







# **Transportation Impact Study**Genius Kids Child Development Center *Upper Allen Township, Cumberland County*

**For Submission To:** Upper Allen Township

## GENIUS KIDS CHILD DEVELOPMENT CENTER TRANSPORTATION IMPACT STUDY

FOR SUBMISSION TO:

Upper Allen Township, Cumberland County, PA

Prepared For:

**BL** Companies

2601 Market Place, Suite 360 Harrisburg, PA 17110 November 17, 2023

TPD # BLC.00127



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#### **EXECUTIVE SUMMARY**

The purpose of this study is to examine the potential traffic impact associated with the proposed Genius Kids Child Development Center along Old Schoolhouse Lane in Upper Allen Township, Cumberland County, PA. Based on this evaluation, the following conclusions were reached:

- 1. This report has been prepared in accordance with PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's *Publication 282*, *Appendix A*, dated September 2022 and with Upper Allen Township's Subdivision and Land Development Ordinance § 220-3.7.F.
- 2. The project scope and the extent of the study area were based on the contents of the TIS Scoping Memo and the associated feedback from representatives of Upper Allen Township. The approved study area intersections included in this TIA are as follows:
  - » Cumberland Parkway & Old Schoolhouse Lane (West);
  - » Cumberland Parkway & Old Schoolhouse Lane (East);
  - » Old Schoolhouse Lane & Proposed Site Driveways.
- 3. The proposed development is located on the northern side of Old Schoolhouse Lane, in the vacant parcel east of the Wylie Dental Group building. The project involves construction of an 8,500 square foot Genius Kids Child Development Center.
- 4. Access to the development is proposed via two (2) full-movement driveway connections to Old Schoolhouse Lane.
- 5. The available sight distances at the proposed site driveways to Old Schoolhouse Lane exceed PennDOT's sight distance criteria.
- 6. Based on ITE trip generation data, the proposed development is anticipated to generate **53 new vehicle-trips** during the weekday A.M. peak hour and **53 new vehicle-trips** during the weekday P.M. peak hour.
- 7. Capacity analyses were conducted to determine the quality of operation (LOS) at the study area intersections for the 2023 existing, 2025 and 2035 base (no-build), and 2025 and 2035 projected (build) conditions. The capacity analyses were conducted in accordance with the standards contained in Appendix A Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT *Publication 282*, dated September 2022.
- 8. Levels of Service (LOS) for the study area intersections have been summarized in matrix form. **Table**I details the overall intersection LOS for each study area intersection.

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TABLE I
OVERALL INTERSECTION LEVEL OF SERVICE SUMMARY

	_	Ov	м					
Intersection	Time Period	Time		dout Year	2035 Ho	rizon Year	Meets LOS Requirements?	
	renou	Existing	Base	Projected	Base	Projected	Requirements:	
Cumberland Parkway &	A.M.	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	YES	
Old Schoolhouse Lane (West)	P.M.	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	YES	
Cumberland Parkway &	A.M.	A (0.3)	A (0.3)	A (0.8)	A (0.3)	A (0.8)	YES	
Old Schoolhouse Lane (East)	P.M.	A (0.4)	A (0.4)	A (0.8)	A (0.4)	A (0.8)	YES	
Old Schoolhouse Lane &	A.M.			A (4.0)		A (3.9)	YES	
Proposed Site Driveway (West)	P.M.			A (3.8)		A (3.8)	YES	
Old Schoolhouse Lane &	A.M.			A (1.3)		A (1.3)	YES	
Proposed Site Driveway (East)	P.M.			A (1.4)		A (1.3)	YES	

Base = No-Build scenario

Projected = Build scenario with development

- Under 2025 and 2035 projected (build) conditions, the study intersection will operate in accordance with the standards contained in Appendix A - Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT *Publication 282*, dated September 2022.
- 10. Traffic Planning and Design Inc. (TPD) recommends the following roadway improvements as outlined at the study area intersections:

#### **Cumberland Parkway & Old Schoolhouse Lane (West)**

» No improvements are necessary.

#### **Cumberland Parkway & Old Schoolhouse Lane (East)**

» No improvements are necessary.

#### **Old Schoolhouse Lane & Proposed Site Driveways**

- » Design the site driveways as full-movement driveways in accordance with Township standards.
- » Provide one entering lane and one exiting lane.
- » Provide a "Stop" sign, (PennDOT designation R1-1) on exiting driveway approaches.
- » Provide and perpetually maintain required sight lines per Township standards.
- » Provide sidewalk along the property frontage to Old Schoolhouse Lane and provide crosswalks and ADA ramps at the proposed site driveways.

As part of the Township's Land Development process, the applicant will coordinate and fund the implementation of the recommended roadway improvements. Additionally, all improvements will be constructed to accommodate non-motorized access/circulation and be ADA-compliant unless otherwise approved by the Township.

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#### INTRODUCTION

Traffic Planning and Design, Inc. (TPD) has completed a Transportation Impact Study (TIS) for the proposed Genius Kids Child Development Center in Upper Allen Township, Cumberland County, Pennsylvania. As shown in **Figure 1**, the proposed development is located on the northern side of Old Schoolhouse Lane, in the vacant parcel east of the Wylie Dental Group building. As shown in **Figure 2**, the project involves construction of an 8,500 square foot Genius Kids Child Development Center. Access to the development is proposed via two (2) full movement driveways to Old Schoolhouse Lane.

This report has been prepared in accordance with PennDOT's Policies and Procedures for Transportation Impact Studies, found in PennDOT's Publication 282, Appendix A, dated September 2022 and in accordance with Upper Allen Township Subdivision and Land Development Ordinance § 220-3.7.F. The project scope and the extent of the study area were based on the contents of the TIS Scoping Memo and the associated feedback from representatives of Upper Allen Township during the scoping process. All relevant correspondence pertaining to this project has been included in **Appendix A**.

#### **EXISTING ROADWAY NETWORK**

A field review of the existing roadway system in the study area was conducted. The existing roadway characteristics within the study area are summarized in **Table 1**.

TABLE 1
ROADWAY CHARACTERISTICS WITHIN STUDY AREA

Roadway	Ownership	Functional Classification/ Roadway Type	Predominant Directional Orientation	Posted Speed Limit	
Cumberland Parkway	Township	Minor Collector	East-West	35 mph	
Old Schoolhouse Lane	Township	Local Road	East-West	25 mph	

Details of the existing intersection controls, lane configurations, lane widths, and approach grades, as well as photographs of the study area intersections are included in **Appendix B**.

#### **Land Use Context**

In Publication 10X (Design Manual Part IX, Appendix B), there is guidance pertaining to defining the land use context(s) for a given area. Based upon review of this information, the land uses surrounding the proposed site best fits the **Suburban Neighborhood** designation, as described below:

**Suburban Neighborhood**, "Predominantly low-density residential communities with houses typically arranged along a curvilinear system of streets with limited connectivity to regional road networks. Neighborhoods can include community facilities (schools, churches, recreation) and some small businesses or offices."

#### **Roadway Type**

In Section 1.2 of the Design Manual, Part 2, there is guidance pertaining to defining the transportation context(s) for a given area. Comparing the existing condition roadway characteristics to the various options presented in Table 1.2 of the Design Manual, Part 2, the study area roadways best fit the following categories, as described below:

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**Neighborhood Collector**, traffic volumes of 6,000 or less vehicles per day, intersection spacing of 300 to 660 feet, a desired operating speed of 25-35 mph, and a description as follows: "Often similar in appearance to local roadways. Typically classified as "Minor Collector".""

» Cumberland Parkway

**Local Road**, traffic volumes of 3,000 or less vehicles per day, intersection spacing of 200 to 660 feet, and a desired operating speed of 20-30 mph.

