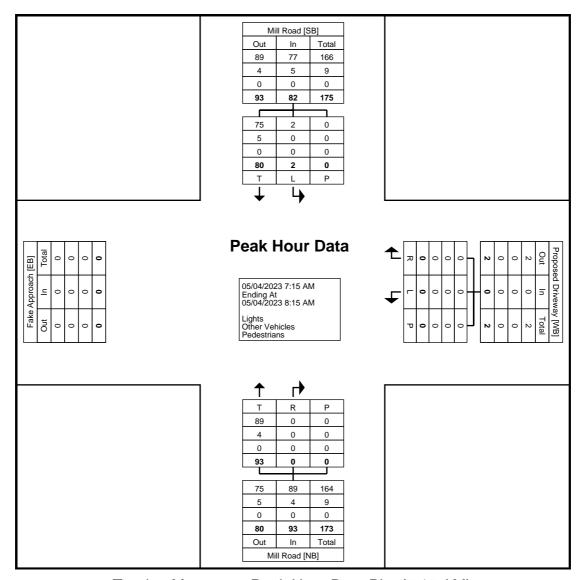
Turning Movement Peak Hour Data (7:15 AM)

			. •					~ (. • ,				
		Proposed	Driveway			Mill F	Road		•	Mill F	Road		
Ot and Time		West	oound			North	oound			South	bound		
Start Time	Left	Right	Peds	App. Total	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Int. Total
7:15 AM	0	0	0	0	31	0	0	31	0	13	0	13	44
7:30 AM	0	0	0	0	22	0	0	22	1	25	0	26	48
7:45 AM	0	0	0	0	23	0	0	23	1	24	0	25	48
8:00 AM	0	0	0	0	17	0	0	17	0	18	0	18	35
Total	0	0	0	0	93	0	0	93	2	80	0	82	175
Approach %	0.0	0.0	-	-	100.0	0.0	-	-	2.4	97.6	-	-	-
Total %	0.0	0.0	-	0.0	53.1	0.0	-	53.1	1.1	45.7	-	46.9	-
PHF	0.000	0.000	-	0.000	0.750	0.000	-	0.750	0.500	0.800	-	0.788	0.911
Lights	0	0	-	0	89	0	-	89	2	75	-	77	166
% Lights	-	-	-	-	95.7	-	-	95.7	100.0	93.8	-	93.9	94.9
Other Vehicles	0	0	-	0	4	0	-	4	0	5	-	5	9
% Other Vehicles	-	-	-	-	4.3	-	-	4.3	0.0	6.3	-	6.1	5.1
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	_		_				_	_			_		_



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:15 AM)



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 5

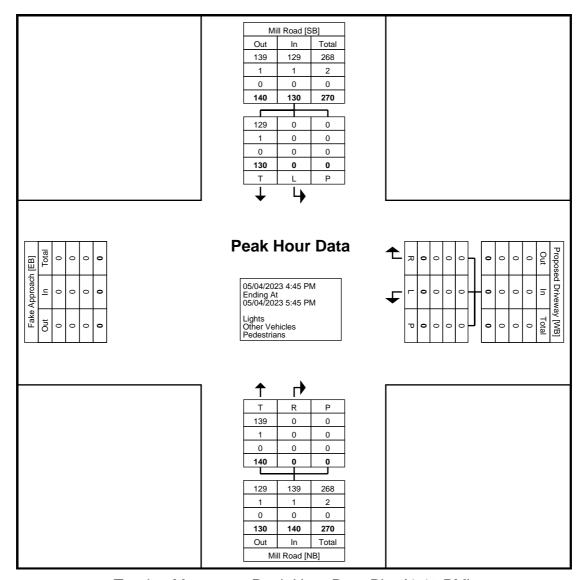
Turning Movement Peak Hour Data (4:45 PM)

	_			.9				~ · · · ·	,				
		Proposed	Driveway	_		Mill F	Road		•	Mill F	Road		
Ota et Tierra		West	oound			North	oound			South	bound		
Start Time	Left	Right	Peds	App. Total	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Int. Total
4:45 PM	0	0	0	0	26	0	0	26	0	19	0	19	45
5:00 PM	0	0	0	0	35	0	0	35	0	42	0	42	77
5:15 PM	0	0	0	0	46	0	0	46	0	35	0	35	81
5:30 PM	0	0	0	0	33	0	0	33	0	34	0	34	67
Total	0	0	0	0	140	0	0	140	0	130	0	130	270
Approach %	0.0	0.0	-	-	100.0	0.0	-	-	0.0	100.0	-	-	-
Total %	0.0	0.0	-	0.0	51.9	0.0	-	51.9	0.0	48.1	-	48.1	-
PHF	0.000	0.000	-	0.000	0.761	0.000	-	0.761	0.000	0.774	-	0.774	0.833
Lights	0	0	-	0	139	0	-	139	0	129	-	129	268
% Lights	-	-	-	-	99.3	-	-	99.3	-	99.2	-	99.2	99.3
Other Vehicles	0	0	-	0	1	0	-	1	0	1	-	1	2
% Other Vehicles	-	-	-	-	0.7	-	-	0.7	-	0.8	-	0.8	0.7
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	_	_	-	-	-	_	-	-	_	_	-	-	-



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Count Name: (2) AM/PM Mill Road x Proposed Access Site Code: Start Date: 05/04/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:45 PM)



Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 1

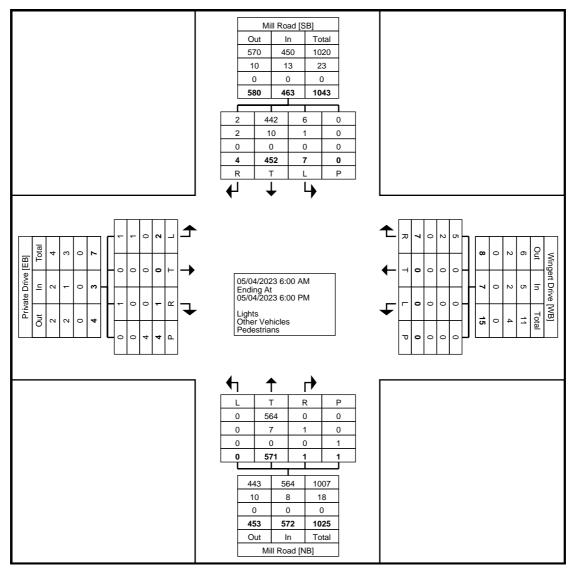
Turning Movement Data

							ı	urnir	ig ivi	over	nen	ı Dai	la								
		Pi	rivate Dri	ve			W	ingert Dri	ive				Mill Road	t				Mill Road	i		
		E	astboun	d			V	Vestboun	ıd			N	lorthbour	nd			S	outhbour	nd		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7	0	1	0	0	1	8
6:15 AM	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7	0	5	0	0	5	12
6:30 AM	0	0	0	0	0	0	0	0	0	0	0	11	0	0	11	0	1	0	0	1	12
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12	0	9	0	0	9	21
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	37	0	0	37	0	16	0	0	16	53
7:00 AM	0	0	0	0	0	0	0	1	0	1	0	22	0	0	22	0	4	0	0	4	27
7:15 AM	1	0	0	0	1	0	0	1	0	1	0	29	0	1	29	0	11	2	0	13	44
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22	0	24	0	0	24	46
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22	0	23	1	0	24	46
Hourly Total	1	0	0	0	1	0	0	2	0	2	0	95	0	1	95	0	62	3	0	65	163
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	17	0	0	17	0	18	0	0	18	35
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	17	0	0	17	0	15	0	0	15	32
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	21	0	0	21	1	15	1	0	17	38
8:45 AM	1	0	0	0	1	0	0	1	0	1	0	21	0	0	21	0	16	0	0	16	39
Hourly Total	1	0	0	0	1	0	0	1	0	1	0	76	0	0	76	1	64	1	0	66	144
*** BREAK ***	-	-		-	-	-	-		-	-	-			-	-	-	-		-	-	-
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22	0	20	0	0	20	42
3:15 PM	0	0	0	0	0	0	0	1	0	1	0	23	0	0	23	0	27	0	0	27	51
3:30 PM	0	0	0	0	0	0	0	0	0	0	0	27	0	0	27	1	29	0	0	30	57
3:45 PM	0	0	0	0	0	0	0	0	0	0	0	32	0	0	32	0	17	0	0	17	49
Hourly Total	0	0	0	0	0	0	0	1	0	1	0	104	0	0	104	1	93	0	0	94	199
4:00 PM	0	0	1	0	1	0	0	1	0	1	0	28	1	0	29	2	27	0	0	29	60
4:15 PM	0	0	0	1	0	0	0	0	0	0	0	33	0	0	33	1	28	0	0	29	62
4:30 PM	0	0	0	0	0	0	0	1	0	1	0	32	0	0	32	0	25	0	0	25	58
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	28	0	0	28	0	16	0	0	16	44
Hourly Total	0	0	1	1	1	0	0	2	0	2	0	121	1	0	122	3	96	0	0	99	224
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	36	0	0	36	1	41	0	0	42	78
5:15 PM	0	0	0	0	0	0	0	1	0	1	0	40	0	0	40	0	36	0	0	36	77
5:30 PM	0	0	0	3	0	0	0	0	0	0	0	38	0	0	38	1	31	0	0	32	70
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	24	0	0	24	0	13	0	0	13	37
Hourly Total	0	0	0	3	0	0	0	1	0	1	0	138	0	0	138	2	121	0	0	123	262
Grand Total	2	0	1	4	3	0	0	7	0	7	0	571	1	1	572	7	452	4	0	463	1045
Approach %	66.7	0.0	33.3	-	-	0.0	0.0	100.0	-	-	0.0	99.8	0.2	-	-	1.5	97.6	0.9	-		-
Total %	0.2	0.0	0.1	-	0.3	0.0	0.0	0.7	-	0.7	0.0	54.6	0.1	-	54.7	0.7	43.3	0.4	-	44.3	-
Lights	1	0	1	-	2	0	0	5	-	5	0	564	0	-	564	6	442	2	-	450	1021
% Lights	50.0	-	100.0	-	66.7	-	-	71.4	-	71.4	-	98.8	0.0	-	98.6	85.7	97.8	50.0	-	97.2	97.7
Other Vehicles	1	0	0	-	1	0	0	2	-	2	0	7	1	-	8	1	10	2	-	13	24
% Other Vehicles	50.0	-	0.0	-	33.3	-	-	28.6	-	28.6	-	1.2	100.0	-	1.4	14.3	2.2	50.0	-	2.8	2.3
Pedestrians	-	-	-	4	-	-	-	-	0	-	-	-	-	1	-	-	-	-	0		-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-



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Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 3

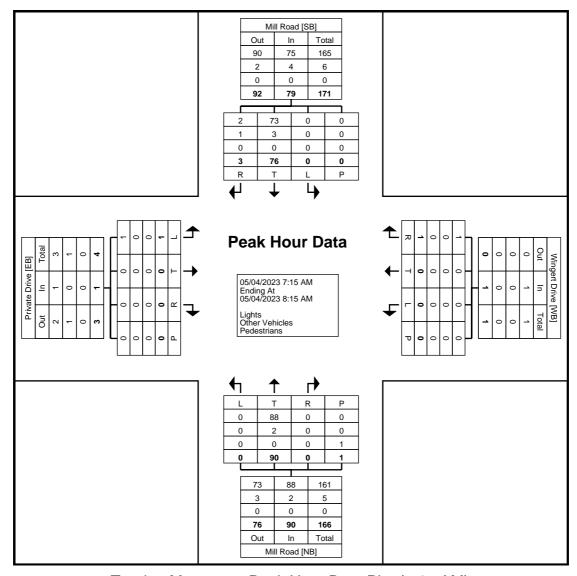
Turning Movement Peak Hour Data (7:15 AM)

						:		0				. – ~	·~ (.		,						
		Pr	rivate Dri	ve			W	ingert Dr	ive				Mill Road	t				Mill Road	t		
		E	astboun	d		1	V	Vestbour	nd			N	orthbour	nd			S	outhbour	nd		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
7:15 AM	1	0	0	0	1	0	0	1	0	1	0	29	0	1	29	0	11	2	0	13	44
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22	0	24	0	0	24	46
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22	0	23	1	0	24	46
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	17	0	0	17	0	18	0	0	18	35
Total	1	0	0	0	1	0	0	1	0	1	0	90	0	1	90	0	76	3	0	79	171
Approach %	100.0	0.0	0.0	-	-	0.0	0.0	100.0	-	-	0.0	100.0	0.0	-	-	0.0	96.2	3.8	-	-	
Total %	0.6	0.0	0.0	-	0.6	0.0	0.0	0.6	-	0.6	0.0	52.6	0.0	-	52.6	0.0	44.4	1.8	-	46.2	-
PHF	0.250	0.000	0.000	-	0.250	0.000	0.000	0.250	-	0.250	0.000	0.776	0.000	-	0.776	0.000	0.792	0.375	-	0.823	0.929
Lights	1	0	0	-	1	0	0	1	-	1	0	88	0	-	88	0	73	2	-	75	165
% Lights	100.0	-	-	-	100.0	-	-	100.0	-	100.0	-	97.8	-	-	97.8	-	96.1	66.7	-	94.9	96.5
Other Vehicles	0	0	0	-	0	0	0	0	-	0	0	2	0	-	2	0	3	1	-	4	6
% Other Vehicles	0.0	-	-	-	0.0	-	-	0.0	-	0.0	-	2.2	-	-	2.2	-	3.9	33.3	-	5.1	3.5
Pedestrians	-	-	-	0	_	-	_	-	0	_	-	-	_	1	-	-	-	-	0	-	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-



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Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:15 AM)



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Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 5

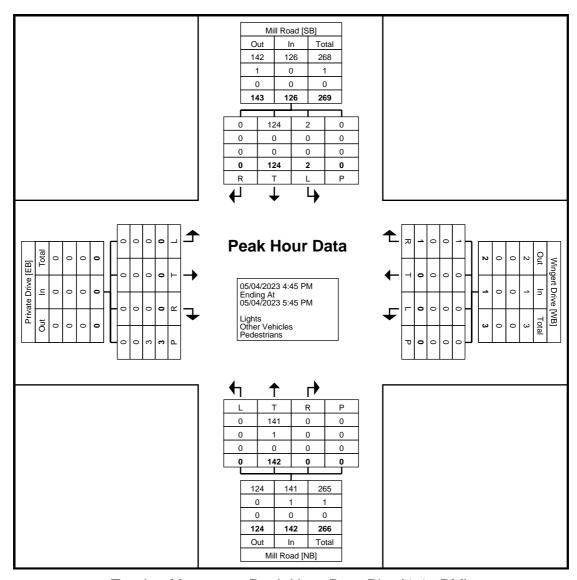
Turning Movement Peak Hour Data (4:45 PM)

		Pr	ivate Dri	ve]	Wi	ingert Dri	ve				Mill Road	l	,		1	Mill Road	ļ		
		E	astboun	d			V	Vestboun	d			N	orthbour	ıd			S	outhbour	nd		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	28	0	0	28	0	16	0	0	16	44
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	36	0	0	36	1	41	0	0	42	78
5:15 PM	0	0	0	0	0	0	0	1	0	1	0	40	0	0	40	0	36	0	0	36	77
5:30 PM	0	0	0	3	0	0	0	0	0	0	0	38	0	0	38	1	31	0	0	32	70
Total	0	0	0	3	0	0	0	1	0	1	0	142	0	0	142	2	124	0	0	126	269
Approach %	0.0	0.0	0.0	-	-	0.0	0.0	100.0	-	-	0.0	100.0	0.0	-	-	1.6	98.4	0.0	-	-	-
Total %	0.0	0.0	0.0	-	0.0	0.0	0.0	0.4	-	0.4	0.0	52.8	0.0	-	52.8	0.7	46.1	0.0	-	46.8	-
PHF	0.000	0.000	0.000	-	0.000	0.000	0.000	0.250	-	0.250	0.000	0.888	0.000	-	0.888	0.500	0.756	0.000	-	0.750	0.862
Lights	0	0	0	-	0	0	0	1	-	1	0	141	0	-	141	2	124	0	-	126	268
% Lights	•	-	-	-	-	-	-	100.0	-	100.0	ı	99.3	-	-	99.3	100.0	100.0	-	-	100.0	99.6
Other Vehicles	0	0	0	-	0	0	0	0	-	0	0	1	0	-	1	0	0	0	-	0	1
% Other Vehicles	-	-	-	-	-	-	-	0.0	-	0.0	-	0.7	-	-	0.7	0.0	0.0	-	-	0.0	0.4
Pedestrians	-	-	-	3	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	100.0	-	-	_	-	-	-	-	-	-	-		-	-	_	-	-	-



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Count Name: (3) AM/PM Mill Road x Wingert Drive Site Code: Start Date: 05/04/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:45 PM)



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 1

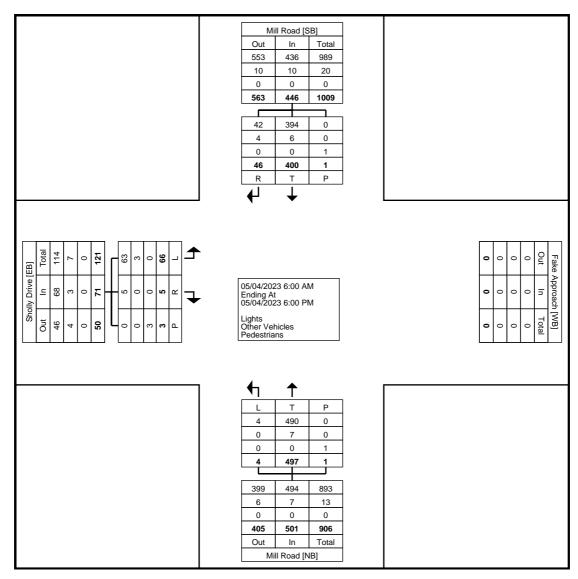
Turning Movement Data

				ΙŲ	ımıng ı	vioven	ient D	ala					
		Sholly	Drive			Mill	Road			Mill	Road		
Ctort Time		Easth	ound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
6:00 AM	0	0	0	0	0	8	0	8	1	0	0	. 1	9
6:15 AM	1	0	0	1	0	7	0	7	5	1	0	6	14
6:30 AM	2	0	0	2	0	7	0	7	1	0	0	1	10
6:45 AM	5	0	0	5	0	12	0	12	7	1	0	8	25
Hourly Total	8	0	0	8	0	34	0	34	14	2	0	16	58
7:00 AM	3	0	0	3	0	17	0	17	6	0	0	6	26
7:15 AM	5	0	1	5	0	22	0	22	11	0	0	11	38
7:30 AM	7	0	0	7	0	18	0	18	24	2	0	26	51
7:45 AM	5	1	0	6	0	13	0	13	21	3	0	24	43
Hourly Total	20	1	1	21	0	70	0	70	62	5	0	67	158
8:00 AM	2	0	0	2	1	13	0	14	11	4	0	15	31
8:15 AM	4	0	0	4	0	16	0	16	15	0	0	15	35
8:30 AM	2	0	0	2	0	16	0	16	12	3	0	15	33
8:45 AM	6	3	0	9	2	15	0	17	12	2	0	14	40
Hourly Total	14	3	0	17	3	60	0	63	50	9	0	59	139
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	0	0	2	0	0	19	0	19	20	3	0	23	42
3:15 PM	0	0	0	0	0	21	0	21	25	3	0	28	49
3:30 PM	1	0	0	1	0	31	1	31	24	2	0	26	58
3:45 PM	3	0	0	3	0	26	0	26	13	2	0	15	44
Hourly Total	4	0	2	4	0	97	1	97	82	10	0	92	193
4:00 PM	3	0	0	3	1	22	0	23	26	6	0	32	58
4:15 PM	3	1	0	4	0	34	0	34	26	1	0	27	65
4:30 PM	3	0	0	3	0	28	0	28	17	2	0	19	50
4:45 PM	1	0	0	1	0	28	0	28	16	2	0	18	47
Hourly Total	10	1	0	11	1	112	0	113	85	11	0	96	220
5:00 PM	3	0	0	3	0	26	0	26	37	3	0	40	69
5:15 PM	2	0	0	2	0	47	0	47	32	2	1	34	83
5:30 PM	3	0	0	3	0	28	0	28	31	3	0	34	65
5:45 PM	2	0	0	2	0	23	0	23	7	1	0	8	33
Hourly Total	10	0	0	10	0	124	0	124	107	9	1	116	250
Grand Total	66	5	3	71	4	497	1	501	400	46	1	446	1018
Approach %	93.0	7.0	-	-	0.8	99.2	-	-	89.7	10.3	-	-	-
Total %	6.5	0.5	-	7.0	0.4	48.8	-	49.2	39.3	4.5	-	43.8	-
Lights	63	5	-	68	4	490	-	494	394	42	-	436	998
% Lights	95.5	100.0	-	95.8	100.0	98.6	-	98.6	98.5	91.3	-	97.8	98.0
Other Vehicles	3	0	-	3	0	7	-	7	6	4	-	10	20
% Other Vehicles	4.5	0.0	-	4.2	0.0	1.4	-	1.4	1.5	8.7	-	2.2	2.0
Pedestrians	-	-	3	-	-	-	1	-	-	-	1	-	-
% Pedestrians	-	-	100.0	-	-	-	100.0	-	-	-	100.0	-	-



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 3

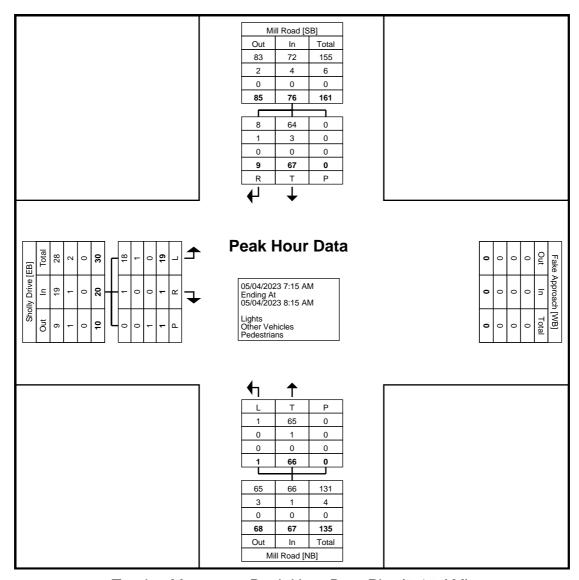
Turning Movement Peak Hour Data (7:15 AM)

	i							١,	,				
		Sholly	Drive			Mill F	Road			Mill F	Road		
Start Time		Easth	oound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
7:15 AM	5	0	1	5	0	22	0	22	11	0	0	11	38
7:30 AM	7	0	0	7	0	18	0	18	24	2	0	26	51
7:45 AM	5	1	0	6	0	13	0	13	21	3	0	24	43
8:00 AM	2	0	0	2	1	13	0	14	11	4	0	15	31
Total	19	1	1	20	1	66	0	67	67	9	0	76	163
Approach %	95.0	5.0	-	-	1.5	98.5	-	-	88.2	11.8	-	-	-
Total %	11.7	0.6	-	12.3	0.6	40.5	-	41.1	41.1	5.5	-	46.6	-
PHF	0.679	0.250	-	0.714	0.250	0.750	-	0.761	0.698	0.563	-	0.731	0.799
Lights	18	1	-	19	1	65	-	66	64	8	-	72	157
% Lights	94.7	100.0	-	95.0	100.0	98.5	-	98.5	95.5	88.9	-	94.7	96.3
Other Vehicles	1	0	-	1	0	1	-	1	3	1	-	4	6
% Other Vehicles	5.3	0.0	-	5.0	0.0	1.5	-	1.5	4.5	11.1	-	5.3	3.7
Pedestrians	-	-	1	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	-	-	-



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:15 AM)



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 5

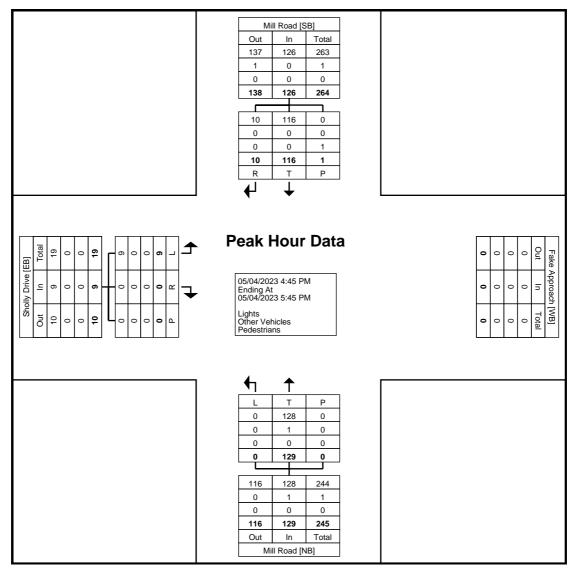
Turning Movement Peak Hour Data (4:45 PM)

		Sholly	Drive	Ŭ		Mill F	Road	`	,		Road		
		Eastb	oound			North	bound			South	bound		
Start Time	Left	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
4:45 PM	1	0	0	1	0	28	0	28	16	2	0	18	47
5:00 PM	3	0	0	3	0	26	0	26	37	3	0	40	69
5:15 PM	2	0	0	2	0	47	0	47	32	2	1	34	83
5:30 PM	3	0	0	3	0	28	0	28	31	3	0	34	65
Total	9	0	0	9	0	129	0	129	116	10	1	126	264
Approach %	100.0	0.0	-	-	0.0	100.0	-	-	92.1	7.9	-	-	-
Total %	3.4	0.0	-	3.4	0.0	48.9	-	48.9	43.9	3.8	-	47.7	-
PHF	0.750	0.000	-	0.750	0.000	0.686	-	0.686	0.784	0.833	-	0.788	0.795
Lights	9	0	-	9	0	128	-	128	116	10	-	126	263
% Lights	100.0	-	-	100.0	-	99.2	-	99.2	100.0	100.0	-	100.0	99.6
Other Vehicles	0	0	-	0	0	1	-	1	0	0	-	0	1
% Other Vehicles	0.0	-	-	0.0	-	0.8	-	0.8	0.0	0.0	-	0.0	0.4
Pedestrians	-	-	0	-	-	-	0	-	1	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-		-	-	100.0	-	-



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Count Name: (4) AM/PM Mill Road x Sholly Drive Site Code: Start Date: 05/04/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:45 PM)

APPENDIX C

VOLUME DEVELOPMENT WORKSHEETS

		W Lisbu	ırn Roa	ad (SR	2004)	and M	ill Road	d		
1	Adjacent intersections:	West	0	East	0	North	0	South	0	

Time Period: Weekday A.M. Peak Hour

	E	Eastbour	nd	W	estbour/	nd	N-	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	10	216	77	3	235	0	94	2	1	0	1	44	683
Balancing													0
2023 Existing Volumes (Balanced)	10	216	77	3	235	0	94	2	1	0	1	44	683
2026 Base growth (0.59% compounded for 3 yrs)	0	4	1	0	4	0	2	0	0	0	0	1	12
2036 Base growth (0.59% compounded for 13 yrs)	1	17	6	0	19	0	7	0	0	0	0	3	53
													0
151-237 Gettysburg Pike		3			4								7
													0
													0
													0
													0
Total Nearby Developments	0	3	0	0	4	0	0	0	0	0	0	0	7
2026 Base (No-Build) Volumes	10	223	78	3	243	0	96	2	1	0	1	45	702
2036 Base (No-Build) Volumes	11	236	83	3	258	0	101	2	1	0	1	47	743
Trip Distribution - New Trips							1		1				2
Total Trip Distribution	0	0	0	0	0	0	1	0	1	0	0	0	2
2026 Projected (Build) Volumes	10	223	78	3	243	0	97	2	2	0	1	45	704
2036 Projected (Build) Volumes	11	236	83	3	258	0	102	2	2	0	1	47	745

	E	astbour	d	V	/estbour	nd	N	orthbour	nd	Sc	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	28	283	123	7	411	Ő	125	2	9	3	2	39	1032
Balancing													0
2023 Existing Volumes (Balanced)	28	283	123	7	411	0	125	2	9	3	2	39	1032
2026 Base growth (0.59% compounded for 3 yrs)	0	5	2	0	7	0	2	0	0	0	0	1	17
2036 Base growth (0.59% compounded for 13 yrs)	2	22	10	1	33	0	10	0	1	0	0	3	82
0													0
151-237 Gettysburg Pike		3			3								6
0													0
0													0
0													0
0													0
Total Nearby Developments	0	3	0	0	3	0	0	0	0	0	0	0	6
2026 Base (No-Build) Volumes	28	291	125	7	421	0	127	2	9	3	2	40	1055
2036 Base (No-Build) Volumes	30	308	133	8	447	0	135	2	10	3	2	42	1120
0													
Trip Distribution - New Trips			2	1			1						4
Total Trip Distribution	0	0	2	1	0	0	1	0	0	0	0	0	4
2026 Projected (Build) Volumes	28	291	127	8	421	0	128	2	9	3	2	40	1059
2036 Projected (Build) Volumes	30	308	135	9	447	0	136	2	10	3	2	42	1124

Mill Road and Existing/Proposed Site Driveway

2 Adjacent intersections: West 0 East 0 North 0 South 0

Time Period: Weekday A.M. Peak Hour

		Eastbour	nd	W	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	0	0	0	0	0	0	0	93	0	2	80	0	175
Balancing													0
2023 Existing Volumes (Balanced)	0	0	0	0	0	0	0	93	0	2	80	0	175
2026 Base growth (0.59% compounded for 3 yrs)	0	0	0	0	0	0	0	2	0	0	1	0	3
2036 Base growth (0.59% compounded for 13 yrs)	0	0	0	0	0	0	0	7	0	0	6	0	13
													0
151-237 Gettysburg Pike													0
													0
													0
													0
													0
Total Nearby Developments	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	0	0	0	0	0	0	0	95	0	2	81	0	178
2036 Base (No-Build) Volumes	Ö	Ö	0	Ö	0	0	0	100	Ö	2	86	Ö	188
,													
Trip Distribution - New Trips						2							2
Total Trip Distribution	0	0	0	0	0	2	0	0	0	0	0	0	2
2026 Projected (Build) Volumes	0	0	0	0	0	2	0	95	0	2	81	0	180
2036 Projected (Build) Volumes	0	0	0	0	0	2	0	100	0	2	86	0	190

		Eastbou	ınd	٧	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	le	t thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	0	0	0	0	0	0	0	140	0	0	130	0	270
Balancing													0
2023 Existing Volumes (Balanced)	0	0	0	0	0	0	0	140	0	0	130	0	270
2026 Base growth (0.59% compounded for 3 yrs)	0	0	0	0	0	0	0	2	0	0	2	0	4
2036 Base growth (0.59% compounded for 13 yrs)	0	0	0	0	0	0	0	11	0	0	10	0	21
	0												0
151-237 Gettysburg Pike													0
	0												0
	0												0
	0												0
	0												0
Total Nearby Developments	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	0	0	0	0	0	0	0	142	0	0	132	0	274
2036 Base (No-Build) Volumes	0	0	0	0	0	0	0	151	0	0	140	0	291
	0												
Trip Distribution - New Trips						1				3			4
Total Trip Distribution	0	0	0	0	0	1	0	0	0	3	0	0	4
2026 Projected (Build) Volumes	0	0	0	0	0	1	0	142	0	3	132	0	278
2036 Projected (Build) Volumes	0	0	0	0	0	1	0	151	0	3	140	0	295

	N	/lill Roa	d and I	Private	Drive	Winge	rt Driv	e		
3	Adjacent intersections:	West	0	East	0	North	0	South	0	

Time Period: Weekday A.M. Peak Hour

		Eastbour	nd	٧	estbour/	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	1	0	0	0	0	1	0	90	0	0	76	3	171
Balancing													0
2023 Existing Volumes (Balanced)	1	0	0	0	0	1	0	90	0	0	76	3	171
2026 Base growth (0.59% compounded for 3 yrs)	0	0	0	0	0	0	0	2	0	0	1	0	3
2036 Base growth (0.59% compounded for 13 yrs)	0	0	0	0	0	0	0	7	0	0	6	0	13
													0
151-237 Gettysburg Pike													0
													0
													0
													0
													0
Total Nearby Developments	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	1	0	0	0	0	1	0	92	0	0	77	3	174
2036 Base (No-Build) Volumes	1	0	0	0	0	1	0	97	0	0	82	3	184
Trip Distribution - New Trips													0
Total Trip Distribution	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Projected (Build) Volumes	1	0	0	0	0	1	0	92	0	0	77	3	174
2036 Projected (Build) Volumes	1	0	0	0	0	1	0	97	0	0	82	3	184

		Eas	tbound	d	W	/estbour	nd	N ₁	orthbour	nd	S	outhbou	nd	Intersection
	le	eft t	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	(0	0	0	0	0	1	0	142	0	2	124	0	269
Balancing														0
2023 Existing Volumes (Balanced)	(0	0	0	0	0	1	0	142	0	2	124	0	269
2026 Base growth (0.59% compounded for 3 yrs)	(0	0	0	0	0	0	0	3	0	0	2	0	5
2036 Base growth (0.59% compounded for 13 yrs)	(0	0	0	0	0	0	0	11	0	0	10	0	21
	0													0
151-237 Gettysburg Pike														0
	0													0
	0													0
	0													0
	0													0
Total Nearby Developments	(0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	(0	0	0	0	0	1	0	145	0	2	126	0	274
2036 Base (No-Build) Volumes	(0	0	0	0	0	1	0	153	0	2	134	0	290
	0													
Trip Distribution - New Trips														0
Total Trip Distribution	(0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Projected (Build) Volumes	(0	0	0	0	0	1	0	145	0	2	126	0	274
2036 Projected (Build) Volumes		0	0	0	0	0	1	0	153	0	2	134	0	290

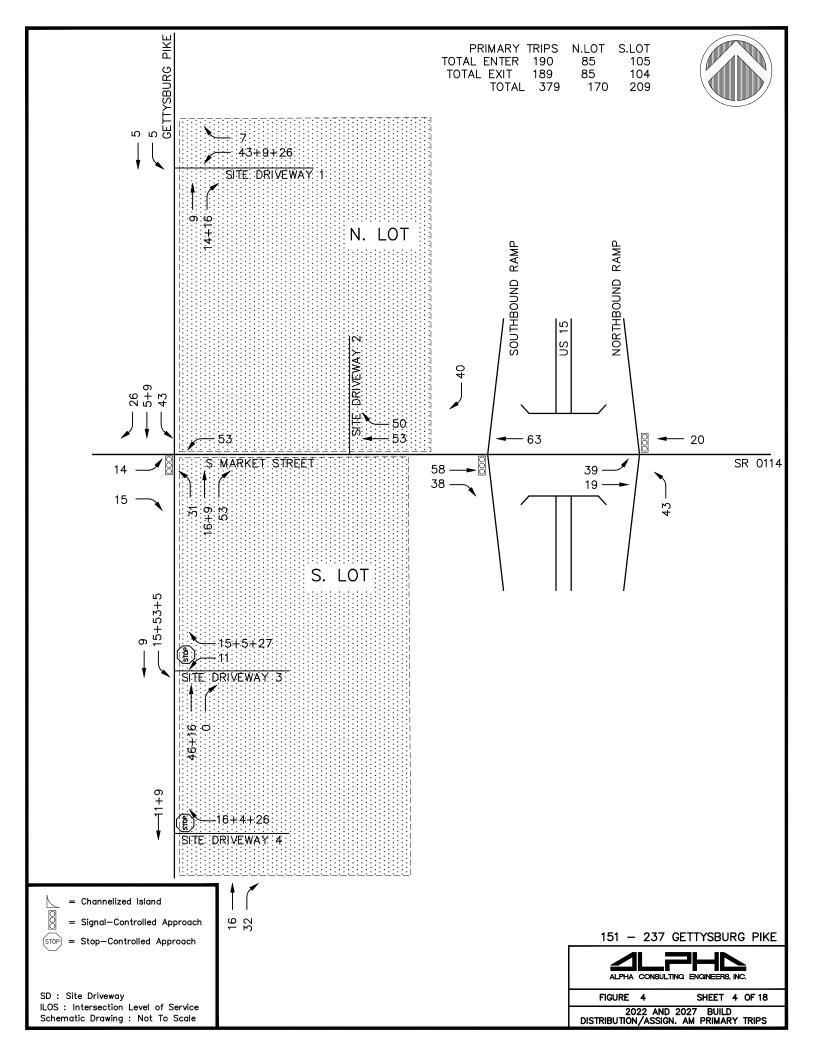
		N	/ill Ro	ad and	Sholly	/ Drive				
4	Adjacent intersections:	West	0	East	0	North	0	South	0	

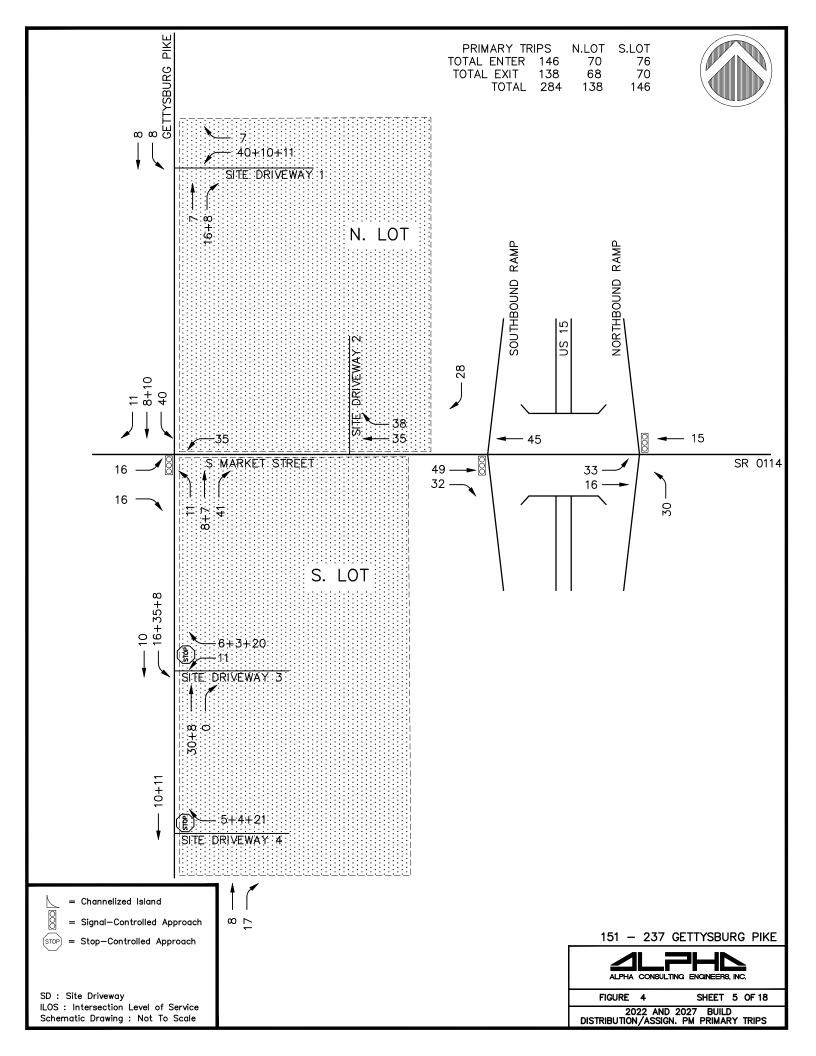
Time Period: Weekday A.M. Peak Hour

		Eastbour	nd	٧	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	19	0	1	0	0	0	1	66	0	0	67	9	163
Balancing													0
2023 Existing Volumes (Balanced)	19	0	1	0	0	0	1	66	0	0	67	9	163
2026 Base growth (0.59% compounded for 3 yrs)	0	0	0	0	0	0	0	1	0	0	1	0	2
2036 Base growth (0.59% compounded for 13 yrs)	2	0	0	0	0	0	0	5	0	0	5	1	13
													0
151-237 Gettysburg Pike													0
													0
													0
													0
													0
Total Nearby Developments	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	19	0	1	0	0	0	1	67	0	0	68	9	165
2036 Base (No-Build) Volumes	21	0	1	0	0	0	1	71	0	0	72	10	176
Trip Distribution - New Trips	•												0
Total Trip Distribution	0	0	0	0	0	0	0	0	0	0	0	0	0
2026 Projected (Build) Volumes	19	0	1	0	0	0	1	67	0	0	68	9	165
2036 Projected (Build) Volumes	21	0	1	0	0	0	1	71	0	0	72	10	176