» Old Schoolhouse Lane

#### **Bicycle and Pedestrian Facilities**

Based on observations during field visits at the study area, paved shoulders and/or travel lanes currently accommodate pedestrian and bicycle traffic in the vicinity of the development. As part of the proposed development, sidewalks are to be provided along the property frontage of Old Schoolhouse Lane and crosswalks and ADA ramps are to be provided at the proposed site driveways.

#### **Mass Transit Facilities**

Bus service in Cumberland County is provided via Rabit Transit, there are no bus stops in the study area. However, Winding Hill Park and Ride, served by route 15N, exists within a mile to the east of the proposed site and provides service between Harrisburg and Gettysburg.

#### **EXISTING TRAFFIC CONDITIONS**

#### **Manual Turning Movement Counts**

Manual traffic counts were conducted on 15-minute intervals during the weekday morning (6:00 to 9:00 A.M.) and weekday evening (3:00 to 6:00 P.M.) peak periods. Data pertaining to heavy vehicles, pedestrians and transit vehicles were observed during the manual counts. Peak hours and count dates for the study area intersections are identified in **Table 2**.

TABLE 2
MANUAL TRAFFIC COUNT INFORMATION

Intersection	Date of Traffic Counts	Time Period	Intersection Peak Hour <sup>1</sup>
Cumberland Parkway & Old Schoolhouse Lane (West)	Thursday October 12, 2022	Weekday A.M.	7:30 to 8:30 A.M.
	Thursday October 12, 2023	Weekday P.M.	4:15 to 5:15 P.M.
Cumberland Parkway & Old Schoolhouse Lane (East)	Thursday October 12, 2022	Weekday A.M.	7:30 to 8:30 A.M.
	Thursday October 12, 2023	Weekday P.M.	4:30 to 5:30 P.M.
Old Schoolhouse Lane &	Thomas de la Octobra 12, 2022	Weekday A.M.	7:45 to 8:45 A.M.
Proposed Site Driveways	Thursday October 12, 2023	Weekday P.M.	4:30 to 5:30 P.M.

<sup>&</sup>lt;sup>1</sup>Peak Hour consists of the four consecutive 15-minute intervals where the highest traffic volumes occur.

The existing condition traffic volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 3**. Manual traffic count data sheets are contained in **Appendix C**.

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#### **BASE (NO-BUILD) CONDITIONS**

#### **Annual Background Growth**

A background growth factor for the roadways in the study area was developed based on growth factors obtained from the PennDOT Bureau of Planning and Research (BPR) for September 2023 to July 2024. The PennDOT BPR suggests using a background growth trend factor of 0.54% per year in Cumberland County for urban non-interstate roadways. As such, the background growth factor was applied annually to yield an overall growth percentage of 1.01% (0.54 per year for 2 years) for 2025 opening year and of 1.07% (0.54 per year for 12 years) for 2035 horizon year.

#### **Nearby Proposed Developments**

No background developments were identified during the scoping process.

Base condition traffic volumes for the opening year 2025 and horizon year 2035 conditions for the weekday A.M. and P.M. peak hours are shown in **Figures 4 and 5**, respectively. Traffic volume development worksheets are contained in **Appendix D**.

#### SCHEDULED ROADWAY IMPROVEMENTS

#### **Programmed Improvements**

There are no proposed improvement projects at the study intersection.

#### **Nearby Development Improvements**

No background developments and/or improvements were identified during the scoping process.

#### **PROPOSED SITE ACCESS**

Access to the development is proposed via two (2) full-movement driveway connections to Old Schoolhouse Lane.

#### **Sight Distance Analysis**

A sight distance analysis was prepared for the proposed site driveway locations. In general, recommended safe sight distances depend upon the posted speed limit and roadway grades. The existing sight distances at the proposed driveway were measured in accordance with PennDOT Publication 282 <u>Highway Occupancy Permit Guidelines</u> and compared to PennDOT's and the Township's safe stopping sight distance standard, which is calculated by the following equation:

#### $SSSD = 1.47VT + V^2/[30(f\pm q)]$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Vehicle Speed

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements

g = Percent of Roadway Grade Divided by 100

**Table 3** shows the measured and PennDOT's/Township's acceptable sight distances (SSSD) for vehicles entering and exiting the site.

### TABLE 3 SIGHT DISTANCE ANALYSIS

		Posted		Sight Distances (feet)	
Movement	Direction	Speed (mph)	Grade <sup>1</sup>	SSSD	EXIST
	Old Schoolhouse Lane & Proposed	Site Access	s (West)		
Exiting	To the left	25	-1%	148′	250′
Movements	To the right	25	+1%	145′	500′+
Entering Left	Approaching same direction	25	+1%	145′	500′+
Turns	Approaching opposite direction	25	-1%	148′	280′
	Old Schoolhouse Lane & Propose	d Site Acces	s (East)		
Exiting	To the left	25	-1%	148′	155′
Movements	To the right	25	+1%	145′	500′+
Entering Left	Approaching same direction	25	+1%	145′	500′+
Turns	Approaching opposite direction	25	-1%	148′	155′

EXIST = Existing (measured) Sight Distance SSSD = PennDOT & Township Safe Stopping Sight Distance 1 = Roadway Grade Approaching Access

As shown in **Table 3**, the measured sight distances at the study intersection exceed applicable PennDOT and Township safe stopping sight distance (SSSD). Sight distance measurements are provided in **Appendix B**.

#### TRIP GENERATION

The trip generation rates for the proposed development were obtained from the manual *Trip Generation*, 11<sup>th</sup> Edition, an Institute of Transportation Engineers (ITE) Informational Report. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

The data for Land Use Code 565 (Day Care Center) was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour.

The following should be noted with respect to the trip generation methodology:

- » Based on the information provided in the manual *Trip Generation*, not all of the trips generated by the site will be "new" to the nearby roadway system. In addition to the "new trips" generated by the development, there will be "pass-by trips", which are trips that are drawn from the passing traffic stream and do not add trips to the adjacent roadways. Pass-by trip percentages were utilized as specified in the appendices of the manual *Trip Generation*, Eleventh Edition, 2021.
- Published data related to pass-by percentages are not available for Land Use Code #565 (Day Care Center) during the weekday A.M. peak hour. However, given the trip characteristics of a typical Day Care Center with parent drop-off/pick-up to and from there place of employment, etc. it is reasonable to assume that pass-by trips will occur during the weekday A.M. peak hour as well. Since ITE provides a weekday P.M. pass-by rate of 44% it is reasonable to assume that pass-by trips occur at the same rate during the weekday A.M. peak hour.

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**Table 4** shows the trip generation rates and directional split for the analyzed time periods.

TABLE 4
ITE TRIP GENERATION DATA

Land Use	ITE #	Time Period	Equation	Entering %	Exiting %	Pass-By %
		Average Weekday	T = 47.62*(X)	50%	50%	
Day Care Center 565	565	Weekday A.M. Peak Hour	T = 11.00*(X)	53%	47%	44% <sup>1</sup>
		Weekday P.M. Peak Hour	T = 11.12*(X)	47%	53%	44%

T = number of site-generated vehicular trips
 X = independent variable (ksf of Gross Floor Area)
 1 = ITE pass-by rates not published. See TPD justification above.

The calculated trip generation for the proposed development for the full build-out year is shown in **Table 5**.

TABLE 5
TRIP GENERATION SUMMARY

The Board	Total Trips		Pass-by Trips			New Trips			
Time Period	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit
Average Weekday	405	202	203						
Weekday A.M. Peak Hour	94	50	44	41	22	19	53	28	25
Weekday P.M. Peak Hour	95	44	51	42	20	22	53	24	29

Based on the trip generation analysis summarized in **Table 5**, the proposed development will generate approximately 53 new trips during the weekday A.M. peak hour and 53 new trips during the weekday P.M. peak hour at full build-out. Detailed trip generation calculations are contained in the approved scoping application provided in **Appendix A**.

#### TRIP DISTRIBUTION

#### **New Trips**

Trip distributions were determined based on existing traffic patterns within the study area, the most logical route of travel, and the location/configuration of the proposed site driveways. The new trips for the proposed development were distributed to the local roadway network based on the percentages shown in **Table 6** and presented graphically in **Figure 6A**.

TABLE 6
TRIP DISTRIBUTION PERCENTAGES

Direction - To/From	Assignment (To/From)	Distribution Percentage
East	via Cumberland Parkway	51%
West	via Cumberland Parkway	49%

#### **Pass-By Trips**

Pass-by trips for the /retail component of the proposed development were based on the existing traffic patterns in the vicinity of the site and the location and configuration of the site driveways. The pass-by trips for the proposed development were distributed to the local roadway network based on the percentages illustrated in **Figure 7A**.

Calculations for the proposed site trips are provided within the approved scoping memo and associated attachments in **Appendix A**. The distribution and assignment of site-generated trips for the proposed development during the weekday A.M. and weekday P.M. peak hours are shown in **Figures 6B, 7B,** and **8** for primary (new) trips, pass-by trips, and total development trips, respectively.

#### PROJECTED (BUILD) CONDITION TRAFFIC VOLUMES

The site-generated trips for the proposed development were added to the 2025 and 2035 base (no-build) condition traffic volumes to develop 2025 and 2035 projected (build) condition traffic volumes. Projected condition traffic volumes for full build-out opening year 2025 and horizon year 2035 for the weekday A.M. and weekday P.M. peak hours are shown in **Figures 9** and **10**. Traffic volume development worksheets are contained in **Appendix D**.

#### LEVELS OF SERVICE FOR AN INTERSECTION

For analysis of intersections, level of service is defined in terms of delay, which is a measure of driver discomfort and frustration, fuel consumption, and lost travel time. LOS criteria is stated in terms of control delay per vehicle for a one-hour analysis period. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The criteria are shown in **Table 7**. Delay, as it relates to level of service, is a complex measure and is dependent upon several variables. For signalized intersections, these variables include the quality of vehicle progression, the cycle length, the green time ratio, and the volume/capacity ratio for the lane group in question. For unsignalized intersections, delay is related to the availability of gaps in the flow of traffic on the major street and the driver's discretion in selecting an appropriate gap for a particular movement from the minor street (straight across, left or right turn).

TABLE 7

LEVEL OF SERVICE CRITERIA

UNSIGNALIZED AND SIGNALIZED INTERSECTIONS 1

Level of Service	Control Delay Per Vehicle (Seconds)					
Level of Service	Signalized	Unsignalized				
А	< 10	< 10				
В	> 10 and < 20	> 10 and < 15				
С	> 20 and < 35	> 15 and < 25				
D	> 35 and < 55	> 25 and < 35				
Е	> 55 and < 80	> 35 and < 50				
F	> 80 or v/c > 1.0	> 50 or v/c > 1.0				

 $1 = Obtained \ from \ Exhibits \ 19-8 \ and \ 20-2 \ of \ the \ Transportation \ Research \ Board's \ Highway \ Capacity \ Manual \ 6^{th} \ Edition$ 

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#### CAPACITY ANALYSIS METHODOLOGY

Capacity analyses were conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the 6<sup>th</sup> edition *Highway Capacity Manual* (HCM) using *Synchro 11* software (Version 11.1, Build 2, Revision 9), a CUBIC/Trafficware product.

The following conditions were analyzed, as applicable:

- » Existing conditions;
- 2025 Base conditions (Build-out year without development);
- » 2025 Projected conditions (Build-out year with development);
- » 2035 Base conditions (Analysis year without development);
- 2035 Projected conditions (Analysis year with development);

The following should be noted with respect to the capacity analyses:

- » The Pennsylvania default values for two-way stop controlled intersections in a suburban land use context contained in Chapter 10 of PennDOT's *Publication 46* were utilized for the base critical headway and base follow-up headways. Worksheets related to the calculated critical and follow-up headways are included at the beginning of **Appendix E**.
- Per PennDOT standards, a heavy vehicle percentage of 2% was utilized for all turning movements to/from the proposed site access locations.

The capacity analysis worksheets are included in **Appendix E**.

#### **PennDOT Standards**

PennDOT's Transportation Impact Study Guidelines outlined in PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's Publication 282, Appendix A, dated September 2022 contain the following criteria regarding levels of service:

- Page 32 of the Guidelines state that if evaluation of the With Development Horizon Year Scenario to the Without Development Horizon Year Scenario indicates that the overall intersection level of service has dropped, the applicant will be required to mitigate the level of service if the increase in overall intersection delay is greater than 10-seconds. If the overall intersection delay increase is less than or equal to 10-seconds, mitigation of the intersection will not be required. If the intersection level of service meets the level of service requirements, applicants may still be required to provide mitigation to address critical lanes or approaches. For locations where the level of service of the design horizon year without the development is LOS F and with development, the delay increases more than 10 seconds, the remedies shall provide an estimated delay which will be no worse than the delay for the design year without the development.
- Page 33 of the Guidelines state that for mitigation scenarios, applicants are expected to mitigate the overall intersection LOS to the original Without Development LOS; the 10-second delay variance is not applied to mitigation scenarios. Applicants may be required to address available storage and queue lengths at critical movements or approaches even if the overall LOS requirements are met.
- » Page 34 of the Guidelines state that if signalization is the preferred alternative for mitigation, overall intersection LOS C in rural areas and LOS D in urban areas is acceptable.

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Page 35 of the Guidelines states new signalized or unsignalized intersections established to serve as access to the development shall be designed to operate at minimum LOS C for rural areas, and minimum LOS D for urban areas.

#### **Upper Allen Township Standards**

Upper Allen Township's Transportation Impact Study Guidelines outlined in Upper Allen Township's Subdivision and Land Development Ordinance § 220-3.7.F contain the following criteria regarding levels of service:

Section (2)[c] of the Guidelines state that the study should assess impacts to adjacent roadways and identify improvements that may be required to maintain a level of service rating of D on the affected roadway network. Level of service E or F indicate the study should identify specific improvements to restore LOS to a rating of D or better.

#### LEVELS OF SERVICE IN THE STUDY AREA

Level of service (LOS) matrices for the study area intersections are shown in **Table 8** for the weekday A.M. and **Table 9** for the weekday P.M. peak hours.

TABLE 8
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY – AM PEAK HOUR

to to consider a	Maranasi	2023	2025 Ope	ening Year	2035 Horizon Year		
Intersection	Movement Existing	Existing	Base	Projected	Base	Projected	
Cumberland Parkway &	WB L/T	А	Α	Α	Α	А	
Old Schoolhouse Lane	NB L/R	В	В	В	В	В	
(West)	ILOS	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	
Cumberland Parkway &	WB L/T	А	А	Α	Α	А	
Old Schoolhouse Lane	NB L/R	В	В	В	В	В	
(East)	ILOS	A (0.3)	A (0.3)	A (0.8)	A (0.3)	A (0.8)	
Old Schoolhouse Lane &	EB L/T			Α		А	
Proposed Site Driveway	SB L/R			Α		А	
(West)	ILOS			A (4.0)		A (3.9)	
Old Schoolhouse Lane &	EB L/T			Α	-	А	
Proposed Site Driveway	SB L/R			Α		Α	
(East)	ILOS			A (1.3)		A (1.3)	

Base = No-Build scenario, Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

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TABLE 9
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY – PM PEAK HOUR

Intersection	Movement 2023		2025 Opening Year		2035 Horizon Year	
intersection	Movement	Existing	Base	Projected	Base	Projected
Cumberland Parkway &	WB L/T	А	Α	Α	Α	А
Old Schoolhouse Lane	NB L/R	В	В	В	В	В
(West)	ILOS	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)
Cumberland Parkway &	WB L/T	А	Α	Α	Α	А
Old Schoolhouse Lane	NB L/R	В	В	В	В	В
(East)	ILOS	A (0.4)	A (0.4)	A (0.8)	A (0.4)	A (0.8)
Old Schoolhouse Lane &	EB L/T			Α		А
Proposed Site Driveway	SB L/R			Α		А
(West)	ILOS			A (3.8)		A (3.8)
Old Schoolhouse Lane & Proposed Site Driveway	EB L/T			А		А
	SB L/R			Α		А
(East)	ILOS			A (1.4)		A (1.3)

Base = No-Build scenario, Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

As shown in **Tables 8 and 9**, under the 2025 and 2035 projected (build) conditions, the study area intersections will operate in accordance with the standards contained in Appendix A – Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT Publication 282, dated September 2022 and will meet Upper Allen Township SALDO requirements.