		East	bound		W	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	le	eft th	nru ri	ight	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Traffic Counts	9	•	0	0	0	0	0	0	129	0	0	116	10	264
Balancing														0
2023 Existing Volumes (Balanced)	9)	0	0	0	0	0	0	129	0	0	116	10	264
2026 Base growth (0.59% compounded for 3 yrs)	()	0	0	0	0	0	0	2	0	0	2	0	4
2036 Base growth (0.59% compounded for 13 yrs)	1	1	0	0	0	0	0	0	10	0	0	9	1	21
	0													0
151-237 Gettysburg Pike														0
	0													0
	0													0
	0													0
	0													0
Total Nearby Developments	()	0	0	0	0	0	0	0	0	0	0	0	0
2026 Base (No-Build) Volumes	9	9	0	0	0	0	0	0	131	0	0	118	10	268
2036 Base (No-Build) Volumes	1	0	0	0	0	0	0	0	139	0	0	125	11	285
	0													
Trip Distribution - New Trips														0
Total Trip Distribution	()	0	0	0	0	0	0	0	0	0	0	0	0
2026 Projected (Build) Volumes	9	9	0	0	0	0	0	0	131	0	0	118	10	268
2036 Projected (Build) Volumes	1	0	0	0	0	0	0	0	139	0	0	125	11	285

APPENDIX D NEARBY DEVELOPMENT





APPENDIX ECAPACITY ANALYSES

Mill Road & Existing/Projected Site Driveway

Crititcal Headway

			tc base	tc hv	phv	t cg	G	t 3lt	Base Crit
major left	AM	SB L	4.3	1	0%	0	-3	0	4.3
major iert	PM	SB L	4.3	1	0%	0	-3	0	4.3
minor right	AM	WB R	6.2	1	0%	0.1	6	0	6.8
Illilloi ligit	PM	WB R	6.2	1	0%	0.1	6	0	6.8
minor left	AM	WB L	7.1	1	0%	0.2	6	0.7	7.6
minor left	PM	WB L	7.1	1	0%	0.2	6	0.7	7.6

Follow-up headway

			t fbase	t fhv	phv	Follow-up
major left	AM	SB L	3	0.9	0%	3.0
major left	PM	SB L	3	0.9	0%	3.0
minor right	AM	WB R	3.1	0.9	0%	3.1
THITIOI TIGHT	PM	WB R	3.1	0.9	0%	3.1
minor left	AM	WB L	3	0.9	0%	3.0
minor left	PM	WB L	3	0.9	0%	3.0

Mill Road & Wingert Drive

Crititcal Headway

		Movement	tc base	tc hv	phv	t cg	G	t 3lt	Base Crit
	AM	NB L	4.3	1	0%	0	6	0	4.3
masiar laft	PM	NB L	4.3	1	0%	0	6	0	4.3
major left	AM	SB L	4.3	1	0%	0	-4	0	4.3
	PM	SB L	4.3	1	0%	0	-4	0	4.3
	AM	EB R	6.2	1	0%	0.1	1	0	6.3
minor right	PM	EB R	6.2	1	0%	0.1	1	0	6.3
minor right	AM	WB R	6.2	1	0%	0.1	-3	0	5.9
	PM	WB R	6.2	1	0%	0.1	-3	0	5.9
	AM	EB L	7.1	1	0%	0.2	1	0	7.3
minor left	PM	EB L	7.1	1	0%	0.2	1	0	7.3
minor left	AM	WB L	7.1	1	0%	0.2	-3	0	6.5
	PM	WB L	7.1	1	0%	0.2	-3	0	6.5

Follow-up headway

	•	Movement	t fbase	t fhv	phv	Follow-up
	AM	NB L	3	0.9	0%	3.0
masia a laft	PM	NB L	3	0.9	0%	3.0
major left	AM	SB L	3	0.9	0%	3.0
	PM	SB L	3	0.9	0%	3.0
	AM	EB R	3.1	0.9	0%	3.1
	PM	EB R	3.1	0.9	0%	3.1
minor right	AM	WB R	3.1	0.9	0%	3.1
	PM	WB R	3.1	0.9	0%	3.1
	AM	EB L	3	0.9	0%	3.0
minor left	PM	EB L	3	0.9	0%	3.0
minor left	AM	WB L	3	0.9	0%	3.0
	PM	WB L	3	0.9	0%	3.0

Mill Road & Sholly Drive

Crititcal Headway

			tc base	tc hv	phv	t cg	G	t 3lt	Base Crit
major loft	AM	NB L	4.3	1	0%	0	-1	0	4.3
major left	PM	NB L	4.3	1	0%	0	-1	0	4.3
minor right	AM	EB R	6.2	1	0%	0.1	-1	0	6.1
minor right	PM	EB R	6.2	1	0%	0.1	-1	0	6.1
minor left	AM	EB L	7.1	1	5%	0.2	-1	0.7	6.3
minor left	PM	EB L	7.1	1	0%	0.2	-1	0.7	6.2

Follow-up headway

			t fbase	t fhv	phv	Follow-up
major loft	AM	NB L	3	0.9	0%	3.0
major left	PM	NB L	3	0.9	0%	3.0
minor right	AM	EB R	3.1	0.9	0%	3.1
minor right	PM	EB R	3.1	0.9	0%	3.1
minor left	AM	EB L	3	0.9	5%	3.0
minor iert	PM	EB L	3	0.9	0%	3.0

2023 EXISTING CONDITIONS

	•	→	•	•	←	•	•	†	/	\	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	10	216	77	3	235	0	94	2	1	0	1	44
Future Volume (vph)	10	216	77	3	235	0	94	2	1	0	1	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)	1					1						
Confl. Bikes (#/hr)												
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	20%	6%	1%	33%	6%	0%	3%	0%	100%	0%	100%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	337	0	0	264	0	0	107	0	0	1	49
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

intersection Summary

Area Type:

Other

Intersection	
Intersection Delay, s/veh	11.4
Intersection Delay, s/veh Intersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ની	7
Traffic Vol, veh/h	10	216	77	3	235	0	94	2	1	0	1	44
Future Vol, veh/h	10	216	77	3	235	0	94	2	1	0	1	44
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	20	6	1	33	6	0	3	0	100	0	100	0
Mvmt Flow	11	240	86	3	261	0	104	2	1	0	1	49
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			2				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	2			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			2			1				1	
HCM Control Delay	12			11.8			10.1				8.7	
HCM LOS	В			В			В				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	97%	3%	1%	0%	0%
Vol Thru, %	2%	71%	99%	100%	0%
Vol Right, %	1%	25%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	97	303	238	1	44
LT Vol	94	10	3	0	0
Through Vol	2	216	235	1	0
RT Vol	1	77	0	0	44
Lane Flow Rate	108	337	264	1	49
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.173	0.456	0.39	0.002	0.075
Departure Headway (Hd)	5.795	4.876	5.305	7.978	5.538
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	612	734	672	451	651
Service Time	3.895	2.945	3.381	5.678	3.238
HCM Lane V/C Ratio	0.176	0.459	0.393	0.002	0.075
HCM Control Delay	10.1	12	11.8	10.7	8.7
HCM Lane LOS	В	В	В	В	Α
HCM 95th-tile Q	0.6	2.4	1.9	0	0.2

	•	•	†	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1			4
Traffic Volume (vph)	0	0	93	0	2	80
Future Volume (vph)	0	0	93	0	2	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	4%	0%	0%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	102	0	0	90
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\AM Files.syn

Intersection				_		
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		WDR		INDIX	ODL	
Lane Configurations	¥	0	∱	0	0	€
Traffic Vol, veh/h	0	0	93	0	2	80
Future Vol, veh/h	0	0	93	0	2	80
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	0	4	0	0	6
Mvmt Flow	0	0	102	0	2	88
NA - 1 (NA1	A'		4.1.4		4.1.0	
	Minor1		//ajor1		Major2	
Conflicting Flow All	194	102	0	0	102	0
Stage 1	102	-	-	-	-	-
Stage 2	92	-	-	-		-
Critical Hdwy	7.6	6.8	-	-	4.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	6.6	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	863	1000	-	-	1108	-
Stage 1	1038	-	_	_	_	_
Stage 2	1053	_	-	_	_	-
Platoon blocked, %	1000		_	_		_
Mov Cap-1 Maneuver	861	1000		_	1108	
Mov Cap-1 Maneuver	861	1000	_	_	1100	_
	1038	<u>-</u>	-	<u>-</u>	_	<u>-</u>
Stage 1		-	-	-	-	-
Stage 2	1051	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0.2	
HCM LOS	A		U		0.2	
I IOIVI LOS	A					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	_	1108	-
HCM Lane V/C Ratio		_	_		0.002	_
HCM Control Delay (s)		_	_	0	8.3	0
HCM Lane LOS			_	A	Α	A
HCM 95th %tile Q(veh)		-		- -	0	- -
How som while Q(ven)		-	_	_	U	_

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	1	0	0	0	0	1	0	90	0	0	76	3
Future Volume (vph)	1	0	0	0	0	1	0	90	0	0	76	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)			1	1								
Confl. Bikes (#/hr)												
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	4%	33%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	1	0	0	97	0	0	85	0
Sign Control Intersection Summary		Stop			Stop			Free			Free	

Intersection Summary

Area Type:

Control Type: Unsignalized

Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	0	90	0	0	76	3
Future Vol, veh/h	1	0	0	0	0	1	0	90	0	0	76	3
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	4	33
Mvmt Flow	1	0	0	0	0	1	0	97	0	0	82	3
Major/Minor N	Minor2		ľ	Minor1		1	Major1		N	//ajor2		
Conflicting Flow All	182	181	85	182	182	97	85	0	0	97	0	0
Stage 1	84	84	-	97	97	-	-	-	-	-	_	-
Stage 2	98	97	_	85	85	-	_	_	_	_	-	_
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	_	_	4.3	_	_
Critical Hdwy Stg 1	6.3	5.7	_	5.5	4.9	_	-	_	-	-	-	_
Critical Hdwy Stg 2	6.3	5.7	_	5.5	4.9	_	-	_	_	_	_	_
Follow-up Hdwy	3	4	3.1	3	4	3.1	3	-	-	3	-	-
Pot Cap-1 Maneuver	894	709	1038	931	738	1033	1123	-	-	1112	_	-
Stage 1	1073	825	_	1077	832	_	-	_	-	-	-	-
Stage 2	1053	814	-	1092	840	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	893	709	1037	930	738	1033	1123	-	-	1112	-	-
Mov Cap-2 Maneuver	893	709	-	930	738	-	-	-	-	-	-	-
Stage 1	1073	825	-	1077	832	-	-	-	-	-	-	-
Stage 2	1052	814	-	1091	840	-	-	-	-	-	-	-
Ğ												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9			8.5			0			0		
HCM LOS	A			A								
	, ,			, ,								
Minor Lane/Major Mvm	t	NBL	NBT	NRR	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1123	-			1033	1112		-			
HCM Lane V/C Ratio		-	_		0.001		-	_				
HCM Control Delay (s)		0	_	-	9	8.5	0	_	-			
HCM Lane LOS		A	_	-	A	6.5 A	A	-	_			
HCM 95th %tile Q(veh)		0		-	0	0	0	_	-			
How som while Q(ven)		U	-	_	U	U	U	-	-			

Control Type: Unsignalized

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	ĵ.	<u> </u>
Traffic Volume (vph)	19	1	1	66	67	9
Future Volume (vph)	19	1	1	66	67	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	2%	5%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	25	0	0	84	95	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\AM Files.syn

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Intersection						
Int Delay, s/veh	1.1					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		4	€	♣	•
Traffic Vol, veh/h	19	1	1	66	67	9
Future Vol, veh/h	19	1	1	66	67	9
Conflicting Peds, #/hr	0	0	_ 1	_ 0	_ 0	_ 1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	0	0	2	5	11
Mvmt Flow	24	1	1	83	84	11
Maiaa/Miaa	Min		1-1-4		1-1-0	
	Minor2		Major1		//ajor2	
Conflicting Flow All	176	91	96	0	-	0
Stage 1	91	-	-	-	-	-
Stage 2	85	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.25	-	-	-	-	-
Critical Hdwy Stg 2	5.25	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	948	1035	1113	-	-	-
Stage 1	1091	-	-	-	-	-
Stage 2	1098	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	945	1034	1112	-	_	-
Mov Cap 1 Maneuver	945	-	- 1.12	_	_	_
Stage 1	1089	_	_	_	_	_
Stage 2	1003	_		_		_
Staye 2	1037	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.1		0	
HCM LOS	Α				-	
	, ,					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1112	-	949	-	-
HCM Lane V/C Ratio		0.001	-	0.026	-	-
HCM Control Delay (s)	8.2	0	8.9	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh	1)	0	-	0.1	-	-
	,					

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	28	283	123	7	411	0	125	2	9	3	2	39
Future Volume (vph)	28	283	123	7	411	0	125	2	9	3	2	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	462	0	0	444	0	0	145	0	0	5	41
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type:

Other

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	28	283	123	7	411	0	125	2	9	3	2	39
Future Vol, veh/h	28	283	123	7	411	0	125	2	9	3	2	39
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	1	0	0	1	0	1	0	0	0	0	0
Mvmt Flow	30	301	131	7	437	0	133	2	10	3	2	41
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			1			1		
HCM Control Delay	16.6			16.9			11.9			9.7		
HCM LOS	С			С			В			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2	
Vol Left, %	92%	6%	2%	60%	0%	
Vol Thru, %	1%	65%	98%	40%	0%	
Vol Right, %	7%	28%	0%	0%	100%	
Sign Control	Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	136	434	418	5	39	
LT Vol	125	28	7	3	0	
Through Vol	2	283	411	2	0	
RT Vol	9	123	0	0	39	
Lane Flow Rate	145	462	445	5	41	
Geometry Grp	5	2	2	7	7	
Degree of Util (X)	0.261	0.641	0.638	0.011	0.073	
Departure Headway (Hd)	6.498	5	5.165	7.38	6.355	
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	
Cap	551	720	698	483	561	
Service Time	4.554	3.038	3.203	5.148	4.123	
HCM Lane V/C Ratio	0.263	0.642	0.638	0.01	0.073	
HCM Control Delay	11.9	16.6	16.9	10.2	9.6	
HCM Lane LOS	В	С	С	В	Α	
HCM 95th-tile Q	1	4.7	4.6	0	0.2	

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)			4
Traffic Volume (vph)	0	0	140	0	0	130
Future Volume (vph)	0	0	140	0	0	130
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	1%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	169	0	0	157
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	b					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\PM Files.syn

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1>			4
Traffic Vol, veh/h	0	0	140	0	0	130
Future Vol, veh/h	0	0	140	0	0	130
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	6	<u>-</u>	3	_	<u>-</u>	-3
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	0	0	169	0	0	157
IVIVIIIL FIOW	U	U	109	U	U	157
Major/Minor N	Minor1	N	//ajor1	N	/lajor2	
Conflicting Flow All	326	169	0	0	169	0
Stage 1	169	-	-	-	-	-
Stage 2	157	-	-	-	-	-
Critical Hdwy	7.6	6.8	-	-	4.3	-
Critical Hdwy Stg 1	6.6	-	-	_	_	-
Critical Hdwy Stg 2	6.6	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	_	3	-
Pot Cap-1 Maneuver	689	907	-	_	1051	-
Stage 1	944	_	_	_	_	_
Stage 2	960	_	-	_	-	_
Platoon blocked, %	- 500		_	_		_
Mov Cap-1 Maneuver	689	907	_	_	1051	_
Mov Cap-1 Maneuver	689	- 501	_		-	_
Stage 1	944		-	_	_	_
Stage 2	960	_	_		_	_
Staye 2	900	-	_	-	<u>-</u>	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	.+	NBT	NRDV	VBLn1	SBL	SBT
		וטוו				
Capacity (veh/h)		-	-		1051	-
HCM Control Polov (a)		-	-	-	-	-
HCM Control Delay (s)		-	-	0	0	-
HCM Lane LOS		-	-	Α	A	-
HCM 95th %tile Q(veh)		-	-	-	0	-

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	1	0	142	0	2	124	0
Future Volume (vph)	0	0	0	0	0	1	0	142	0	2	124	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)							3					3
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	1	0	0	165	0	0	146	0
Sign Control Intersection Summary		Stop			Stop			Free			Free	

Intersection Summary

Area Type:

Other

Intersection Int Delay, s/veh O.1
Lane Configurations
Lane Configurations
Traffic Vol, veh/h
Future Vol, veh/h
Conflicting Peds, #/hr
Sign Control Stop Stop Stop Stop Stop Stop Stop Free Free
RT Channelized
Storage Length
Veh in Median Storage, # - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 - - 0 0 - - 0 0 - - 0
Grade, % - 1 - - -3 - - 6 -
Peak Hour Factor 86
Heavy Vehicles, %
Mynt Flow 0 0 0 0 1 0 165 0 2 144 0 Major/Minor Minor2 Minor1 Major1 Major2 Conflicting Flow All 317 316 147 313 316 165 147 0 0 165 0 0 Stage 1 151 151 - 165 165 -
Major/Minor Minor2 Minor1 Major1 Major2 Conflicting Flow All 317 316 147 313 316 165 147 0 0 165 0 0 Stage 1 151 151 - 165 165 -
Conflicting Flow All 317 316 147 313 316 165 147 0 0 165 0 0 Stage 1 151 151 - 165 165 Stage 2 166 165 - 148 151
Conflicting Flow All 317 316 147 313 316 165 147 0 0 165 0 0 Stage 1 151 151 - 165 165 Stage 2 166 165 - 148 151
Stage 1 151 151 - 165 165
Stage 2 166 165 - 148 151 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -
Critical Hdwy 7.3 6.7 6.3 6.5 5.9 5.9 4.3 - 4.3 - - Critical Hdwy Stg 1 6.3 5.7 - 5.5 4.9 -
Critical Hdwy Stg 1 6.3 5.7 - 5.5 4.9
Critical Hdwy Stg 2 6.3 5.7 - 5.5 4.9
Follow-up Hdwy 3 4 3.1 3 4 3.1 3 - 3 - 3 - Pot Cap-1 Maneuver 718 593 956 775 636 951 1070 - 1055 - Stage 1 981 770 - 998 787 Stage 2 961 759 - 1017 796 Platoon blocked, %
Pot Cap-1 Maneuver 718 593 956 775 636 951 1070 - - 1055 - - Stage 1 981 770 - 998 787 - <t< td=""></t<>
Stage 1 981 770 - 998 787 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -
Stage 2 961 759 - 1017 796
Platoon blocked, % Mov Cap-1 Maneuver 714 590 953 773 633 951 1067 - 1055 - Mov Cap-2 Maneuver 714 590 - 773 633 Stage 1 978 766 - 998 787 Stage 2 960 759 - 1015 792 Approach EB WB NB SB HCM Control Delay, s 0 8.8 0 0.1 HCM LOS A A
Mov Cap-1 Maneuver 714 590 953 773 633 951 1067 - - 1055 - - Mov Cap-2 Maneuver 714 590 - 773 633 -
Mov Cap-2 Maneuver 714 590 - 773 633 - </td
Stage 1 978 766 - 998 787 -
Stage 2 960 759 - 1015 792
Approach EB WB NB SB HCM Control Delay, s 0 8.8 0 0.1 HCM LOS A A
HCM Control Delay, s 0 8.8 0 0.1 HCM LOS A A
HCM LOS A A
HCM LOS A A
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR
Capacity (veh/h) 1067 951 1055
HCM Lane V/C Ratio 0.001 0.002
HCM Control Delay (s) 0 0 8.8 8.4 0 -
HCM Lane LOS A A A A A -
HCM 95th %tile Q(veh) 0 0 0