#### **95<sup>TH</sup> PERCENTILE QUEUE ANALYSIS**

Queue analyses were conducted at the study area intersections using *Synchro 11* software and reporting the HCM 6<sup>th</sup> Edition 95<sup>th</sup> percentile queue lengths. The queue analysis results are summarized in **Table 10** for the weekday AM and weekday PM peak hours.

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TABLE 10
HCM 95<sup>TH</sup> PERCENTILE QUEUE ANALYSIS

Movement	Available Storage Length <sup>1</sup>	2025 Base (No-Build)		2025 Projected (Build)		2035 Base (No-Build)		2035 Projected (Build)				
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.			
Cumberland Parkway & Old Schoolhouse Lane (West)												
WB L/T	300′	0'	0'	0'	0'	0'	0′	0′	0'			
NB L/R	500′+	3′	5'	8′	10′	3'	5'	8′	13′			
Cumberland Parkway & Old Schoolhouse Lane (East)												
WB L/T	460′	3'	3'	5′	5′	3'	3'	5'	5′			
NB L/R	200′+	3′	3′	5′	5′	3′	3′	5′	5′			
Old Schoolhouse Lane & Proposed Site Driveway (West)												
EB L/T	250′+			3′	3′			3'	3′			
SB L/R	100′+			3′	3′			3′	3′			
Old Schoolhouse Lane & Proposed Site Driveway (East)												
EB L/T	75′+			0'	0′			0'	0′			
SB L/R	100′+			0'	0'			0'	0′			

Base = No-Build Scenario

Projected = Build Scenario

Existing Storage/Proposed Storage

1 =Available storage measured to nearest public intersection.

As shown in **Table 10**, the 2025 and 2035 projected (build) condition queues will be accommodated within the existing/proposed storage length. Queue analysis worksheets are included with the capacity analysis worksheets provided in **Appendix E**.

#### **AUXILIARY TURN LANE ANALYSIS**

#### Methodology

TPD evaluated auxiliary turn lane warrants at the site access intersection of Old Schoolhouse Lane and the Proposed Western Site Driveway. The warrant analysis methodology contained within Chapter 11 of PennDOT's *Publication 46*, Section 11.17 was utilized for this evaluation. The auxiliary turn lane warrant analysis worksheets are contained in **Appendix F**.

#### **Findings**

**Table 11** summarizes the results of the auxiliary turn lane analysis at the site access intersections.

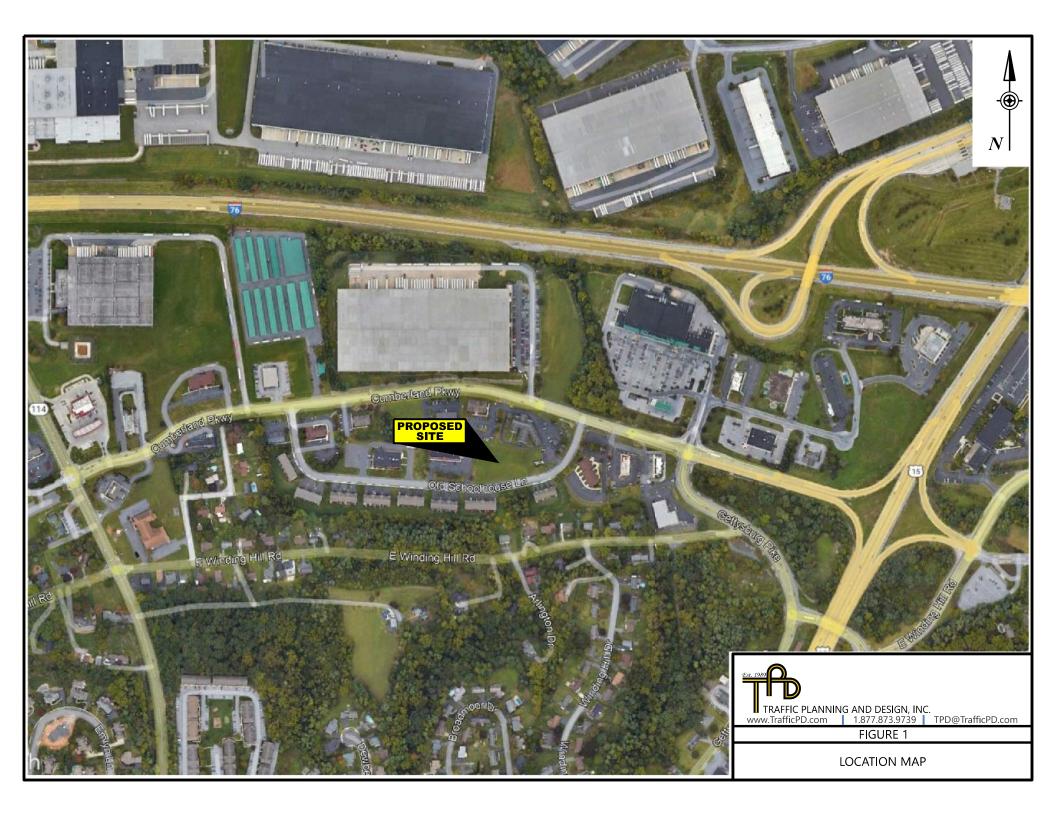
TABLE 11
AUXILIARY TURN LANE ANALYSIS SUMMARY

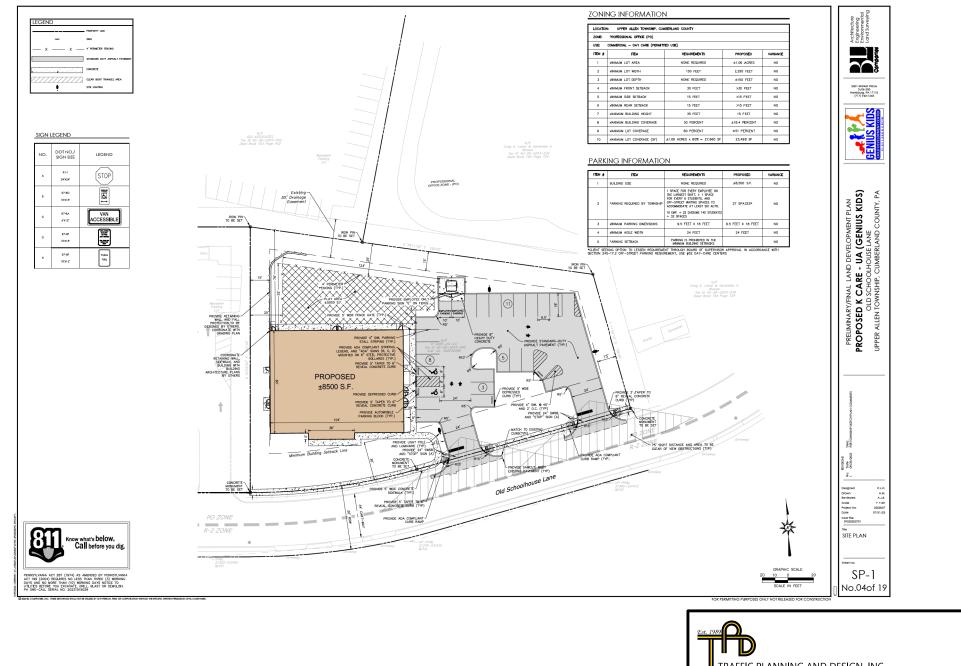
Intersection	Auxiliary Lane	Peak Hour	Warrant Satisfied?	Required Lane Length	Proposed Lane Length	
	M/D Dialet Total Land	A.M.	No			
Old Schoolhouse Lane &	WB Right-Turn Lane	P.M.	No			
Proposed Site Driveway (West)	EB Left-Turn Lane	A.M.	No			
		P.M.	No			
	) ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) (	A.M.	No			
Old Schoolhouse Lane &	WB Right-Turn Lane	P.M.	No			
Proposed Site Driveway (East)	EB Left-Turn Lane	A.M.	No			
		P.M.	No			

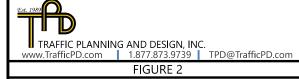
#### **RECOMMENDATIONS AND CONCLUSIONS**

The recommendations and conclusions for this Transportation Impact Study (TIS) are listed within the Executive Summary of this report.

Page 13— www.TrafficPD.com

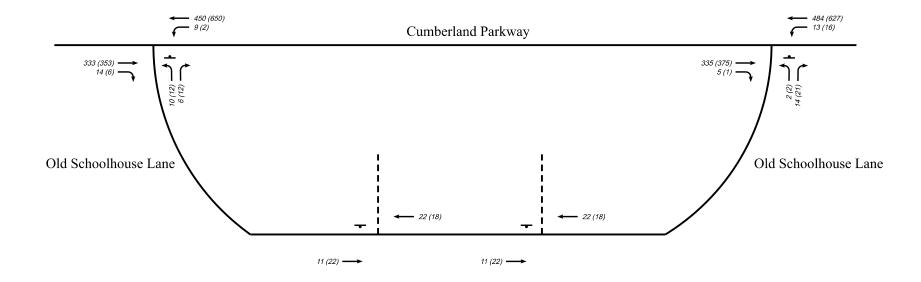




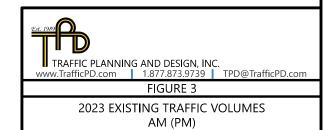


SITE PLAN

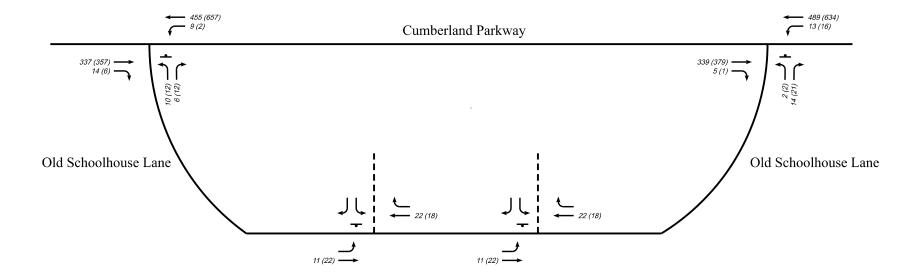


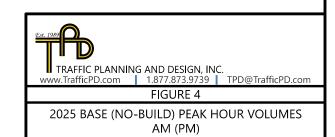




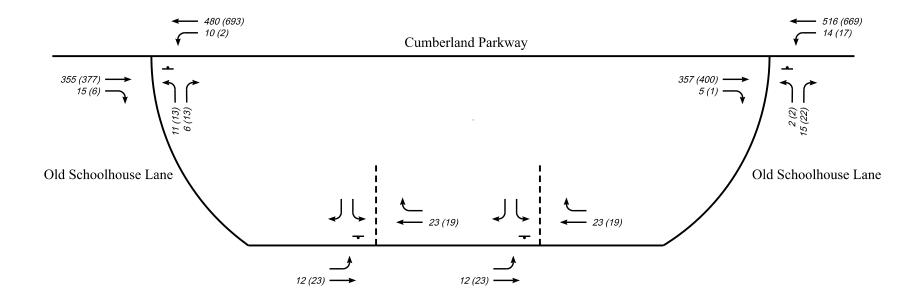








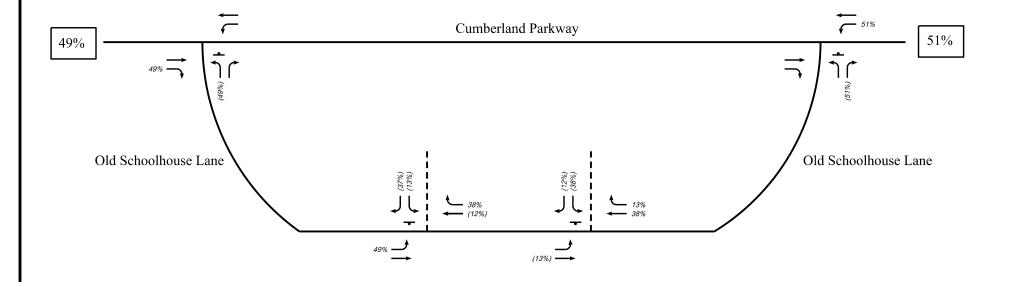






2035 BASE (NO-BUILD) PEAK HOUR VOLUMES AM (PM)





KEY: STOP CONTROLLED - · PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE



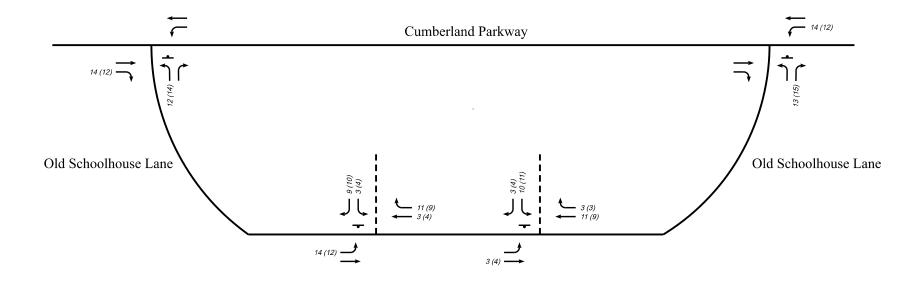
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TPD@TrafficPD.com

FIGURE 6A

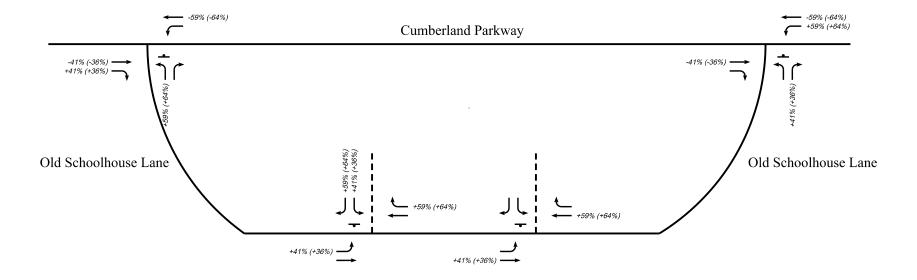
SITE TRIP DISTRIBUTION PERCENTAGES ENTER (EXIT)