	•	•	1	†	↓	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	f)	
Traffic Volume (vph)	9	0	0	129	116	10
Future Volume (vph)	9	0	0	129	116	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)	1					
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	11	0	0	161	158	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\PM Files.syn

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EDI	INDL			SDN
Lane Configurations	Y	٥	٥	વ	116	10
Traffic Vol, veh/h	9	0	0	129	116	10
Future Vol, veh/h	9	0	0	129	116	10
Conflicting Peds, #/hr	1	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	0	0	0	1	0	0
Mvmt Flow	11	0	0	161	145	13
Major/Minor	Minor2	ı	laior1		/aiar2	
			Major1		/lajor2	
Conflicting Flow All	314	152	158	0	-	0
Stage 1	152	-	-	-	-	-
Stage 2	162	-	-	-	-	-
Critical Hdwy	6.2	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.2	-	-	-	-	-
Critical Hdwy Stg 2	5.2	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	794	958	1060	-	-	-
Stage 1	1026	-	-	-	-	-
Stage 2	1015	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	794	958	1060	-	-	-
Mov Cap-2 Maneuver	794	_	-	_	_	-
Stage 1	1026	-	_	-	_	-
Stage 2	1015	_	_	_	_	_
	. 5 . 5					
Approach	EB		NB		SB	
HCM Control Delay, s	9.6		0		0	
HCM LOS	Α					
Minor Long/Major Maria	o.t	NDI	NDT	EDI 51	CDT	CDD
Minor Lane/Major Mvn	TIC .	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1060	-		-	-
HCM Lane V/C Ratio		-	-	0.014	-	-
HCM Control Delay (s)	0	-	9.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh	1)	0	-	0	-	-

2026 BASE CONDITIONS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	10	223	78	3	243	0	96	2	1	0	1	45
Future Volume (vph)	10	223	78	3	243	0	96	2	1	0	1	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)	1					1						
Confl. Bikes (#/hr)												
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	20%	6%	1%	33%	6%	0%	3%	0%	100%	0%	100%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	346	0	0	273	0	0	110	0	0	1	50
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type: Other
Control Type: Unsignalized

ntersection	
ntersection Delay, s/veh	11.7
ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	10	223	78	3	243	0	96	2	1	0	1	45
Future Vol, veh/h	10	223	78	3	243	0	96	2	1	0	1	45
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	20	6	1	33	6	0	3	0	100	0	100	0
Mvmt Flow	11	248	87	3	270	0	107	2	1	0	1	50
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			2				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	2			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			2			1				1	
HCM Control Delay	12.3			12.1			10.3				8.8	
HCM LOS	В			В			В				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	97%	3%	1%	0%	0%
Vol Thru, %	2%	72%	99%	100%	0%
Vol Right, %	1%	25%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	99	311	246	1	45
LT Vol	96	10	3	0	0
Through Vol	2	223	243	1	0
RT Vol	1	78	0	0	45
Lane Flow Rate	110	346	273	1	50
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.182	0.471	0.405	0.002	0.078
Departure Headway (Hd)	5.949	4.903	5.33	8.041	5.599
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	606	726	667	448	644
Service Time	3.952	2.986	3.421	5.744	3.303
HCM Lane V/C Ratio	0.182	0.477	0.409	0.002	0.078
HCM Control Delay	10.3	12.3	12.1	10.8	8.8
HCM Lane LOS	В	В	В	В	Α
HCM 95th-tile Q	0.7	2.5	2	0	0.3

Lane Group WBL WBR NBT NBR SBL SBT Lane Configurations ↑
Traffic Volume (vph) 0 0 95 0 2 81 Future Volume (vph) 0 0 95 0 2 81 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Width (ft) 10 10 11 11 11 11
Traffic Volume (vph) 0 0 95 0 2 81 Future Volume (vph) 0 0 95 0 2 81 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Width (ft) 10 10 11 11 11 11
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 Lane Width (ft) 10 10 11 11 11 11
Lane Width (ft) 10 10 11 11 11 11
\ /
Grade (%) 6% 3% -3%
5.54.5 (7.5)
Storage Length (ft) 0 0 0
Storage Lanes 1 0 0 0
Taper Length (ft) 25 25
Link Speed (mph) 25 25
Link Distance (ft) 387 260 368
Travel Time (s) 10.6 7.1 10.0
Confl. Peds. (#/hr)
Confl. Bikes (#/hr)
Peak Hour Factor 0.91 0.91 0.91 0.91 0.91 0.91
Growth Factor 100% 100% 100% 100% 100% 100%
Heavy Vehicles (%) 0% 0% 4% 0% 0% 6%
Bus Blockages (#/hr) 0 0 0 0 0
Parking (#/hr)
Mid-Block Traffic (%) 0% 0%
Shared Lane Traffic (%)
Lane Group Flow (vph) 0 0 104 0 0 91
Sign Control Stop Free Free
Intersection Summary
Area Type: Other

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩.	אטוי	1\ B1	אפא	ODL	<u>₽</u>
Traffic Vol, veh/h	0	0	95	0	2	81
Future Vol, veh/h	0	0	95	0	2	81
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	riee -	None		None
Storage Length	0	None -	-	None -	_	None -
Veh in Median Storage			0	-	-	0
		-				-3
Grade, %	6	- 01	3	- 01	- 01	
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	0	4	0	0	6
Mvmt Flow	0	0	104	0	2	89
Major/Minor	Minor1	N	//ajor1	N	/lajor2	
Conflicting Flow All	197	104	0	0	104	0
Stage 1	104	-	-	-	-	-
Stage 2	93	_	_	_	_	_
Critical Hdwy	7.6	6.8	_	_	4.3	_
Critical Hdwy Stg 1	6.6	0.0	_	_	-7.0	_
Critical Hdwy Stg 2	6.6	_	_	<u>-</u>	-	-
Follow-up Hdwy	3	3.1	_	-	3	_
Pot Cap-1 Maneuver	858	998	_		1106	
	1035	330	-	-	1100	-
Stage 1		-	-	-	-	-
Stage 2	1052	-	-	-	-	-
Platoon blocked, %	0=5	000	-	-	4400	-
Mov Cap-1 Maneuver	856	998	-	-	1106	-
Mov Cap-2 Maneuver	856	-	-	-	-	-
Stage 1	1035	-	-	-	-	-
Stage 2	1050	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0.2	
HCM LOS	A				J.L	
1.0111 200	, \					
NA' 1 (NA - ' - NA		NET	NDD	MDL 4	051	ODT
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1106	-
HCM Lane V/C Ratio		-	-	-	0.002	-
HCM Control Delay (s)	-	-	0	8.3	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh	1)	-	-	-	0	-
212 72 27 27 (10)	1					

	•	→	•	•	←	•	•	†	/	>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	1	0	0	0	0	1	0	92	0	0	77	3
Future Volume (vph)	1	0	0	0	0	1	0	92	0	0	77	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)			1	1								
Confl. Bikes (#/hr)												
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	4%	33%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	1	0	0	99	0	0	86	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Area Type:

Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LUL	4	LDIN	VVDL	4	אוטוע	NDL	4	NUN	ODL	4	ODIN
Traffic Vol, veh/h	1	0	0	0	0	1	0	92	0	0	77	3
Future Vol, veh/h	1	0	0	0	0	1	0	92	0	0	77	3
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	- Olop	- Otop	None	- Olop	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	.# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	, <i>''</i>	1	_	_	-3	_	_	6	_	_	-4	_
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	4	33
Mvmt Flow	1	0	0	0	0	1	0	99	0	0	83	3
		_										
Major/Minor N	Minor2		ı	Minor1		ı	Major1		N	//ajor2		
Conflicting Flow All	185	184	86	185	185	99	86	0	0	99	0	0
Stage 1	85	85	-	99	99	-	-	-	-	-	-	-
Stage 2	100	99	_	86	86	_	_	_	_	_	_	_
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	-	-	4.3	_	_
Critical Hdwy Stg 1	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Follow-up Hdwy	3	4	3.1	3	4	3.1	3	_	_	3	_	_
Pot Cap-1 Maneuver	890	707	1036	927	735	1030	1122	-	-	1111	-	-
Stage 1	1071	824	-	1075	831			_	_	-	-	_
Stage 2	1050	813	-	1090	839	-		-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	889	707	1035	926	735	1030	1122	-	-	1111	-	-
Mov Cap-2 Maneuver	889	707	-	926	735	-	-	-	-	-	-	-
Stage 1	1071	824	-	1075	831	-	-	-	-	-	-	-
Stage 2	1049	813	-	1089	839	-	-	-	-	-	-	-
Ü												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.1			8.5			0			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1122	-	-	889	1030	1111	-	-			
HCM Lane V/C Ratio		-	_		0.001		-	_	_			
HCM Control Delay (s)		0	-	-	9.1	8.5	0	-	-			
HCM Lane LOS		A	_	_	A	A	A	_	<u>-</u>			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			

Control Type: Unsignalized

	•	•	4	†	↓	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	ĵ.	
Traffic Volume (vph)	19	1	1	67	68	9
Future Volume (vph)	19	1	1	67	68	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	2%	5%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	25	0	0	85	96	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\AM Files.syn

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
	₩.	EBR	INDL			SDN
Lane Configurations		1	1	€	1 → 68	0
Traffic Vol, veh/h	19	1	1	67		9
Future Vol, veh/h	19	1	1	67	68	9
Conflicting Peds, #/hr		0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag		-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	0	0	2	5	11
Mvmt Flow	24	1	1	84	85	11
Major/Minor	Minor2	A	Jaior1		/laior2	
			Major1		Major2	
Conflicting Flow All	178	92	97	0	-	0
Stage 1	92	-	-	-	-	-
Stage 2	86	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.25	-	-	-	-	-
Critical Hdwy Stg 2	5.25	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	946	1034	1112	-	-	-
Stage 1	1090	-	-	-	-	-
Stage 2	1097	-	-	-	-	-
Platoon blocked, %				_	_	-
Mov Cap-1 Maneuver	943	1033	1111	-	_	-
Mov Cap-2 Maneuver		-		_	_	_
Stage 1	1088	_	_	_	_	_
Stage 2	1096			_		_
Staye 2	1030	-	-	-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.1		0	
HCM LOS	Α					
, <u></u>						
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1111	-	947	-	-
HCM Lane V/C Ratio		0.001		0.026	-	-
HCM Control Delay (s	5)	8.2	0	8.9	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh	ר)	0	-	0.1	-	-
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	28	291	125	7	421	0	127	2	9	3	2	40
Future Volume (vph)	28	291	125	7	421	0	127	2	9	3	2	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	473	0	0	455	0	0	147	0	0	5	43
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type:

Other

intersection												
Intersection Delay, s/veh	16.5											
Intersection LOS	С											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	28	291	125	7	421	0	127	2	9	3	2	40
Future Vol, veh/h	28	291	125	7	421	0	127	2	9	3	2	40
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	1	0	0	1	0	1	0	0	0	0	0
Mvmt Flow	30	310	133	7	448	0	135	2	10	3	2	43
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB			SB		

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	2	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	2	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	2	1	1
HCM Control Delay	17.4	17.8	12	9.8
HCM LOS	С	С	В	Α

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	92%	6%	2%	60%	0%
Vol Thru, %	1%	66%	98%	40%	0%
Vol Right, %	7%	28%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	138	444	428	5	40
LT Vol	127	28	7	3	0
Through Vol	2	291	421	2	0
RT Vol	9	125	0	0	40
Lane Flow Rate	147	472	455	5	43
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.268	0.661	0.658	0.011	0.076
Departure Headway (Hd)	6.561	5.036	5.201	7.452	6.427
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	546	718	692	478	554
Service Time	4.625	3.081	3.246	5.229	4.203
HCM Lane V/C Ratio	0.269	0.657	0.658	0.01	0.078
HCM Control Delay	12	17.4	17.8	10.3	9.7
HCM Lane LOS	В	С	С	В	Α
HCM 95th-tile Q	1.1	5	4.9	0	0.2

Control Type: Unsignalized

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĥ			ર્ન
Traffic Volume (vph)	0	0	142	0	0	132
Future Volume (vph)	0	0	142	0	0	132
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	1%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	171	0	0	159
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\PM Files.syn

Intersection						
Int Delay, s/veh	0					
		WIDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			स्
Traffic Vol, veh/h	0	0	142	0	0	132
Future Vol, veh/h	0	0	142	0	0	132
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	0	0	171	0	0	159
N.A. ' (N.A.	N 4'					
	Minor1		//ajor1		Major2	
Conflicting Flow All	330	171	0	0	171	0
Stage 1	171	-	-	-	-	-
Stage 2	159	-	-	-	-	-
Critical Hdwy	7.6	6.8	-	-	4.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	6.6	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	684	904	-	-	1050	-
Stage 1	941	-	-	_	-	_
Stage 2	957	-	_	-	_	-
Platoon blocked, %	301		_	_		_
Mov Cap-1 Maneuver	684	904	_	_	1050	_
Mov Cap-1 Maneuver	684	30 4 -			1030	_
				-		
Stage 1	941	-	-	-	-	-
Stage 2	957	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A					
I IOIVI LOO	٨					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	1050	-
HCM Lane V/C Ratio		-	-	-	-	_
HCM Control Delay (s)		_	_	0	0	-
HCM Lane LOS		_	_	A	A	_
HCM 95th %tile Q(veh)	_	_	-	0	_
HOW JOHN JOHN Q (VEI)	1				U	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	1	0	145	0	2	126	0
Future Volume (vph)	0	0	0	0	0	1	0	145	0	2	126	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)							3					3
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	1	0	0	169	0	0	149	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type:

Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	1	0	145	0	2	126	0
Future Vol, veh/h	0	0	0	0	0	1	0	145	0	2	126	0
Conflicting Peds, #/hr	0	0	0	0	0	0	3	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	_	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	0	0
Mvmt Flow	0	0	0	0	0	1	0	169	0	2	147	0
Major/Minor	linor2		, and a	liner1			Major1			/oier2		
		202		Minor1	202		Major1	0		Major2	^	^
Conflicting Flow All	324	323	150	320	323	169	150	0	0	169	0	0
Stage 1	154	154	-	169	169	-	-	-	-	-	-	-
Stage 2	170	169	6.2	151	154	- 	4.2	-	-	12	-	-
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	-	-	4.3	-	-
Critical Hdwy Stg 1	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.3	5.7	2 1	5.5	4.9	2 1	3	-	-	-	-	-
Follow-up Hdwy	710	4 507	3.1 952	3	621	3.1 946	1067	-	-	3 1051	-	-
Pot Cap-1 Maneuver	977	587 767		767 994	631 784		1007	-	-		-	-
Stage 1	956	755	-		784	-	-	-	-	-	-	-
Stage 2 Platoon blocked, %	930	100	-	1014	194	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	706	584	949	765	628	946	1064	-	-	1051	-	
Mov Cap-1 Maneuver	706	584	949	765	628	340	1004	-	-	1001	-	_
Stage 1	974	763		994	784	-	-		-	-	-	
Stage 1 Stage 2	955	755	-	1012	790	-	-	-	-	_	-	-
Slaye Z	900	100	_	1012	1 30	_	<u>-</u>	<u>-</u>	-	_	-	<u>-</u>
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			8.8			0			0.1		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt		NBL	NBT	NRR F	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1064	1101	ואטויו		946	1051	051	אופט			
HCM Lane V/C Ratio		1004	-	-	_	0.001		_	_			
HCM Control Delay (s)		0	_	_	0	8.8	8.4	0	_			
HCM Lane LOS		A	-	-	A	0.0 A	0.4 A	A				
HCM 95th %tile Q(veh)		0	-	-	A -	0	0	- A	-			
HOW SOUT MILE Q(VEII)		U	-	-	_	U	U	_	_			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	ĥ	
Traffic Volume (vph)	9	0	0	131	118	10
Future Volume (vph)	9	0	0	131	118	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)	1					
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	11	0	0	164	161	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	0.3					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			4	ĵ.	
Traffic Vol, veh/h	9	0	0	131	118	10
Future Vol, veh/h	9	0	0	131	118	10
Conflicting Peds, #/hr	1	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	0	0	0	1	0	0
Mvmt Flow	11	0	0	164	148	13
	Minor2		//ajor1		/lajor2	
Conflicting Flow All	320	155	161	0	-	0
Stage 1	155	-	-	-	-	-
Stage 2	165	-	-	-	-	-
Critical Hdwy	6.2	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.2	-	-	-	-	-
Critical Hdwy Stg 2	5.2	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	788	954	1058	-	-	-
Stage 1	1023	-	-	-	_	_
Stage 2	1012	-	_	_	_	_
Platoon blocked, %	1012			_	_	_
Mov Cap-1 Maneuver	788	954	1058	_	_	_
Mov Cap-1 Maneuver	788	334	1000		_	
	1023		-	<u>-</u>		-
Stage 1		-	-	-	-	-
Stage 2	1012	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.6		0		0	
HCM LOS	A					
	, ,					
Minor Lane/Major Mvm	ıt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1058	-	788	-	-
HCM Lane V/C Ratio		-	-	0.014	-	-
HCM Control Delay (s)		0	-	9.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

2026 PROJECTED CONDITIONS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	10	223	78	3	243	0	97	2	2	0	1	45
Future Volume (vph)	10	223	78	3	243	0	97	2	2	0	1	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)	1					1						
Confl. Bikes (#/hr)												
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	20%	6%	1%	33%	6%	0%	3%	0%	100%	0%	100%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	346	0	0	273	0	0	112	0	0	1	50
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type: Other
Control Type: Unsignalized

ntersection	
ntersection Delay, s/veh ntersection LOS	11.8
ntersection LOS	В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Traffic Vol, veh/h	10	223	78	3	243	0	97	2	2	0	1	45
Future Vol, veh/h	10	223	78	3	243	0	97	2	2	0	1	45
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	20	6	1	33	6	0	3	0	100	0	100	0
Mvmt Flow	11	248	87	3	270	0	108	2	2	0	1	50
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			2				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	2			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			2			1				1	
HCM Control Delay	12.4			12.1			10.3				8.8	
HCM LOS	В			В			В				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2	
Vol Left, %	96%	3%	1%	0%	0%	
Vol Thru, %	2%	72%	99%	100%	0%	
Vol Right, %	2%	25%	0%	0%	100%	
Sign Control	Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	101	311	246	1	45	
LT Vol	97	10	3	0	0	
Through Vol	2	223	243	1	0	
RT Vol	2	78	0	0	45	
Lane Flow Rate	112	346	273	1	50	
Geometry Grp	5	2	2	7	7	
Degree of Util (X)	0.185	0.471	0.405	0.002	0.078	
Departure Headway (Hd)	5.941	4.909	5.336	8.044	5.603	
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	
Cap	607	726	667	447	643	
Service Time	3.944	2.992	3.427	5.748	3.307	
HCM Lane V/C Ratio	0.185	0.477	0.409	0.002	0.078	
HCM Control Delay	10.3	12.4	12.1	10.8	8.8	
HCM Lane LOS	В	В	В	В	Α	
HCM 95th-tile Q	0.7	2.5	2	0	0.3	

HCM 6th AWSC c:\pwworking\projectwise\tpd_dlenker\d2073138\AM Files.syn

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Lane Group WBL WBR NBT NBR SBL SB
Lane Configurations 🏋 🔓
Traffic Volume (vph) 0 2 95 0 2 8
Future Volume (vph) 0 2 95 0 2 8
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Lane Width (ft) 10 10 11 11 11 11
Grade (%) 6% 3% -3%
Storage Length (ft) 0 0 0
Storage Lanes 1 0 0 0
Taper Length (ft) 25 25
Link Speed (mph) 25 25
Link Distance (ft) 387 260 368
Travel Time (s) 10.6 7.1 10.0
Confl. Peds. (#/hr)
Confl. Bikes (#/hr)
Peak Hour Factor 0.91 0.91 0.91 0.91 0.91 0.91
Growth Factor 100% 100% 100% 100% 100% 100%
Heavy Vehicles (%) 2% 2% 4% 2% 2% 6%
Bus Blockages (#/hr) 0 0 0 0 0
Parking (#/hr)
Mid-Block Traffic (%) 0% 0%
Shared Lane Traffic (%)
Lane Group Flow (vph) 2 0 104 0 0 9
Sign Control Stop Free Free
Intersection Summary
Area Type: Other

Intersection	_					
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WBL	WDN		NDI	SDL	
Lane Configurations		2	}	٥	2	र्स 81
Traffic Vol, veh/h	0	2	95	0	2	
Future Vol, veh/h	0	2	95	0	2	81
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	4	2	2	6
Mvmt Flow	0	2	104	0	2	89
Major/Minor	Minor1		laiar1		Majora	
	Minor1		Major1		Major2	
Conflicting Flow All	197	104	0	0	104	0
Stage 1	104	-	-	-	-	-
Stage 2	93	-	-	-		-
Critical Hdwy	7.62	6.82	-	-	4.3	-
Critical Hdwy Stg 1	6.62	-	-	-	-	-
Critical Hdwy Stg 2	6.62	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	858	997	-	-	1106	-
Stage 1	1035	-	-	-	-	-
Stage 2	1051	-	-	-	-	-
Platoon blocked, %	. 30 1		_	_		_
Mov Cap-1 Maneuver	856	997		_	1106	
Mov Cap-1 Maneuver	856	991	_	_	1100	_
	1035		_	_	<u>-</u>	_
Stage 1		-	-	-	-	-
Stage 2	1049	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0.2	
HCM LOS	Α		- 0		J.2	
TIOWI LOO						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	997	1106	-
HCM Lane V/C Ratio		_	-	0.002		-
HCM Control Delay (s)		_	_	8.6	8.3	0
HCM Lane LOS		_	_	A	A	A
HCM 95th %tile Q(veh)	_	_	0	0	-
1.5W 50th 70th Q(Ven	1			U	U	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	1	0	0	0	0	1	0	92	0	0	77	3
Future Volume (vph)	1	0	0	0	0	1	0	92	0	0	77	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)			1	1								
Confl. Bikes (#/hr)												
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	4%	33%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	1	0	0	99	0	0	86	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Area Type: Other Control Type: Unsignalized

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	0	92	0	0	77	3
Future Vol, veh/h	1	0	0	0	0	1	0	92	0	0	77	3
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	4	33
Mvmt Flow	1	0	0	0	0	1	0	99	0	0	83	3
Major/Minor N	Minor2			Minor1		_ 1	Major1		N	//ajor2		
Conflicting Flow All	185	184	86	185	185	99	86	0	0	99	0	0
Stage 1	85	85	- 00	99	99	99	- 00	-	-	99	-	-
Stage 2	100	99	-	86	86	-	-	-	-	_	- -	-
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3		-	4.3		
Critical Hdwy Stg 1	6.3	5.7	0.5	5.5	4.9	5.9	4.5	_	_	4.5	<u> </u>	_
Critical Hdwy Stg 2	6.3	5.7	-	5.5	4.9	-	-	-	-	-		-
Follow-up Hdwy	3	3.7	3.1	3.3	4.9	3.1	3	_	_	3	_	_
Pot Cap-1 Maneuver	890	707	1036	927	735	1030	1122	-	_	1111	_	-
Stage 1	1071	824	1030	1075	831	1030	1122	_	_	1111	<u> </u>	_
Stage 2	1071	813	-	1075	839		-		-	-	-	
Platoon blocked, %	1000	010	_	1030	009	_		_	_	_	_	_
Mov Cap-1 Maneuver	889	707	1035	926	735	1030	1122	_	_	1111	_	
Mov Cap-1 Maneuver	889	707	-	926	735	-	-	_	_	-	_	_
Stage 1	1071	824	_	1075	831	_	_	_	_	_	_	_
Stage 2	1049	813	_	1073	839	_	_	_	_	_	_	_
Olago Z	1070	010		1000	555							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.1			8.5			0			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NBR F	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1122	-			1030	1111					
HCM Lane V/C Ratio		-	_	_	0.001		-	_	_			
HCM Control Delay (s)		0	_	_	9.1	8.5	0	_	_			
HCM Lane LOS		A	_	_	Α	Α	A	_	_			
HCM 95th %tile Q(veh)		0	_	_	0	0	0	_	_			
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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	ĵ.	
Traffic Volume (vph)	19	1	1	67	68	9
Future Volume (vph)	19	1	1	67	68	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	2%	5%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	25	0	0	85	96	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\AM Files.syn

La Caraca Cara						
Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			सी	f)	
Traffic Vol, veh/h	19	1	1	67	68	9
Future Vol, veh/h	19	1	1	67	68	9
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		_	_	0	0	_
Grade, %	-1	_	_	-1	1	_
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	0	0	2	5	11
Mvmt Flow	24	1	1	84	85	11
IVIVIIIL I IOW	24		ļ	04	00	- 11
Major/Minor N	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	178	92	97	0	-	0
Stage 1	92	-	-	-	-	-
Stage 2	86	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.25	-	-	_	_	-
Critical Hdwy Stg 2	5.25	_	_	_	_	_
Follow-up Hdwy	3	3.1	3	_	_	_
Pot Cap-1 Maneuver	946	1034	1112	_	_	_
Stage 1	1090	-	-	_	_	_
Stage 2	1097			_	_	
Platoon blocked, %	1031	_	_	_	_	_
	943	1022	1111	_		-
Mov Cap-1 Maneuver		1033	1111	-	-	-
Mov Cap-2 Maneuver	943	-	-	-	-	-
Stage 1	1088	-	-	-	-	-
Stage 2	1096	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.1		0	
HCM LOS	Α		0.1		U	
I IOW LOS						
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1111	-	947	-	-
HCM Lane V/C Ratio		0.001	_	0.026	-	-
HCM Control Delay (s)		8.2	0	8.9	-	-
HCM Lane LOS		A	A	A	_	_
HCM 95th %tile Q(veh))	0	-	0.1	_	_
1.5W 55th 70th Q(Ven)		U		V. I		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Volume (vph)	28	291	127	8	421	0	128	2	9	3	2	40
Future Volume (vph)	28	291	127	8	421	0	128	2	9	3	2	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	475	0	0	457	0	0	148	0	0	5	43
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											

Intersection												
Intersection Delay, s/veh	16.7											
Intersection LOS	С											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	28	291	127	8	421	0	128	2	9	3	2	40
Future Vol, veh/h	28	291	127	8	421	0	128	2	9	3	2	40
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	1	0	0	1	0	1	0	0	0	0	0
Mvmt Flow	30	310	135	9	448	0	136	2	10	3	2	43
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			1			1		
HCM Control Delay	17.6			17.9			12.1			9.9		
HCM LOS	С			С			В			Α		
Lane		NBLn1	EBLn1	WBLn1	SBLn1	SBLn2						
Vol Left, %		92%	6%	2%	60%	0%						
Vol Thru, %		1%	65%	98%	40%	0%						
Vol Right, %		6%	28%	0%	0%	100%						
Sign Control		Stop	Stop	Stop	Stop	Stop						

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f.			4
Traffic Volume (vph)	0	1	142	0	3	132
Future Volume (vph)	0	1	142	0	3	132
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	1%	2%	2%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	171	0	0	163
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Area Type:

Other

Intersection						
Int Delay, s/veh	0.1					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽	•	•	4
Traffic Vol, veh/h	0	1	142	0	3	132
Future Vol, veh/h	0	1	142	0	3	132
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	2	2	1	2	2	1
Mvmt Flow	0	1	171	0	4	159
NA ' (NA)						
	Minor1		/lajor1		Major2	
Conflicting Flow All	338	171	0	0	171	0
Stage 1	171	-	-	-	-	-
Stage 2	167	-	-	-	-	-
Critical Hdwy	7.62	6.82	-	-	4.3	-
Critical Hdwy Stg 1	6.62	-	-	-	-	-
Critical Hdwy Stg 2	6.62	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	673	903	-	-	1050	-
Stage 1	940	-	_	_	-	_
Stage 2	946	_	_	-	-	-
Platoon blocked, %	0 10		_	_		_
Mov Cap-1 Maneuver	670	903	_	_	1050	_
Mov Cap-1 Maneuver	670	-			1000	_
Stage 1	940	-	_	<u>-</u>	-	<u>-</u>
	940		-	-	•	-
Stage 2	942	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		0.2	
HCM LOS	A		- 0		J.Z	
TIOWI LOO						
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	903	1050	-
HCM Lane V/C Ratio		_	_	0.001		-
HCM Control Delay (s)		_	-	9	8.4	0
HCM Lane LOS		_	_	A	A	A
HCM 95th %tile Q(veh)		_	_	0	0	-
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	1	0	145	0	2	126	0
Future Volume (vph)	0	0	0	0	0	1	0	145	0	2	126	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)							3					3
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	1	0	0	169	0	0	149	0
Sign Control Intersection Summary		Stop			Stop			Free			Free	

Intersection Summary

Area Type:

Other

Intersection												
Int Delay, s/veh	0.1											
•		CDT.	EDD.	WDI	MPT	WEE	ND	NET	NDD	ODL	ODT	ODD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	•	4	^	•	4	4	•	4	•	•	4	^
Traffic Vol, veh/h	0	0	0	0	0	1	0	145	0	2	126	0
Future Vol, veh/h	0	0	0	0	0	1	0	145	0	2	126	0
Conflicting Peds, #/hr	0	0	0	0	0	0	3	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	0	0
Mvmt Flow	0	0	0	0	0	1	0	169	0	2	147	0
Major/Minor M	linor2		N	/linor1		ı	Major1		N	/lajor2		
Conflicting Flow All	324	323	150	320	323	169	150	0	0	169	0	0
Stage 1	154	154	-	169	169	-	-	-	-	-	-	-
Stage 2	170	169	_	151	154	_	_	_	_	_	_	_
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	_	_	4.3	_	_
Critical Hdwy Stg 1	6.3	5.7	-	5.5	4.9	- 0.0	- 1.5	_	_	u	_	_
Critical Hdwy Stg 2	6.3	5.7	_	5.5	4.9	_	_		_	_	_	
Follow-up Hdwy	3	4	3.1	3.3	4.3	3.1	3	<u> </u>		3	_	_
Pot Cap-1 Maneuver	710	587	952	767	631	946	1067			1051		
Stage 1	977	767	952	994	784	340	1007	_	_	1001	_	_
Stage 1	956	755		1014	794	-	-	-	-	-	-	-
Platoon blocked, %	300	100	-	1014	134	-	•	-	•	-	-	-
	706	584	949	765	628	946	1064	-	-	1051	-	-
Mov Cap-1 Maneuver				765		940	1004	-	-	1051	-	-
Mov Cap-2 Maneuver	706	584	-	765	628	-	-	-	-	-	-	-
Stage 1	974	763	-	994	784	-	-	-	-	-	-	-
Stage 2	955	755	-	1012	790	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			8.8			0			0.1		
HCM LOS	A			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1064	-	-	-	946	1051	-	-			
HCM Lane V/C Ratio		_	-	_	_		0.002	_	_			
HCM Control Delay (s)		0	_	_	0	8.8	8.4	0	_			
HCM Lane LOS		A	-	_	A	A	Α	A	_			
HCM 95th %tile Q(veh)		0	_	_	-	0	0	-	_			
HOW JOHN JOHN Q(VEII)		U		_		U	U					

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	ĵ.	
Traffic Volume (vph)	9	0	0	131	118	10
Future Volume (vph)	9	0	0	131	118	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)	1					
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	11	0	0	164	161	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		LDK	INDL			אמט
Lane Configurations	¥	0		4124	110	40
Traffic Vol, veh/h	9	0	0	131	118	10
Future Vol, veh/h	9	0	0	131	118	10
Conflicting Peds, #/hr	1	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	0	0	0	1	0	0
Mymt Flow	11	0	0	164	148	13
IVIVIII(I IOVV		U	U	104	170	10
Major/Minor I	Minor2	<u> </u>	//ajor1	N	/lajor2	
Conflicting Flow All	320	155	161	0	-	0
Stage 1	155	-	_	-	_	-
Stage 2	165	_	_	_	_	_
Critical Hdwy	6.2	6.1	4.3	_	_	_
Critical Hdwy Stg 1	5.2	- 0.1		_	<u> </u>	_
, ,	5.2	_		_	_	-
Critical Hdwy Stg 2			-			
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	788	954	1058	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1012	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	788	954	1058	-	-	-
Mov Cap-2 Maneuver	788	-	-	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1012	_	_	_	_	-
2.5.30 -						
Approach	EB		NB		SB	
HCM Control Delay, s	9.6		0		0	
HCM LOS	Α					
Minor Lone /Maior Mario	.1	NDI	NDT	CDL 4	CDT	CDD
Minor Lane/Major Mvm	IL .	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1058	-		-	-
HCM Lane V/C Ratio		-	-	0.014	-	-
HCM Control Delay (s)		0	-	9.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-
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2036 BASE CONDITIONS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Traffic Volume (vph)	11	236	83	3	258	0	101	2	1	0	1	47
Future Volume (vph)	11	236	83	3	258	0	101	2	1	0	1	47
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)	1					1						
Confl. Bikes (#/hr)												
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	20%	6%	1%	33%	6%	0%	3%	0%	100%	0%	100%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	366	0	0	290	0	0	115	0	0	1	52
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type:

Control Type: Unsignalized

Other

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	11	236	83	3	258	0	101	2	1	0	1	47
Future Vol, veh/h	11	236	83	3	258	0	101	2	1	0	1	47
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	20	6	1	33	6	0	3	0	100	0	100	0
Mvmt Flow	12	262	92	3	287	0	112	2	1	0	1	52
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			2				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	2			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			2			1				1	
HCM Control Delay	13.1			12.8			10.6				9	
HCM LOS	R			R			R				Δ	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	97%	3%	1%	0%	0%
Vol Thru, %	2%	72%	99%	100%	0%
Vol Right, %	1%	25%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	104	330	261	1	47
LT Vol	101	11	3	0	0
Through Vol	2	236	258	1	0
RT Vol	1	83	0	0	47
Lane Flow Rate	116	367	290	1	52
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.195	0.505	0.443	0.003	0.083
Departure Headway (Hd)	6.066	5.063	5.494	8.171	5.727
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	593	715	658	439	627
Service Time	4.085	3.063	3.494	5.893	3.448
HCM Lane V/C Ratio	0.196	0.513	0.441	0.002	0.083
HCM Control Delay	10.6	13.1	12.8	10.9	9
HCM Lane LOS	В	В	В	В	Α
HCM 95th-tile Q	0.7	2.9	2.3	0	0.3

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M.	•	f)	•		ર્ન
Traffic Volume (vph)	0	0	100	0	2	86
Future Volume (vph)	0	0	100	0	2	86
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	4%	0%	0%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	110	0	0	97
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	WDIX	1>	HOIL	ODL	<u>લ</u>
Traffic Vol, veh/h	0	0	100	0	2	86
Future Vol, veh/h	0	0	100	0	2	86
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	riee -	None		None
					-	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	0	4	0	0	6
Mvmt Flow	0	0	110	0	2	95
Major/Minor I	Minor1	N	/lajor1	N	/lajor2	
	209	110	0	0	110	0
Conflicting Flow All						
Stage 1	110	-	-	-	-	-
Stage 2	99	-	-	-	-	-
Critical Hdwy	7.6	6.8	-	-	4.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	6.6	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	841	989	-	-	1101	-
Stage 1	1026	-	-	-	-	-
Stage 2	1043	_	_	_	_	_
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	839	989	_	_	1101	_
Mov Cap-1 Maneuver	839	303			-	_
•			-	-		
Stage 1	1026	-	-	-	-	-
Stage 2	1041	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0.2	
HCM LOS	A		•		0.2	
TOW LOO						
NA: I (NA : NA		NET	NDD	MDL 4	051	OPT
Minor Lane/Major Mvm	τ	NBT	NRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1101	-
HCM Lane V/C Ratio		-	-		0.002	-
HCM Control Delay (s)		-	-	0	8.3	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	-	0	-

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	1	0	0	0	0	1	0	97	0	0	82	3
Future Volume (vph)	1	0	0	0	0	1	0	97	0	0	82	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)			1	1								
Confl. Bikes (#/hr)												
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	4%	33%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	1	0	0	104	0	0	91	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type: Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	0	97	0	0	82	3
Future Vol, veh/h	1	0	0	0	0	1	0	97	0	0	82	3
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None		_	None	_	_	None
Storage Length	_	_	_	_	_	-	_	_	-	_	-	-
Veh in Median Storage	.# -	0	-	-	0	-	_	0	_	-	0	_
Grade, %	, -	1	_	_	-3	-	-	6	-	_	-4	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	4	33
Mvmt Flow	1	0	0	0	0	1	0	104	0	0	88	3
NA - ' /NA'	4:			1' 4			14-1-4			4.1.		
	Minor2	454		Minor1	10-		Major1			Major2		
Conflicting Flow All	195	194	91	195	195	104	91	0	0	104	0	0
Stage 1	90	90	-	104	104	-	-	-	-	-	-	-
Stage 2	105	104	-	91	91	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	-	-	4.3	-	-
Critical Hdwy Stg 1	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Follow-up Hdwy	3	4	3.1	3	4	3.1	3	-	-	3	-	-
Pot Cap-1 Maneuver	876	697	1030	914	727	1024	1118	-	-	1106	-	-
Stage 1	1064	820	-	1069	827	-	-	-	-	-	-	-
Stage 2	1043	808	-	1084	836	-	-	-	-	-	-	-
Platoon blocked, %			1077			100	44.5	-	-	44	-	-
Mov Cap-1 Maneuver	875	697	1029	913	727	1024	1118	-	-	1106	-	-
Mov Cap-2 Maneuver	875	697	-	913	727	-	-	-	-	-	-	-
Stage 1	1064	820	-	1069	827	-	-	-	-	-	-	-
Stage 2	1042	808	-	1083	836	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.1			8.5			0			0		
HCM LOS	Α			Α			- 0			- 0		
1.5m 200	, ,			, ,								
					·	VD1 (0.51	05-	055			
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1118	-	-		1024	1106	-	-			
HCM Lane V/C Ratio		-	-	-	0.001		-	-	-			
HCM Control Delay (s)		0	-	-	9.1	8.5	0	-	-			
HCM Lane LOS		Α	-	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	^}	
Traffic Volume (vph)	21	1	1	71	72	10
Future Volume (vph)	21	1	1	71	72	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	2%	5%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	0	0	90	103	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	1.2					
-		EDD.	NDI	NDT	ODT	ODD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	f)	
Traffic Vol, veh/h	21	1	1	71	72	10
Future Vol, veh/h	21	1	1	71	72	10
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	0	0	2	5	11
Mvmt Flow	26	1	1	89	90	13
Major/Minor	Aire a =O		1-1-1		Anie C	
	linor2		Major1		Major2	
Conflicting Flow All	189	98	104	0	-	0
Stage 1	98	-	-	-	-	-
Stage 2	91	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.25	-	-	-	-	-
Critical Hdwy Stg 2	5.25	-	-	-	-	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	932	1026	1106	-	-	-
Stage 1	1083	-	-	-	-	-
Stage 2	1091	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	929	1025	1105	-	-	-
Mov Cap-2 Maneuver	929		-	-	_	-
Stage 1	1081	_	_	_	_	_
Stage 2	1090	_	_	_	_	_
J						
Approach	EB		NB		SB	
HCM Control Delay, s	9		0.1		0	
HCM LOS	Α					
Mineral and Marin No.		NDI	NDT	CDL 4	ODT	ODD
Minor Lane/Major Mvmt		NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1105	-	933	-	-
HCM Lane V/C Ratio		0.001		0.029	-	-
HCM Control Delay (s)		8.3	0	9	-	-
		٨	٨	Α		_
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	Α	0.1	-	_