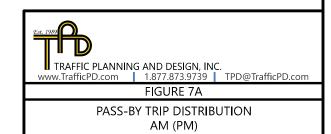




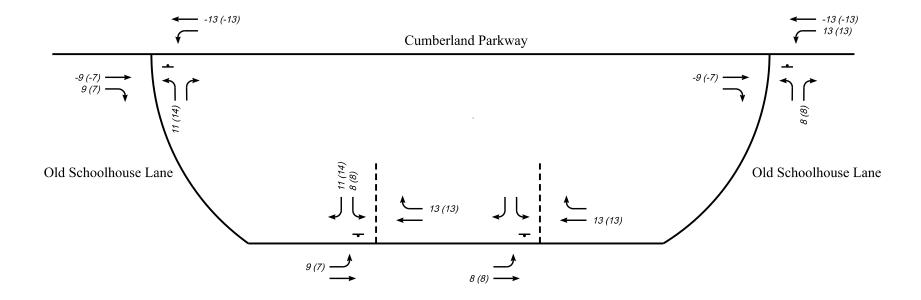


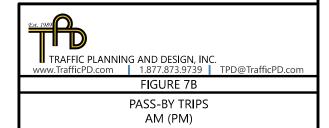




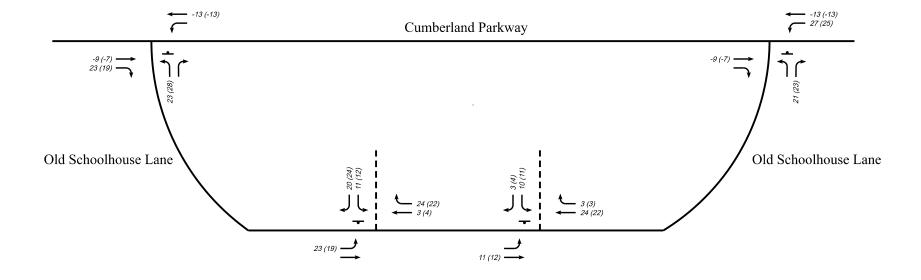








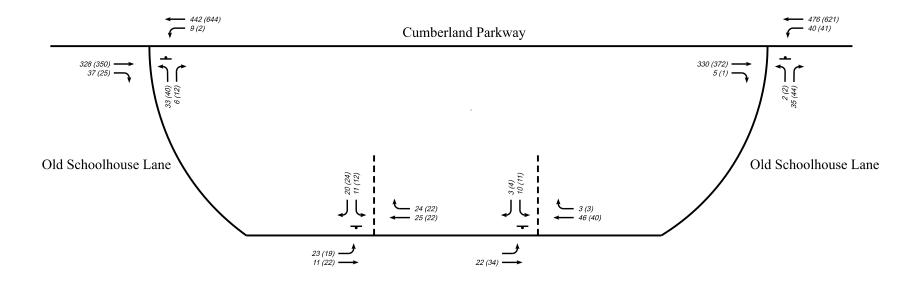


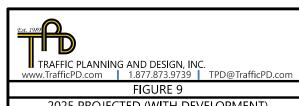




TOTAL PEAK HOUR SITE GENERATED TRIPS AM (PM)

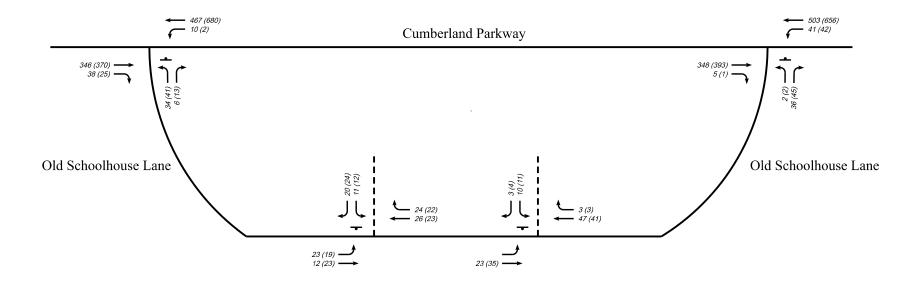


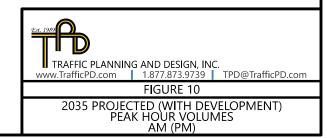




2025 PROJECTED (WITH DEVELOPMENT) PEAK HOUR VOLUMES AM (PM)







# **Appendix A:**Project Correspondence



#### WWW.TRAFFICPD.COM

November 17, 2023

Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

Attention: Ms. Jennifer Boyer, AICP

Re: Response to Upper Allen Township TIS Scope Review

**Genius Kids Child Development Center**Upper Allen Township, Cumberland County, PA
TPD# BLC.00127

Jennifer,

On behalf of the Applicant, Traffic Planning and Design, Inc. (TPD) is submitting the Transportation Impact Study (TIS) package for the proposed Genius Kids Child Development Center in Upper Allen Township, Cumberland County, PA.

#### Response to Upper Allen Township's TIS Scope Submission Review #1

For the discussion below, Transportation Resource Group, Inc. (TRG) comments from the 10/06/2023 TIS scope review letter are shown in *italics*, with the corresponding TPD responses in **bold** type.

1. The trip generation for the proposed 8,500sf Day was calculated using ITE Land Use Code 565 (Day Care Center) and is acceptable for use in the study. According to ITE. The proposed Development is estimated to generate 94 total AM peak hour and 95 total PM peak hour trips. Table 2 shows the total trip generation for the entire site.

#### Noted.

2. The use of pass-by trips for the PM (44%) for the proposed Day Care is consistent with ITE and is acceptable for use in the study. Although ITE does not have data for the AM peak hour, the use of a 44% pass-by for the AM peak hour is also acceptable for use in the study.

#### Noted.

- 3. We concur with the following proposed study intersections to be included in the TIS.
  - Old Schoolhouse Lane/Proposed Site Driveway

The Study area should include the intersections.

- Old Schoolhouse Lane / Cumberland Parkway (East)
- Old Schoolhouse Lane / Cumberland Parkway (West)

#### Noted. The study evaluates all intersections identified above.

- 4. We concur with study time periods:
  - Weekday AM from 6:00 AM to 9:00 AM
  - Weekday PM from 3:00 PM to 6:00 PM

#### Noted.

5. We concur with the 0.54% yearly growth rate for use in the study, consistent with PennDOT's current Growth Factors.

#### Noted.

6. There are no planned developments in the area to be included in the study.

#### Noted.

7. We concur with the proposed trip distribution methodology for the use in the study.

#### Noted.

8. We concur with the use of HCM 6 Methodology for capacity analysis in the study.

#### Noted.

9. We concur with analysis years of existing 2023 and 2025 with and without development. In accordance with the Upper Allen Township SALDO Section 220-3.7.F(3)(c)[2], future project should consider a tenyear growth period. The study should also include a 2035 analysis year.

#### Noted. A 2025 opening year and a 2035 horizon year were evaluated as part of the study.

10. We concur with the auxiliary turn lane warrant analysis according to PennDOT's Publication 46.

#### Noted.

If you have any questions or require additional information to process this application, please call anytime.

#### Sincerely,

TRAFFIC PLANNING AND DESIGN, INC.

Jarred L. Neal, P.E.
Senior Project Manager
JNeal@Trafficpd.com

From: Jennifer Boyer <jboyer@uatwp.org>
Sent: Friday, October 6, 2023 8:03 AM
To: Neal, Jarred; Wheeler, Jason

Cc: Holtzman, Greg

**Subject:** RE: Genius Kids (Upper Allen) - TIS Scope Memo **Attachments:** Genuis Kids Scope Review (10-6-2023).pdf

Jarred, Jason –

Attached is a copy of the letter we received from our traffic engineer regarding your TIS scope.