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Traffic Volume (vph)	30	308	133	8	447	0	135	2	10	3	2	42
Future Volume (vph)	30	308	133	8	447	0	135	2	10	3	2	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	501	0	0	485	0	0	157	0	0	5	45
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

Area Type: Other

EBL

EBT

EBR

WBL

Movement

SBL

SBT

SBR

Intersection	
Intersection Delay, s/veh	19
Intersection LOS	С

WBT

WBR

NBL

NBT

NBR

Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	30	308	133	8	447	0	135	2	10	3	2	42
Future Vol, veh/h	30	308	133	8	447	0	135	2	10	3	2	42
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	1	0	0	1	0	1	0	0	0	0	0
Mvmt Flow	32	328	141	9	476	0	144	2	11	3	2	45
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			1			1		
HCM Control Delay	20.3			20.7			12.6			10.1		
HCM LOS	С			С			В			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	92%	6%	2%	60%	0%
Vol Thru, %	1%	65%	98%	40%	0%
Vol Right, %	7%	28%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	147	471	455	5	42
LT Vol	135	30	8	3	0
Through Vol	2	308	447	2	0
RT Vol	10	133	0	0	42
Lane Flow Rate	156	501	484	5	45
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.293	0.717	0.715	0.011	0.083
Departure Headway (Hd)	6.742	5.155	5.32	7.675	6.648
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	530	696	678	464	535
Service Time	4.816	3.211	3.375	5.466	4.438
HCM Lane V/C Ratio	0.294	0.72	0.714	0.011	0.084
HCM Control Delay	12.6	20.3	20.7	10.6	10
HCM Lane LOS	В	С	С	В	Α
HCM 95th-tile Q	1.2	6.1	6	0	0.3

HCM 6th AWSC c:\pwworking\projectwise\tpd_dlenker\d2073138\PM Files.syn

Lane Group WBL WBR NBT NBR SBL SBT
Lane Configurations Y 5
Traffic Volume (vph) 0 0 151 0 0 140
Future Volume (vph) 0 0 151 0 0 140
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900
Lane Width (ft) 10 10 11 11 11 11
Grade (%) 6% 3% -3%
Storage Length (ft) 0 0 0
Storage Lanes 1 0 0 0
Taper Length (ft) 25 25
Link Speed (mph) 25 25 25
Link Distance (ft) 387 260 368
Travel Time (s) 10.6 7.1 10.0
Confl. Peds. (#/hr)
Confl. Bikes (#/hr)
Peak Hour Factor 0.83 0.83 0.83 0.83 0.83
Growth Factor 100% 100% 100% 100% 100% 100%
Heavy Vehicles (%) 0% 0% 1% 0% 0% 1%
Bus Blockages (#/hr) 0 0 0 0 0
Parking (#/hr)
Mid-Block Traffic (%) 0% 0%
Shared Lane Traffic (%)
Lane Group Flow (vph) 0 0 182 0 0 169
Sign Control Stop Free Free
Intersection Summary
Area Type: Other

-						
Intersection						
Int Delay, s/veh	0					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	^	}	^	_	4
Traffic Vol, veh/h	0	0	151	0	0	140
Future Vol, veh/h	0	0	151	0	0	140
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	0	0	182	0	0	169
			.02		- 5	.00
	Minor1		//ajor1		Major2	
Conflicting Flow All	351	182	0	0	182	0
Stage 1	182	-	-	-	-	-
Stage 2	169	-	-	-	-	-
Critical Hdwy	7.6	6.8	-	-	4.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	_
Critical Hdwy Stg 2	6.6	_	_	-	_	-
Follow-up Hdwy	3	3.1	_	_	3	_
Pot Cap-1 Maneuver	660	890	_	_	1041	_
Stage 1	926	-	_		-	_
Stage 2	944	_		-	-	
	344	_			-	
Platoon blocked, %	660	000	-	-	1011	-
Mov Cap-1 Maneuver	660	890	-	-	1041	-
Mov Cap-2 Maneuver	660	-	-	-	-	-
Stage 1	926	-	-	-	-	-
Stage 2	944	-	-	-	-	-
Approach	WB		NB		SB	
	0		0		0	
HCM Control Delay, s			U		U	
HCM LOS	Α					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-		-	1041	-
HCM Lane V/C Ratio			_		1041	
		-	-	-	-	-
HCM Control Delay (s)		-	-	0	0	-
HCM Lane LOS		-	-	Α	A 0	-
HCM 95th %tile Q(veh)				_		_

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	1	0	153	0	2	134	0
Future Volume (vph)	0	0	0	0	0	1	0	153	0	2	134	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)							3					3
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	1	0	0	178	0	0	158	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Intersection Summary

Area Type: Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	1	0	153	0	2	134	0
Future Vol, veh/h	0	0	0	0	0	1	0	153	0	2	134	0
Conflicting Peds, #/hr	0	0	0	0	0	0	3	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	0	0
Mvmt Flow	0	0	0	0	0	1	0	178	0	2	156	0
Major/Minor N	/linor2		ı	Minor1			Major1		N	/lajor2		
Conflicting Flow All	342	341	159	338	341	178	159	0	0	178	0	0
Stage 1	163	163	159	178	178	1/0	159	-	-	1/0	-	-
Stage 2	179	178	-	160	163	-	-	-	-	-	<u> </u>	-
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3		-	4.3		
Critical Hdwy Stg 1	6.3	5.7	0.5	5.5	4.9	5.5	4.5	_	_	4.5	<u> </u>	_
Critical Hdwy Stg 2	6.3	5.7	_	5.5	4.9	_		-	-			<u>-</u>
Follow-up Hdwy	3	3.7	3.1	3.3	4.9	3.1	3	_	_	3	_	_
Pot Cap-1 Maneuver	689	573	941	748	618	936	1060	_	_	1044	_	_
Stage 1	965	760	34 1	984	779	930	1000	_	_	1044	<u> </u>	_
Stage 2	944	748	-	1004	788	_	_	_		_	_	
Platoon blocked, %	דדט	7 70		1007	100			_	_		_	_
Mov Cap-1 Maneuver	685	570	938	747	615	936	1057	_	_	1044	_	_
Mov Cap-2 Maneuver	685	570	-	747	615	-	-	_	<u>-</u>	-	_	<u>-</u>
Stage 1	962	756	-	984	779	_	_	_	_	_	_	_
Stage 2	943	748	<u>-</u>	1002	784	_	<u>-</u>	_	<u>-</u>	<u>-</u>	_	<u>-</u>
J	0.10	. 10		.002								
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			8.9			0			0.1		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1057	_	_	_	936	1044	_	_			
HCM Lane V/C Ratio		-	_	_	_	0.001		_	_			
HCM Control Delay (s)		0	_	_	0	8.9	8.5	0	_			
HCM Lane LOS		A	_	_	A	Α	A	A	_			
HCM 95th %tile Q(veh)		0	-	_	-	0	0	-	-			

	•	•	1	†	Ţ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	f)	
Traffic Volume (vph)	10	0	0	139	125	11
Future Volume (vph)	10	0	0	139	125	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)	1					
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	174	170	0
Sign Control	Stop			Free	Free	
Intersection Summary						

Area Type: Other
Control Type: Unsignalized

Intersection						
Int Delay, s/veh	0.3					
	EDI		NDI	NDT	ODT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	₽	
Traffic Vol, veh/h	10	0	0	139	125	11
Future Vol, veh/h	10	0	0	139	125	11
Conflicting Peds, #/hr	1	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	0	0	0	1	0	0
Mvmt Flow	13	0	0	174	156	14
	Minor2		Major1		Major2	
Conflicting Flow All	338	163	170	0	-	0
Stage 1	163	-	-	-	-	-
Stage 2	175	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.2	-	-	-	-	-
Critical Hdwy Stg 2	5.2	-	-	-	_	-
Follow-up Hdwy	3	3.1	3	-	-	-
Pot Cap-1 Maneuver	762	944	1050	-	_	-
Stage 1	1014	_	-	_	_	_
Stage 2	1002	_	_	_	_	_
Platoon blocked, %	1002			_	_	_
Mov Cap-1 Maneuver	762	944	1050	_	_	
Mov Cap-1 Maneuver	762	344 -	1000	_	_	_
	1014		-	<u>-</u>		-
Stage 1		-	-	-	-	-
Stage 2	1002	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.8		0		0	
HCM LOS	A					
	, ·					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1050	-	762	-	-
HCM Lane V/C Ratio		-	-	0.016	-	-
HCM Control Delay (s)		0	-	9.8	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0.1	-	-

2036 PROJECTED CONDITIONS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Traffic Volume (vph)	11	236	83	3	258	0	102	2	2	0	1	47
Future Volume (vph)	11	236	83	3	258	0	102	2	2	0	1	47
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)	1					1						
Confl. Bikes (#/hr)												
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	20%	6%	1%	33%	6%	0%	3%	0%	100%	0%	100%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	366	0	0	290	0	0	117	0	0	1	52
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												

intersection Summa

Area Type:

Other

ntersection	
ntersection Delay, s/veh ntersection LOS	12.4
ntersection LOS	В

Movement	EBL	FBI	EBK	WBL	WBI	WBR	NBL	NRT	NBK	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	11	236	83	3	258	0	102	2	2	0	1	47
Future Vol, veh/h	11	236	83	3	258	0	102	2	2	0	1	47
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	20	6	1	33	6	0	3	0	100	0	100	0
Mvmt Flow	12	262	92	3	287	0	113	2	2	0	1	52
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			2				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	2			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			2			1				1	
HCM Control Delay	13.1			12.8			10.6				9	
HCM LOS	В			В			В				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2
Vol Left, %	96%	3%	1%	0%	0%
Vol Thru, %	2%	72%	99%	100%	0%
Vol Right, %	2%	25%	0%	0%	100%
Sign Control	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	106	330	261	1	47
LT Vol	102	11	3	0	0
Through Vol	2	236	258	1	0
RT Vol	2	83	0	0	47
Lane Flow Rate	118	367	290	1	52
Geometry Grp	5	2	2	7	7
Degree of Util (X)	0.198	0.505	0.443	0.003	0.083
Departure Headway (Hd)	6.06	5.069	5.501	8.177	5.733
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes
Cap	594	715	658	439	627
Service Time	4.078	3.069	3.501	5.897	3.452
HCM Lane V/C Ratio	0.199	0.513	0.441	0.002	0.083
HCM Control Delay	10.6	13.1	12.8	10.9	9
HCM Lane LOS	В	В	В	В	Α
HCM 95th-tile Q	0.7	2.9	2.3	0	0.3

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)			ર્ન
Traffic Volume (vph)	0	2	100	0	2	86
Future Volume (vph)	0	2	100	0	2	86
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	4%	2%	2%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	2	0	110	0	0	97
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		WBR		NBK	SBL	
Lane Configurations	¥	0	♣	•	•	4
Traffic Vol, veh/h	0	2	100	0	2	86
Future Vol, veh/h	0	2	100	0	2	86
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	4	2	2	6
Mvmt Flow	0	2	110	0	2	95
IVIVIIICT IOW	U		110	U		30
Major/Minor N	Minor1	N	//ajor1	1	Major2	
Conflicting Flow All	209	110	0	0	110	0
Stage 1	110	-	-	-	-	-
Stage 2	99	_	_	_	_	_
Critical Hdwy	7.62	6.82	_	_	4.3	
Critical Hdwy Stg 1	6.62	0.02	_	_	4.5	_
			-		-	-
Critical Hdwy Stg 2	6.62	-	-	-	-	-
Follow-up Hdwy	3	3.1	-	-	3	-
Pot Cap-1 Maneuver	840	988	-	-	1101	-
Stage 1	1026	-	-	-	-	-
Stage 2	1042	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	838	988	-	-	1101	-
Mov Cap-2 Maneuver	838	-	-	-	-	_
Stage 1	1026	-	_	_	_	_
Stage 2	1040	_	_	_	_	_
Olaye Z	10-10		-			
Approach	WB		NB		SB	
HCM Control Delay, s	8.7		0		0.2	
HCM LOS	A				7.2	
TIOWI LOG	٨					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_		988	1101	_
HCM Lane V/C Ratio		_		0.002		_
HCM Control Delay (s)			-	8.7	8.3	0
		_	_			
HCM Lane LOS		-	-	A	A	Α
HCM 95th %tile Q(veh)		-	-	0	0	-

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	1	0	0	0	0	1	0	97	0	0	82	3
Future Volume (vph)	1	0	0	0	0	1	0	97	0	0	82	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)			1	1								
Confl. Bikes (#/hr)												
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	4%	33%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1	0	0	1	0	0	104	0	0	91	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

Area Type:

Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	0	97	0	0	82	3
Future Vol, veh/h	1	0	0	0	0	1	0	97	0	0	82	3
Conflicting Peds, #/hr	0	0	1	1	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	4	33
Mvmt Flow	1	0	0	0	0	1	0	104	0	0	88	3
Major/Minor N	Minor2		ľ	Minor1		1	Major1		N	//ajor2		
Conflicting Flow All	195	194	91	195	195	104	91	0	0	104	0	0
Stage 1	90	90	-	104	104		-	-	-	-	_	_
Stage 2	105	104	_	91	91	_	_	_	_	_	_	_
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	_	-	4.3	_	_
Critical Hdwy Stg 1	6.3	5.7	_	5.5	4.9	-	_	_	_	-	-	_
Critical Hdwy Stg 2	6.3	5.7	_	5.5	4.9	-	-	_	-	_	_	_
Follow-up Hdwy	3	4	3.1	3	4	3.1	3	_	-	3	-	-
Pot Cap-1 Maneuver	876	697	1030	914	727	1024	1118	-	-	1106	-	-
Stage 1	1064	820	-	1069	827	-	-	-	-	-	-	-
Stage 2	1043	808	-	1084	836	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	875	697	1029	913	727	1024	1118	-	-	1106	-	-
Mov Cap-2 Maneuver	875	697	-	913	727	-	-	-	-	-	-	-
Stage 1	1064	820	-	1069	827	-	-	-	-	-	-	-
Stage 2	1042	808	-	1083	836	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.1			8.5			0			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1118	-	-		1024	1106	-	-			
HCM Lane V/C Ratio		-	_	-	0.001		-	-	_			
HCM Control Delay (s)		0	-	_	9.1	8.5	0	_	_			
HCM Lane LOS		A	-	-	Α	Α	A	-	_			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			
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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ર્ન	f)	
Traffic Volume (vph)	21	1	1	71	72	10
Future Volume (vph)	21	1	1	71	72	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	2%	5%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	0	0	90	103	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Intersection						
Int Delay, s/veh	1.2					
		EDD.	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			4	ĵ.	
Traffic Vol, veh/h	21	1	1	71	72	10
Future Vol, veh/h	21	1	1	71	72	10
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	5	0	0	2	5	11
Mvmt Flow	26	1	1	89	90	13
WWW.CT IOW	20	•	•	00	00	10
	Minor2		Major1		/lajor2	
Conflicting Flow All	189	98	104	0	-	0
Stage 1	98	-	-	-	-	-
Stage 2	91	-	-	-	-	-
Critical Hdwy	6.3	6.1	4.3	-	-	-
Critical Hdwy Stg 1	5.25	-	-	-	-	-
Critical Hdwy Stg 2	5.25	_	-	-	_	_
Follow-up Hdwy	3	3.1	3	_	_	_
Pot Cap-1 Maneuver	932	1026	1106	_	_	-
Stage 1	1083	-		_	_	_
Stage 2	1091	_	_	_	_	_
Platoon blocked, %	1001					_
Mov Cap-1 Maneuver	929	1025	1105	<u>-</u>	_	-
			1103	-	-	_
Mov Cap-2 Maneuver	929	-	-	-	-	-
Stage 1	1081	-	-	-	-	-
Stage 2	1090	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9		0.1		0	
HCM LOS	A		0.1		U	
I IOWI LOG	٨					
Minor Lane/Major Mvm	ıt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1105	_	933	-	_
HCM Lane V/C Ratio		0.001	_		_	_
HCM Control Delay (s)		8.3	0	9	_	_
HCM Lane LOS		A	A	A	_	_
HCM 95th %tile Q(veh)	1	0		0.1	_	_
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Traffic Volume (vph)	30	308	135	9	447	0	136	2	10	3	2	42
Future Volume (vph)	30	308	135	9	447	0	136	2	10	3	2	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	10	10	10	11	11	11	11	11	11
Grade (%)		-1%			2%			3%			-2%	
Storage Length (ft)	0		0	0		0	0		0	0		175
Storage Lanes	0		0	0		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		35			35			25			25	
Link Distance (ft)		461			540			368			479	
Travel Time (s)		9.0			10.5			10.0			13.1	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	504	0	0	486	0	0	158	0	0	5	45
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
A T	Other											

Area Type:

Other

Intersection												
Intersection Delay, s/veh	19.2											
Intersection LOS	С											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Traffic Vol, veh/h	30	308	135	9	447	0	136	2	10	3	2	42
Future Vol, veh/h	30	308	135	9	447	0	136	2	10	3	2	42
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles, %	0	1	0	0	1	0	1	0	0	0	0	0
Mvmt Flow	32	328	144	10	476	0	145	2	11	3	2	45
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB	•	•	NB		
Opposing Lanes	1			1			2			1		

дрргоасті	LD	VVD	IND	OD
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	1	1	2	1
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	2	1	1	1
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	1	2	1	1
HCM Control Delay	20.5	20.9	12.7	10.2
HCM LOS	С	С	В	В

Lane	NBLn1	EBLn1	WBLn1	SBLn1	SBLn2	
Vol Left, %	92%	6%	2%	60%	0%	
Vol Thru, %	1%	65%	98%	40%	0%	
Vol Right, %	7%	29%	0%	0%	100%	
Sign Control	Stop	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	148	473	456	5	42	
LT Vol	136	30	9	3	0	
Through Vol	2	308	447	2	0	
RT Vol	10	135	0	0	42	
Lane Flow Rate	157	503	485	5	45	
Geometry Grp	5	2	2	7	7	
Degree of Util (X)	0.295	0.722	0.718	0.011	0.083	
Departure Headway (Hd)	6.75	5.162	5.33	7.689	6.661	
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	
Сар	529	699	674	463	533	
Service Time	4.829	3.216	3.385	5.483	4.455	
HCM Lane V/C Ratio	0.297	0.72	0.72	0.011	0.084	
HCM Control Delay	12.7	20.5	20.9	10.6	10.1	
HCM Lane LOS	В	С	С	В	В	
HCM 95th-tile Q	1.2	6.2	6.1	0	0.3	

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ.			4
Traffic Volume (vph)	0	1	151	0	3	140
Future Volume (vph)	0	1	151	0	3	140
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	10	11	11	11	11
Grade (%)	6%		3%			-3%
Storage Length (ft)	0	0		0	0	
Storage Lanes	1	0		0	0	
Taper Length (ft)	25				25	
Link Speed (mph)	25		25			25
Link Distance (ft)	387		260			368
Travel Time (s)	10.6		7.1			10.0
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	1%	2%	2%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1	0	182	0	0	173
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
O (IT II II						

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	TTDIX	1>	NDIX	OBL	4
Traffic Vol, veh/h	0	1	151	0	3	140
Future Vol, veh/h	0	1	151	0	3	140
	0	0	0	0	0	0
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	6	-	3	-	-	-3
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	2	2	1	2	2	1
Mvmt Flow	0	1	182	0	4	169
N.A. '. (N.A.)					4 : 0	
	Minor1		Major1		Major2	
Conflicting Flow All	359	182	0	0	182	0
Stage 1	182	-	-	-	-	-
Stage 2	177	-	-	-	-	-
Critical Hdwy	7.62	6.82	-	-	4.3	-
Critical Hdwy Stg 1	6.62	-	-	-	-	-
Critical Hdwy Stg 2	6.62	-	_	_	-	-
Follow-up Hdwy	3	3.1	_	_	3	_
Pot Cap-1 Maneuver	649	889	_	_	1041	_
Stage 1	925	-	_	_	-	_
Stage 2	932	_	_		_	_
Platoon blocked, %	332	_		_	_	
	646	000	-	-	1011	-
Mov Cap-1 Maneuver	646	889	-	-	1041	-
Mov Cap-2 Maneuver	646	-	-	-	-	-
Stage 1	925	-	-	-	-	-
Stage 2	928	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.1		0		0.2	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NBR\	VBLn1	SBL	SBT
Capacity (veh/h)			-		1041	-
HCM Lane V/C Ratio		-			0.003	
		-	-			-
HCM Control Delay (s)		-	-	9.1	8.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)			_	0	0	_

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	0	0	0	0	1	0	153	0	2	134	0
Future Volume (vph)	0	0	0	0	0	1	0	153	0	2	134	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	13	13	13	11	11	11	12	12	12
Grade (%)		1%			-3%			6%			-4%	
Storage Length (ft)	0		0	0		0	0		0	0		0
Storage Lanes	0		0	0		0	0		0	0		0
Taper Length (ft)	25			25			25			25		
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		394			355			163			260	
Travel Time (s)		10.7			9.7			4.4			7.1	
Confl. Peds. (#/hr)							3					3
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	1	0	0	178	0	0	158	0
Sign Control Intersection Summary		Stop			Stop			Free			Free	

Intersection Summary

Area Type: Other

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LUL	4	LDIX	WDL	₩	WDIX	NDL	4	אטא	ODL	- SB1	אופט
Traffic Vol, veh/h	0	0	0	0	0	1	0	153	0	2	134	0
Future Vol, veh/h	0	0	0	0	0	1	0	153	0	2	134	0
Conflicting Peds, #/hr	0	0	0	0	0	0	3	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	- -	None	-	- -	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	1	-	-	-3	-	-	6	-	-	-4	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	0	0
Mvmt Flow	0	0	0	0	0	1	0	178	0	2	156	0
Major/Minor N	linor2		N	Minor1			Major1		N	/lajor2		
Conflicting Flow All	342	341	159	338	341	178	159	0	0	178	0	0
Stage 1	163	163	-	178	178	-	-	-	-	-	-	-
Stage 2	179	178	_	160	163	_	<u>-</u>	_	<u>-</u>	_	_	<u>-</u>
Critical Hdwy	7.3	6.7	6.3	6.5	5.9	5.9	4.3	_	_	4.3	_	_
Critical Hdwy Stg 1	6.3	5.7	-	5.5	4.9	-		_	_		_	_
Critical Hdwy Stg 2	6.3	5.7	-	5.5	4.9	-	-	-	-	-	-	-
Follow-up Hdwy	3	4	3.1	3	4	3.1	3	_	_	3	_	_
Pot Cap-1 Maneuver	689	573	941	748	618	936	1060	_	-	1044	_	_
Stage 1	965	760	-	984	779	-		-	_	_	-	-
Stage 2	944	748	-	1004	788	_	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	685	570	938	747	615	936	1057	-	-	1044	-	-
Mov Cap-2 Maneuver	685	570	-	747	615	-	-	-	-	-	-	-
Stage 1	962	756	-	984	779	_	-	-	-	-	-	-
Stage 2	943	748	-	1002	784	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			8.9			0			0.1		
HCM LOS	A			A								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1057	-	-	-	936	1044	-	-			
HCM Lane V/C Ratio		-	-	-	_	0.001		-	_			
HCM Control Delay (s)		0	-	-	0	8.9	8.5	0	-			
HCM Lane LOS		A	-	-	A	Α	Α	A	_			
HCM 95th %tile Q(veh)		0	-	-	-	0	0	-	-			
.,,												

Control Type: Unsignalized

	•	\rightarrow	•	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	f)	
Traffic Volume (vph)	10	0	0	139	125	11
Future Volume (vph)	10	0	0	139	125	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	11	11	11	11
Grade (%)	-1%			-1%	1%	
Storage Length (ft)	0	0	0			0
Storage Lanes	1	0	0			0
Taper Length (ft)	25		25			
Link Speed (mph)	25			25	25	
Link Distance (ft)	239			378	349	
Travel Time (s)	6.5			10.3	9.5	
Confl. Peds. (#/hr)		1				
Confl. Bikes (#/hr)						
Peak Hour Factor	0.80	0.80	0.80	0.80	0.80	0.80
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	0	174	170	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2073138\PM Files.syn

Intersection						
Int Delay, s/veh	0.3					
	EBL	EDD	NDI	NDT	CDT	SBR
Movement		EBR	NBL	NBT	SBT	SRK
Lane Configurations	**	۸	0	4	♣	44
Traffic Vol, veh/h	10	0	0	139	125	11
Future Vol, veh/h	10	0	0	139	125	11
Conflicting Peds, #/hr	0	1	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	-1	1	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	0	0	0	1	0	0
Mvmt Flow	13	0	0	174	156	14
Major/Minor N	Minor2	N	//ajor1	N	Major2	
Conflicting Flow All	337	164	170	0		0
Stage 1	163	_	_	-	_	-
Stage 2	174	_	-	_	_	_
Critical Hdwy	6.3	6.1	4.3	_	_	_
Critical Hdwy Stg 1	5.2	-	-	_	_	_
Critical Hdwy Stg 2	5.2	_	_	_	_	_
Follow-up Hdwy	3	3.1	3	_	_	_
Pot Cap-1 Maneuver	763	943	1050	_	_	_
Stage 1	1014	-	-	_	_	_
Stage 2	1003	_	_	_	_	_
Platoon blocked, %	1000			_	_	_
Mov Cap-1 Maneuver	763	942	1050			
Mov Cap-1 Maneuver	763	342	1030	_	_	_
Stage 1	1014	-	-	-	-	-
ŭ .	1014			_	-	
Stage 2	1003	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.8		0		0	
HCM LOS	Α					
Minor Long/Major Mare	+	NDI	NDT	EBLn1	CDT	CDD
Minor Lane/Major Mvm	l e	NBL			SBT	SBR
Capacity (veh/h)		1050	-		-	-
HCM Lane V/C Ratio		-		0.016	-	-
LIOM O. IL ID I ()			_	9.8	_	-
HCM Control Delay (s)		0				
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	А	-	-

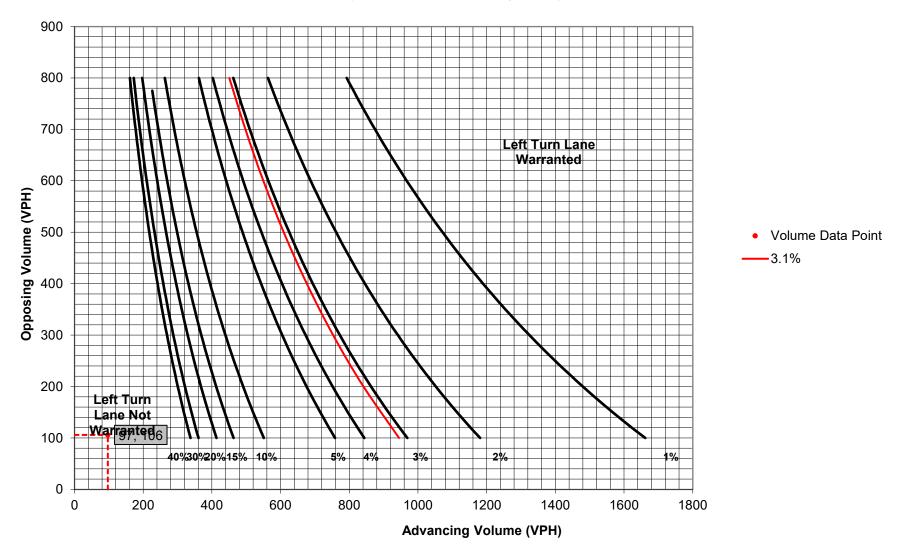
APPENDIX F

AUXILIARY TURN LANE WARRANT ANALYSES

		STUDY LOC	ATION AN	D ANALYS	IS INFORM	1ATION					
				_			2/22/				
ľ	Municipality:		en Township		Analysis		6/23/2				
	County:		and County		Conduct		DZ				
PennDOT Enginee	ering District:		8			ed By:	JW				
					ncy/Company	Name: Ira	affic Planning a	and Design, Inc.			
Intersection & Approach	Description:	Mill Road & Pro	posed Site Driv	eway - Southbo	ound Left						
Ana	alysis Period:	2036 Proje	ected (Build)		Number of Approach Lanes: 1						
	Design Hour:		ak Hour		Undivided or Divided Highway: Undivided						
	tion Control:		nalized								
Posted Speed			25					pe of Analysis			
Тур	e of Terrain:	Ro	lling		Left or Right-1	Turn Lane An	alysis?:	eft Turn Lane			
			VOLUME	CALCULAT	TIONS						
		L	eft Turn Lane	e Volume Cal	culations						
Movement	Include	? Volume	% Trucks	PCEV							
Left	t Yes	2	2.0%	3		Ad	dvancing Volu	me: 97			
Advancing Throu	ıgh -	86	6.0%	94			pposing Volu				
Righ	it Yes	0	0.0%	0			eft Turn Volu				
Left	t Yes	0	0.0%	0							
Opposing Throu	ıgh -	100	4.0%	106							
Righ	t Yes	0	0.0%	0	% Lef	t Turns in Ad	dvancing Volu	me: 3.09%			
		Ri	ight Turn Lan	e Volume Ca	lculations						
Movement	Include		% Trucks	PCEV							
Left		0	0.0%	N/A	Advancing Volumes N/A						
Advancing Through		100	4.0% 0.0%	N/A N/A			dvancing Volu				
Kigii	-	0	0.076	IV/A		KI,	ght Turn Volu	me: N/A			
		TUI	RN LANE V	VARRANT	FINDINGS						
Left Turn	Lane Warra	ant Findings			Rig	ht Turn Lar	ne Warrant F	indings			
Applicable Warran	nt Figure:	Figure 1			Applicable \	Narrant Fig	ure: N	/A			
Warra	int Met?:	No				Warrant M	et?: N	/A			
		TURI	N LANE LEI	NGTH CAL	CULATION	S					
Intersec	tion Control:	Unsignaliz	ed								
Design Hour Volume of T	Turning Lane:	3									
Cycles Per Hou	r (Assumed):	60						_			
Cycles Per Hou	ır (If Known):			Average #	of Vehicles/Cy	cle:	N/A				
_			PennDOT Publ	lication 46, Exh	nibit 11-6			_			
					d (MPH)						
Ту	pe of Traffic Co	ontrol	25-35		0-45	5	0-60				
		High	Low	High	nand Volume Low	High	Low				
	Signalized		A	B or C	B or C	B or C	B or C				
	Unsignalize		А	С	В	B or C	В				
			Left Turn L	ane Storage L	ength, Condit	ion A:	N/A	Feet			
				-	Condit	ion B:	N/A	Feet			
					Condit	ion C:	N/A	Feet			
			Require	ed Left Turn La			N/A	Feet			
			•		•	-	•				
						Δ4di+iα	nal Findings	•			
						Additio	onal Findings: N/A				
Additional Comments / Justif	fications:					Additio					



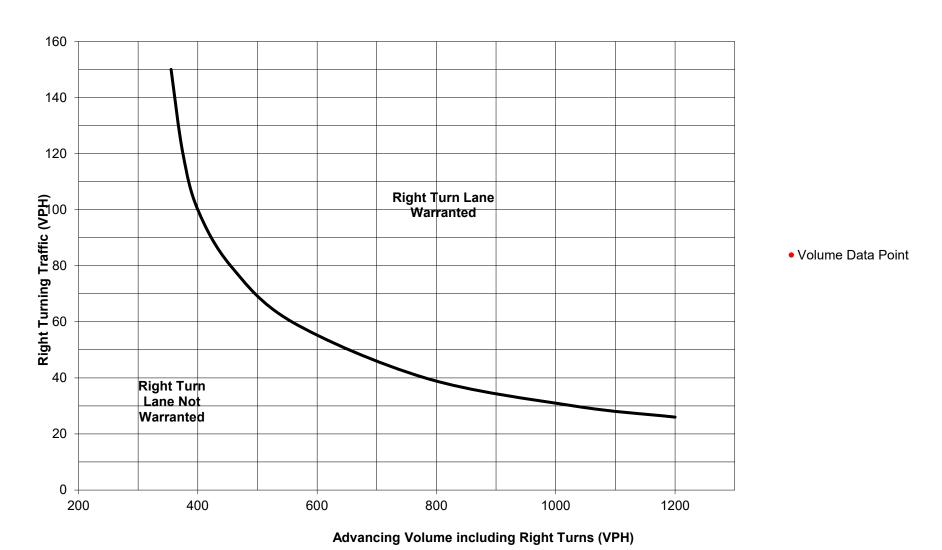
Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)



	CTLIDY I OO										
	STUDY LUC	ATION AN	D ANALYSI	IS INFORM	IATION						
Municipality:	Linner Alle	n Township		Analysis Date: 6/23/2023							
County:		ind County		Conduct		DZ					
PennDOT Engineering District:		8			-	JW					
Tellibot Engineering District.		0	Δger	Checked By: JW Agency/Company Name: Traffic Planning and Design, In-							
					tanic. 11a	ine i idining d	ina Design, i	ne.			
Intersection & Approach Description:	Mill Road & Prop	posed Site Drive	eway - Northbo	und Right							
Analysis Period: 2036 Projected (Build) Number of Approach Lanes: 1											
Analysis Period:					of Approach		1				
Design Hour: Intersection Control:		ak Hour		Undivided o	or Divided Hig	ghway:	Undivided				
	8	nalized 25				Too	oo of Analy	ala			
Posted Speed Limit (MPH): Type of Terrain:		lling		Type of Analysis Left or Right-Turn Lane Analysis?: Right Turn Lane							
Type of Terrain.	Roi	IIII B		Left of Right-1	ann Lanc And	arysis: . Thi	giit Tuili Lui	ic			
		VOLUME	CALCULAT	IONS							
	Le	eft Turn Lane	Volume Calc	culations							
Movement Include	? Volume	% Trucks	PCEV								
Left Yes	2	2.0%	N/A		hΔ	vancing Volui	me.	N/A			
Advancing Through -	86	6.0%	N/A			pposing Volu		N/A			
Right Yes	0	0.0%	N/A			pposing volui eft Turn Volui		N/A			
Left Yes	0	0.0%	N/A			cit iuiii voiui		14/71			
Opposing Through -	100	4.0%	N/A								
Right Yes	0	0.0%	N/A	% Lef	t Turns in Ad	vancing Volu	me:	N/A			
	Ri	ght Turn Lan	e Volume Cal	culations		-					
Movement Include		% Trucks	PCEV								
Left Yes	0	0.0%	0								
Advancing Through -	100	4.0%	106		hΑ	vancing Volui	me:	106			
Right -	0	0.0%	0	Right Turn Volume: 0							
<u> </u>											
	TUR	RN LANE W	/ARRANT F	INDINGS							
Left Turn Lane Warr	ant Findings			Rigl	nt Turn Lan	e Warrant F	indings				
Applicable Warrant Figure:	N/A			Applicable V	Varrant Figu	re: Figu	ıre 9				
Warrant Met?:	N/A	Warrant Met?: No									
			10711 0110								
	TURN	N LANE LEN	NGTH CALC	CULATIONS	S						
Intersection Control:		ed									
Design Hour Volume of Turning Lane:											
Cycles Per Hour (Assumed): Cycles Per Hour (If Known):	60		Average # o	of Vehicles/Cyc	٠١٥٠	N/A	1				
cycles i ei modi (ii known).			· ·	• •	.ic.	14//	ı				
		PennDOT Publ	ication 46, Exhi	ibit 11-6 d (MPH)			1				
	ontrol 25-35			0-45	50)-60					
Type of Traffic C				Turn Demand Volume		30-00					
	High	Low	High	Low	High	Low]				
Signalized		A	B or C	B or C	B or C	B or C					
Unsignalize	ed A	А	С	В	B or C	В	<u></u>				
		ane Storage Le	ength, Condit	ion A:	N/A	Feet					
			Condit	ion B:	N/A	Feet					
						N/A	Feet				
	Required Ri						Right Turn Lane Storage Length: N/A Feet				
		Kequirea		_							
		Kequirea			Additio						
		Kequirea			Additio	nal Findings: N/A					
Additional Comments / Justifications:		Kequired			Additio	nal Findings:					



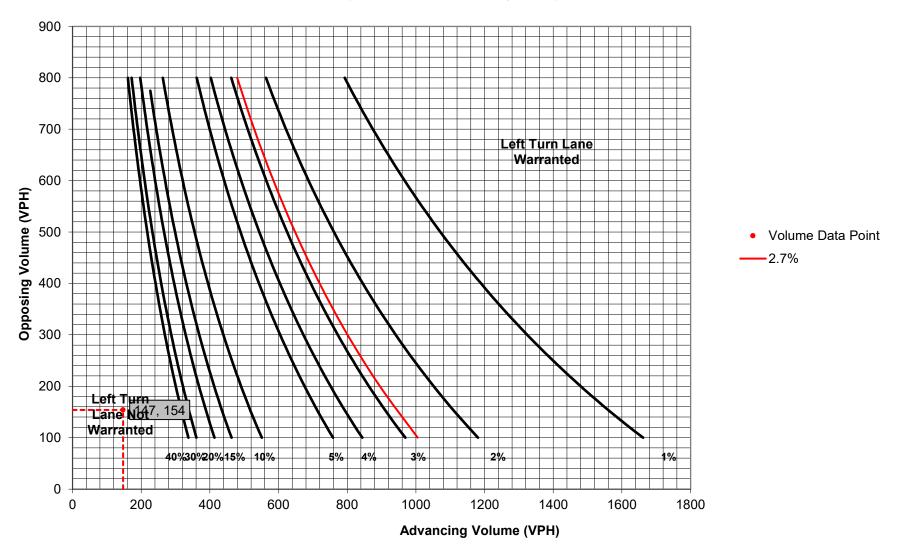
Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