Jen

Jennifer M. Boyer, AICP Community Development Director Upper Allen Township 717.766.0756 www.uatwp.org

From: Neal, Jarred < jneal@trafficpd.com> Sent: Thursday, October 5, 2023 4:03 PM

To: Jennifer Boyer < jboyer@uatwp.org>; Wheeler, Jason < JWheeler@trafficpd.com>; 'cschwab@consulttrg.com'

<cschwab@consulttrg.com>

Cc: Holtzman, Greg <gholtzman@Blcompanies.com>
Subject: RE: Genius Kids (Upper Allen) - TIS Scope Memo

Will do. Thank you Jen.

Jarred Neal, P.E., Senior Project Manager



#### **OFFICES SERVING THE EASTERN UNITED STATES**

From: Jennifer Boyer < <u>jboyer@uatwp.org</u>>
Sent: Thursday, October 5, 2023 4:02 PM

To: Wheeler, Jason < <a href="mailto:JWheeler@trafficpd.com">JWheeler@trafficpd.com</a>; 'cschwab@consulttrg.com' < <a href="mailto:cschwab@consulttrg.com">cschwab@consulttrg.com</a>>

Cc: Neal, Jarred <ineal@trafficpd.com>; Holtzman, Greg <gholtzman@Blcompanies.com>

Subject: RE: Genius Kids (Upper Allen) - TIS Scope Memo

**CAUTION:** External email - do not click links or open attachments unless you recognize the sender and know the content is safe.

#### Jason -

Please have the scope include both intersections of Cumberland Parkway and Old Schoolhouse Lane since all vehicles entering/exiting the daycare must come out onto Cumberland Parkway via one of the two intersections.

Thank you Jen

Jennifer M. Boyer, AICP
Community Development Director
Upper Allen Township
717.766.0756
www.uatwp.org

From: Wheeler, Jason < JWheeler@trafficpd.com > Sent: Tuesday, September 26, 2023 11:58 AM

**To:** Jennifer Boyer < <u>iboyer@uatwp.org</u>>; 'cschwab@consulttrg.com' < <u>cschwab@consulttrg.com</u>>

Cc: Neal, Jarred <<u>ineal@trafficpd.com</u>>; Holtzman, Greg <<u>gholtzman@Blcompanies.com</u>>

Subject: Genius Kids (Upper Allen) - TIS Scope Memo

Jen/Chris,

TPD is pleased to submit the Transportation Impact Study (TIS) Scoping Memo for the proposed Genius Kids Child Development Center to be located along Old Schoolhouse Lane in Upper Allen Township, Cumberland County. Please review the attached at your convenience.

Thank you, Jason

Jason Wheeler, PTP, Project Manager



#### **OFFICES SERVING THE EASTERN UNITED STATES**

0: 717.234.1430 | D: 717.461.8424 | www.TrafficPD.com | f in 7 0 0 0



PA Office 2 East Market Street Suite 2 York, PA 17401-1206 T: (717) 846-4660

Consulting Engineers and Planners www.consulttrg.com

MD Office 901 Dulaney Valley Road Suite 805 Towson, MD 21204-2624 T: (443) 275-2344

October 6, 2023

Upper Allen Township Attn: Jennifer Boyer, AICP Community Development Director/Planner 100 Gettysburg Pike Mechanicsburg, PA 17055

RE: Genuis Kids Child Development Center Review of Proposed Traffic Impact Study Scope Upper Allen Township, Cumberland County TRG Project No. 517.022.23

Dear Ms. Boyer:

Transportation Resource Group, Inc. has completed the review of the TIS Scope Determination for the proposed Genuis Kids Child Development Center located at 176-198 Old Schoolhouse Ln, Mechanicsburg, PA. The submission dated September 26, 2023, was completed by Traffic Planning and Design, Inc. Based on the review, we offer the following comments:

- 1. The trip generation for the proposed 8,500sf Day was calculated using ITE Land Use Code 565 (Day Care Center) and is acceptable for use in the study. According to ITE, the proposed Development is estimated to generate 94 total AM peak hour and 95 total PM peak hour trips. Table 2 shows the total trip generation for the entire site.
- 2. The use of pass-by trips for the PM (44%) for the proposed Day Care is consistent with ITE and is acceptable for use in the study. Although ITE does not have data for the AM peak hour, the use of a 44% pass-by for the AM peak hour is also acceptable for use in the study.
- 3. We concur with the following proposed study intersections to be included in the TIS.
  - Old Schoolhouse Lane / Proposed Site Driveway

The Study area should include the intersections.

- Old Schoolhouse Lane / Cumberland Parkway (East)
- Old Schoolhouse Lane / Cumberland Parkway (West)

- 4. We concur with study time periods:
  - Weekday AM from 6:00 AM to 9:00 AM
  - Weekday PM from 3:00 PM to 6:00 PM
- 5. We concur with the 0.54% yearly growth rate for use in the study, consistent with PennDOT's current Growth Factors.
- 6. There are no planned developments in the area to be included in the study.
- 7. We concur with the proposed trip distribution methodology for the use in the study.
- 8. We concur with the use of HCM 6 Methodology for capacity analysis in the study.
- 9. We concur with analysis years of existing 2023 and 2025 with and without development. In accordance with the Upper Allen Township SALDO Section 220-3.7.F(3)(c)[2], future project should consider a ten-year growth period. The study should also include a 2035 analysis year.
- 10. We concur with the auxiliary turn lane warrant analysis according to PennDOT's Publication 46.

If you have any questions regarding the above review comments, please feel free to give me a call.

Very truly yours,

Transportation Resource Group, Inc.

Christopher E. Schwab, P.E.

Christopher & School

Senior Associate

From: Wheeler, Jason

Sent: Tuesday, September 26, 2023 11:58 AM

**To:** AICP Jennifer Boyer (jboyer@uatwp.org); 'cschwab@consulttrg.com'

Cc: Neal, Jarred; Holtzman, Greg

**Subject:** Genius Kids (Upper Allen) - TIS Scope Memo

Attachments: 2023-09-26 (Genuis Kids - Upper Allen) TIS Scope Memo\_attach.pdf

Jen/Chris,

TPD is pleased to submit the Transportation Impact Study (TIS) Scoping Memo for the proposed Genius Kids Child Development Center to be located along Old Schoolhouse Lane in Upper Allen Township, Cumberland County. Please review the attached at your convenience.

Thank you, Jason

Jason Wheeler, PTP, Project Manager



### **OFFICES SERVING THE EASTERN UNITED STATES**

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### **MEMORANDUM**

To: Jennifer Boyer – Upper Allen Township (Community Development Director/Planner)

From: Jarred L. Neal, P.E., *Traffic Planning and Design, Inc. (TPD)* 

Cc: Greg Holtzman – BL Companies (Project Manager)

Chris Schwab, P.E. – TRG, Inc. (Township Traffic Engineer)

Date: September 26, 2023

Re: Transportation Impact Study (TIS) Scope Determination Request

Genius Kids – Upper Allen

Upper Allen Township, Cumberland County, PA

TPD# BLC.00127

This memo regards a proposed Genius Kids Child Development Center within Upper Allen Township, Cumberland County, PA. The proposed development is located on the north side Old Schoolhouse Lane southwest of the Cumberland Parkway/Old Schoolhouse Lane intersection (easternmost). The project involves construction of an 8,500 s.f. Child Development Center (Infant to Pre-K). Access for the development is proposed via two (2) driveway connections to Old Schoolhouse Lane, ultimately providing connections to Cumberland Parkway.

Prior to preparing the TIS, TPD is requesting the Township's feedback on the scope of the TIS. The following scope parameters are proposed:

### **TIS Scope**

### Study Area:

» Old Schoolhouse Lane and Proposed Site Driveway intersections.