	STUDY LOC	ATION AN	D ANALYSI	S INFORM	ATION					
• •	Municipality: Upper Allen Township County: York County Engineering District: 8			Analysis Date: 6/23/2023 Conducted By: DZ Checked By: JW						
Intersection & Approach Description	Mill Road & Pro	nosed Site Driv		und Left	lame: Traffic Planning	g and Design, Inc.				
intersection & Approach Description	. Willi Road & FTO	posed Site Dilv	eway - 30utilbot	unu Leit						
Analysis Period Design Hour Intersection Control	PM Pe	ected (Build) ak Hour nalized		Number of Approach Lanes: 1 Undivided or Divided Highway: Undivided						
Intersection Control: Unsignalized Posted Speed Limit (MPH): 25 Type of An Type of Terrain: Rolling Left or Right-Turn Lane Analysis?: Left Turn										
		VOLUME	CALCULATI	IONS						
	L	eft Turn Lane	Volume Calc	ulations						
Movement Include	? Volume	% Trucks	PCEV							
Left Yes	3	2.0%	4		Advancing Vol	ume: 147				
Advancing Through -	140	1.0%	143		Opposing Vol					
Right Yes	0	0.0%	0		Left Turn Vol	ume: 4				
Left Yes Opposing Through -	0 151	0.0% 1.0%	0 154							
Right Yes	0	2.0%	0	% Left	Turns in Advancing Vol	ume: 2.72%				
Right Turn Lane Volume Calculations										
Movement Include	? Volume	% Trucks	PCEV			_				
Left Yes	0	0.0%	N/A							
Advancing Through -	151	1.0%	N/A		Advancing Vol					
Right -	0	0 2.0% N/A Right Turn Volume: N/A								
	TUF	RN LANE W	VARRANT F	INDINGS						
Left Turn Lane Warr	ant Findings			Righ	t Turn Lane Warrant	Findings				
Applicable Warrant Figure:	Figure 1]		Applicable Warrant Figure: N/A						
Warrant Met?:	No]	Warrant Met?: N/A							
	TURI	N LANE LEI	NGTH CALC	ULATIONS						
Intersection Control	Unsignalize	-d								
Design Hour Volume of Turning Lane										
Cycles Per Hour (Assumed)	60					_				
Cycles Per Hour (If Known)			Average # o	of Vehicles/Cyc	e: N/A					
		PennDOT Publ	ication 46, Exhi			_				
		25.25		I (MPH)	F0.60	50-60				
Type of Traffic C	ontrol 25-35		40-45 Turn Demand Volume		30-00					
	High	Low	High	Low	High Low					
Signalized Unsignalized		A A	B or C	B or C	B or C B or C	\dashv				
		•	*	*		— Teast				
		Leit Turn L	ane Storage Le	engtn, Conaiti Conditi	•	Feet				
				Conditi	on B: N/A	IFEEL				
					on C. NI/A					
				Conditi	•	Feet				
		Require	ed Left Turn La	Conditi						
	_	Require	ed Left Turn La	Conditi	ngth: N/A Additional Finding	Feet Feet				
Additional Comments / Justifications:		Require	ed Left Turn La	Conditi	ngth: N/A	Feet Feet				



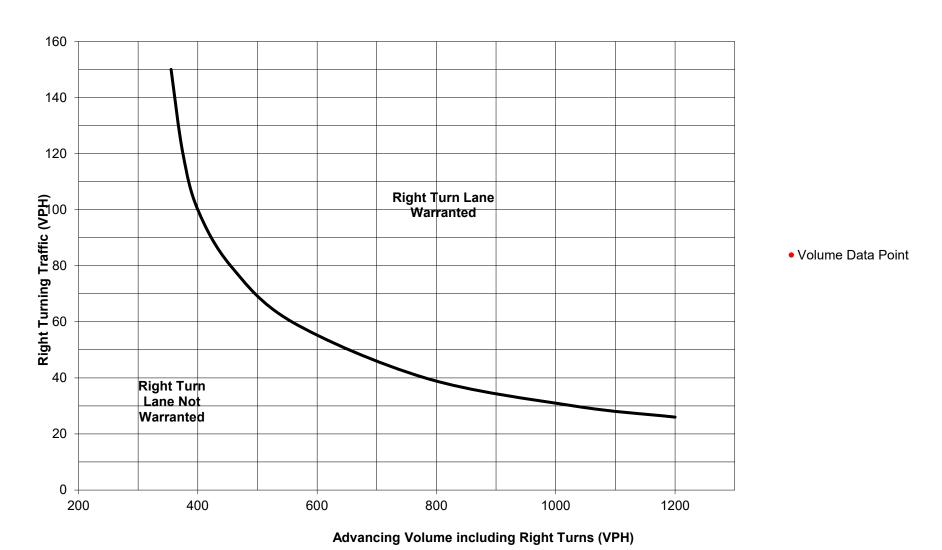
Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)



			STUDY	LOCA	ATION AN	ND A	ANALYS	IS INFORM	IATION				
										6 /22 /3	2022		
Municipality					Township			Analysis		6/23/2023			
County				York Co	-		Conducted By:			DZ JW			
PennDOT Engineering District				8				Check	-			Inc	
								ncy/Company	Name:	affic Planning a	ind Design	, Inc.	
Intersection & A	pproach De	escription:	Mill Road	& Prop	osed Site Driv	vewa	y - Northbo	ound Right					
	Analy	2036 Projected (Build)				Number of Approach Lanes: 1							
		sign Hour:	PM Peak Hour					Undivided o	r Divided H	ighway:	Undivided	t	
	Intersectio		B										
Poste	d Speed Lir	nit (MPH):		25							pe of Anal		
	Туре	of Terrain:		Roll	ing		Left or Right-Turn Lane Analysis?: Right Turn Lane						
					VOLUME	CA	LCULAT	IONS					
				Le	ft Turn Lan	ie Vo	lume Cal	culations					
Moveme	nt	Include	? Vol	ume	% Trucks	F	PCEV						
	Left	Yes		3	2.0%		N/A		A	dvancing Volu	me:	N/A	
Advancing	Through	-	1	40	1.0%		N/A			Opposing Volu		N/A	
-	Right	Yes		0	0.0%		N/A			Left Turn Volu		N/A	
	Left	Yes		0	0.0%	-	N/A						
Opposing	Through	-	1	51	1.0%		N/A						
	Right	Yes		0	2.0%		N/A	% Lef	t Turns in A	dvancing Volu	me:	N/A	
				Rig	ht Turn Lar	ne V	olume Ca	lculations					
Moveme		Include		ume	% Trucks	F	PCEV						
Left Yes				0	0.0%		0						
Advancing Through - Right -		-	_	51 0	1.0% 2.0%		0	Advancing Volume: 154 Right Turn Volume: 0					
	MgHt		0	2.070		U							
				TUR	N LANE V	NAI	RRANT	FINDINGS					
Le	eft Turn La	ane Warra	ant Findi	ngs				Rigl	nt Turn Lai	ne Warrant F	indings		
Applicable Warrant Figure: N/A Applicable Warrant Figure 9													
	Warrant	Met?:	N/A						Warrant M	et?:	lo]	
			•	TURN	LANE LE	NG	TH CALC	CULATIONS	S				
	Intersectio	n Control:	Unsi	ignalized	d								
Design Hour Volu	ıme of Tur	ning Lane:		0									
Cycles Per Hour (Assumed): 60													
Cycles	Per Hour (lf Known):				1	Average # o	of Vehicles/Cyc	:le:	N/A			
				P	ennDOT Pub	olicati	ion 46, Exh	ibit 11-6			_		
					Speed (MPH)								
Type of Traffic Cont		ontrol 25-35 High Low		L	40-45 Turn Demand Volume High Low		5	0-60					
							High Low		1				
		Signalized		A	A	$\neg \dagger$	B or C	B or C	B or C	B or C	1		
		Unsignalize	d	Α	А		С	В	B or C	В]		
Right Turn Lane Storage Length, Condition A: N/A Feet													
Condition B: N/A Feet													
Condition C: N/A Feet													
Required Right Turn Lane Storage Length: N/A Feet													
					4 6	6	,						
Additional Findings: N/A													
Additional Commen	ts / Justifica	tions:									ı		



Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



APPENDIX GPROJECT CORRESPONDENCE



WWW.TRAFFICPD.COM

July 21, 2023

Ms. Jennifer Boyer Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

Re: Response to Township's 7/12/23 Board of Commissioners Comments

2509 Mill Road Townhouses

TPD# RJFA.00006

Mrs. Boyer:

On behalf of the Applicant (Mihail Malinov), Traffic Planning and Design, Inc. (TPD) is resubmitting this Traffic Impact Study (TIS) application package related to transportation improvements associated with the proposed 2509 Mill Road Townhomes Development in Upper Allen Township.

Response to Transportation Resource Group's (TRG) July 12, 2023, Commissioners review letter

For the discussion below, TRG's comments from the 07/12/23 review letter are shown in italics, with the corresponding TPD responses in bold type.

Traffic Impact Study

7. The Applicant was required to submit a traffic study. Section 220-3.7.F(2) requires a traffic study for residential developments containing 20 or more homes. Section 220-3.7.F(3) states, "The Township Zoning Officer, Township Engineer or its designee, the Planning Commission or the Board of Commissioners determines there are existing traffic problems in the area or that the existing transportation network may be inadequate to handle the volume or character of traffic likely to result from the proposed land development, a traffic impact report can be required." The Police Chief recommended a traffic study be performed. The Board of Commissioner also determined a traffic impact report was required.

During the public meetings, residents expressed concerns about the existing traffic congestion in the area, for several reasons. First, they said Mill Road is well-traveled by Messiah University staff, faculty, and visitors. Second, they said there has been an increase in new townhomes built along the road, in what has previously been a mature neighborhood of single-family detached homes. Finally, residents expressed concerns about the narrow width of the road and lack of a shoulder, questioning pedestrian safety and the safety of children who wait for the school bus.

On June 26, 2023, a traffic impact study was prepared. The following intersection were studied: West Lisburn Road & Mill Road; Mill Road & the proposed site driveway; Mill Road & Wingert Drive; and Mill Road & Sholly Drive. No intersection operated or would operate below a LOS C; therefore, no additional improvements are required. The Applicant will design their driveway to provide full access movements, as presented. Curbs and sidewalks will be provided on-site.

Township Engineer/Staff Comments:

- o The trip generation estimates were based on data found in the ITE Trip Generation Manual.
- The sight distance and adequacy of study intersection to accommodate anticipated traffic volumes and the impact on pedestrians was not identified in the study, as requested in the scope review. The Applicant has argued the evaluation is not required as part of Section 220-3.7.F. of the SLDO. Sight distance analysis is provided in the TIS at the site driveway, and the applicant sight distance requirements ae met for the posted speed limit.

The TIS evaluated intersection capacity (i.e. level of service, 95th percentile queues) at the study area intersections to determine their adequacy to accommodate the existing and anticipated traffic volumes. The results of the analysis indicate that all study area intersection approaches are anticipated to operate under an acceptable level of service of C or better and all 95th percentile queue lengths are well within the available storage with full build-out of the 2509 Mill Road Townhome development.

Given the very small size of the development and minimal peak hour trip generation (2 A.M. trips and 4 P.M. peak hour trips) development traffic is not anticipated to have any measurable impact on pedestrians/pedestrian facilities. Sidewalks do not currently exist throughout the study area. In conjunction with the 2509 Mill Road Townhome development, sidewalks will be provided along the Mill Road site frontage and internally throughout the site.

Sight distances were evaluated at the proposed site driveway intersection as applicable per Township Ordinance requirements (PennDOT's SSSD) and also compared to PennDOT's Desirable sight distance requirements, which are more stringent than SSSD. The measured sight distances at the site driveway exceed applicable SSSD and Desirable criteria.

• A safety analysis of Mill Road addressing the saft stopping sight distance for school buses and student pick-up/drop-off, was not identified in the study, as requested in the scope review. The Applicant will be installing sidewalk on-site, which will provide students a place to stand off the street during pick-up and drop off.

Based on the sight distance evaluation provided in the TIS, the measured sight distances at the site driveway exceed applicable SSSD and Desirable criteria for passenger vehicles and single-unit trucks (school bus). Upon review of the Mechanicsburg Area School District (MASH) current school bus routes, stops occur within the study area along Mill Road at 2500 Mill Road, 2509 Mill Road (Proposed Site Location), Mill Road/Sholly Road and Mill Road/Wingert Drive. The Mill Road corridor does not currently have sidewalks; however, the proposed development will be providing sidewalks along the Mill Road site frontage and throughout the site to accommodate students. The sidewalks will provide an adequate refuge area for students to be picked up/dropped off at the 2509 Mill Road driveway intersection.

Sincerely,

TRAFFIC PLANNING AND DESIGN, INC.

Jason T. Wheeler, PTP Project Manager

Jwheeler@TrafficPD.com

Craig Mellott, P.E., PTOE

Vice President

CMellott@Trafficpd.com



WWW.TRAFFICPD.COM

June 26, 2023

Ms. Jennifer Boyer Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

Re: 2509 Mill Road Townhomes

Response to Township TIS Scoping Application Review 1

Scoping Application #S0820220094;

Upper Allen Township, Cumberland County, PA

TPD# RJFA.00006

Dear Boyer:

On behalf of the Applicant, Traffic Planning and Design, Inc. (TPD) is responding to the Township traffic engineer's (TRG) scoping comments on the above-referenced project.

Response to Upper Allen Township's February 7, 2023 Review Letter:

For the discussion below, Upper Allen Township comments from TRG's 02/07/2023 TIS Scoping review email are shown in italics, with the corresponding TPD responses in bold type.

1. Trip generation for the proposed 14 unit townhouse development should be in accordance with ITE Trip Generation Manual, 11th Edition.

The TIS has been prepared using the ITE Trip Generation Manual, 11th Edition.

- 2. The traffic study should include the following study intersections:
 - a. Mill Road / Pheasant Drive (Proposed Access)
 - b. Mill Road / West Lisburn Road
 - c. Mill Road / Wingert Drive
 - d. Mill Road / Sholly Drive

The TIS includes the above-referenced intersections.

3. Traffic counts should be conducted from 6am – 9am and from 3pm – 6pm. Traffic counts should be conducted when the construction on Mill Road is complete.

The traffic counts were conducted in early May 2023 after completion of construction on Mill Road.

4. The scenarios studied should include Existing, Base (without Development), and Projected (with development). Base and Projected should 10 years beyond the construction of the proposed development.

The TIS analyzes the scenarios noted above.

5. A 0.59% per year growth rate should be used to factor existing traffic volumes to the Base and Projected year analysis.

The TIS includes the background growth rate noted above.

6. The study should evaluate sight distance and the adequacy of study intersections to accommodate the anticipated traffic volumes and the impact on pedestrians.

The TIS includes sight distances analysis at the site driveway intersection. LOS and queue analyses have been prepared at the study area intersection in accordance with Township ordinances. Given the very small size of the development, development traffic is not anticipated to have an impact on pedestrians/pedestrian facilities.

7. The study should include a safety analysis of Mill Road addressing the safe stopping sight distance for school buses and student pick-up/drop-off.

Based on a review of §220-3.7 of the Township SALDO, the requested evaluation does not appear to be required for Traffic Impact Studies. Sight distance analysis is provided in the TIS at the site driveway, and applicable sight distance requirements are met for the posted speed limit.

8. The intersection of Mill Road / West Lisburn Road should be studied and recommendations made for improvements as necessary.

This off-site intersection is included in the TIS. Based on the results of the TIS, improvements are not required at this intersection in conjunction with the development based on the LOS/queuing standards in the Township SALDO.

9. The study should include capacity analysis and turn lane analysis.

The TIS includes capacity analysis at the study area and turn lane analysis at the proposed site driveway.

10. Traffic volumes from the 151-237 Gettysburg Pike Development should be included as background traffic in the study. Traffic volume figures for this development are attached.

The requested development has been included in the TIS under base and projected conditions.

If you have any questions or require additional information to process this application, please call anytime.

Sincerely,

TRAFFIC PLANNING AND DESIGN, INC.

Craig Mellott, P.E., PTOE

Vice President

CMellott@Trafficpd.com

Zerphey, Dylan

From: Chris Schwab <cschwab@consulttrg.com>
Sent: Tuesday, February 7, 2023 10:33 AM

To: Lenker, Drake

Cc: Mellott, Craig; ddw@rjfisherengineering.com; Jennifer Boyer

Subject: RE: 2509 Mill Road Townhomes - Scope of Work

Attachments: Traffic Volumes for 151-237 Gettysburg Pike Development.pdf

CAUTION: External email - do not click links or open attachments unless you recognize the sender and know the content is safe.

Drake,

We reviewed the proposed scope of work for the traffic impact study for the proposed townhouse development located at 2509 Mill Road and have the following comments:

- 1. Trip generation for the proposed 14 unit townhouse development should be in accordance with ITE Trip Generation Manual, 11th Edition.
- 2. The traffic study should include the following study intersections:
 - a. Mill Road / Pheasant Drive (Proposed Access)
 - b. Mill Road / West Lisburn Road
 - c. Mill Road / Wingert Drive
 - d. Mill Road / Sholly Drive
- 3. Traffic counts should be conducted from 6am 9am and from 3pm 6pm. Traffic counts should be conducted when the construction on Mill Road is complete.
- 4. The scenarios studied should include Existing, Base (without Development), and Projected (with development). Base and Projected should 10 years beyond the construction of the proposed development.
- 5. A 0.59% per year growth rate should be used to factor existing traffic volumes to the Base and Projected year analysis.
- 6. The study should evaluate sight distance and the adequacy of study intersections to accommodate the anticipated traffic volumes and the impact on pedestrians.
- 7. The study should include a safety analysis of Mill Road addressing the safe stopping sight distance for school buses and student pick-up/drop-off.
- 8. The intersection of Mill Road / West Lisburn Road should be studied and recommendations made for improvements as necessary.
- 9. The study should include capacity analysis and turn lane analysis.
- 10. Traffic volumes from the 151-237 Gettysburg Pike Development should be included as background traffic in the study. Traffic volume figures for this development are attached.

Let me know if you have any questions. Thank you.

Chris

Christopher E. Schwab, PE, TRG, Senior Associate Email: cschwab@consulttrg.com

PA Office: (717) 846-4660 MD Office: (443) 275-2344

Cell: (717) 683-6388 <u>www.consulttrg.com</u> From: Lenker, Drake <dlenker@trafficpd.com> Sent: Wednesday, February 1, 2023 10:35 AM To: Chris Schwab <cschwab@consulttrg.com>

Cc: Mellott, Craig <cmellott@trafficpd.com>; ddw@rjfisherengineering.com

Subject: 2509 Mill Road Townhomes - Scope of Work

Good morning Chris, we just spoke on the phone discussing 2509 Mill Road Townhomes project in Upper Allen Township, Cumberland County just south of Lisburn Road. The proposed project is for the demolition of the one (1) existing single-family home and construction of 14 townhomes. Access to the site will remain utilizing the existing Pleasant Drive to Mill Road. Based on the proposed 14 single-family attached townhomes the trip generation following ITE will have a minimal traffic impact on the local roadways. Cumberland County has a growth rate factor of 0.59% that will be utilized to develop the base and projected volumes of the development. Upper Allen Township is requesting to study Mill Road (T-610) and Pheasant Drive (Proposed Access) & W Lisburn Road (SR 2004) and Mill Road during the (6:00-9:00) AM and (3:00-6:00) PM peak hours during the Existing/Base (without development)/Projected (with development) analysis years. The township requires sight distance measurements to be reviewed at the site access to Mill Road. The township is also, is looking for recommendation at Mill Road and Lisburn Road. Please identify if the township is aware of any nearby developments in the vicinity of the proposed site. Thank you for your time and I look forward to hearing from the Upper Allen Townships feedback.

Drake

Drake Lenker, Transportation Planning Specialist



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