### Study Time Periods:

- » Weekday AM Peak Hour (6:00-9:00 A.M.);
- » Weekday PM Peak Hour (3:00-6:00 P.M.).

### Background Growth Factor & Nearby Developments:

A background growth factor of 0.54% compounded annually will be applied to the existing traffic volumes in accordance with the current PennDOT BPR statistics.

TPD requests that Upper Allen Township, identify any nearby planned developments that will need to be included in this study.

### *Trip Generation:*

The trip generation rates for the proposed development were obtained from the manual *Trip Generation*, Eleventh Edition, 2021, an Institute of Transportation Engineers (ITE) Informational Report. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

The data for Land Use Code 565 (Day Care Center) was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour.

The following should be noted with respect to the trip generation methodology:

- » Based on the information provided in the manual *Trip Generation*, not all of the trips generated by the site will be "new" to the nearby roadway system. In addition to the "new trips" generated by the development, there will be "pass-by trips", which are trips that are drawn from the passing traffic stream and do not add trips to the adjacent roadways. Pass-by trip percentages were utilized as specified in the appendices of the manual *Trip Generation*, Eleventh Edition, 2021.
- Published data related to pass-by percentages are not available for Land Use Code #565 (Day Care Center) during the weekday A.M. peak hour. However, given the trip characteristics of a typical Day Care Center with parent drop-off/pick-up to and from there place of employment, etc. it is reasonable to assume that pass-by trips will occur during the weekday A.M. peak hour as well. Since ITE provides a weekday P.M. pass-by rate of 44% it is reasonable to assume that pass-by trips occur at the same rate during the weekday A.M. peak hour.

**Table 1** shows the trip generation rates and directional split for the analyzed time periods.

TABLE 1
ITE TRIP GENERATION EQUATIONS

Land Use	ITE#	Time Period	Equations/Rates	Entering %	Exiting %	Pass-By %
	' I 565 I Weekday A M Peak Hour I	T = 47.62*(X)	50%	50%		
Day Care Center		Weekday A.M. Peak Hour	T = 11.00*(X)	53%	47%	44% <sup>1</sup>
Certici		Weekday P.M. Peak Hour	T = 11.12*(X)	47%	53%	44%

T = number of site-generated vehicular trips
 X = independent variable (ksf of Gross Floor Area)
 1 = ITE pass-by rates not published, see TPD justification above

The calculated trip generation for the proposed development is shown in **Table 2**.

## TABLE 2 TRIP GENERATION SUMMARY

Time Period	1	Total Trip	S	Pa	ass-by Tri <sub>l</sub>	os	New Trips			
Time Period	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	
Based on 8,500 s.f. of Gross Floor Area										
Average Weekday	405	202	203							
Weekday A.M. Peak Hour	94	50	44	41	22	19	53	31	25	
Weekday P.M. Peak Hour	95	44	51	42	20	22	53	24	29	

### *Trip Distribution:*

The distribution and assignment of new trips generated by the development will be based upon the following: (1) existing traffic patterns in the study area; (2) the most logical route of travel; (3) site driveway locations/configurations.

### Capacity Analyses:

Capacity analyses will be conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the *Highway Capacity Manual*, 6<sup>th</sup> Ed. (HCM) using *Synchro 11* software, a Trafficware product:

- » 2023 Existing Conditions;
- » 2025 Base Conditions (opening year without the proposed development);
- » 2025 Projected Conditions (opening year with full build-out of the proposed development).

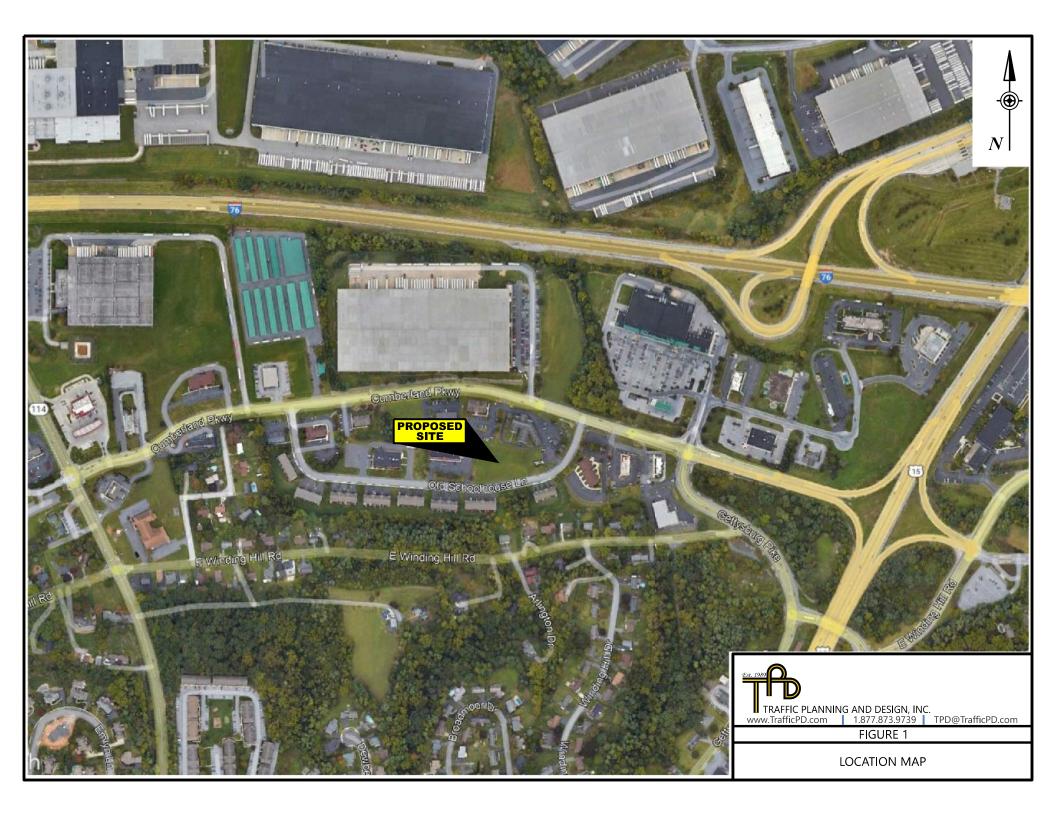
### **Auxiliary Turn Lane Warrants:**

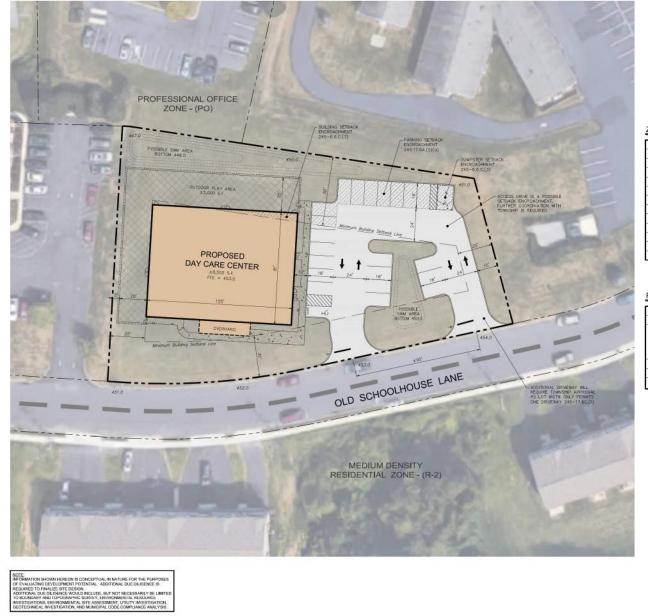
TPD will evaluate auxiliary turn lane warrants on Old Schoolhouse Road approaching the Site Driveway intersections. The warrant analysis was conducted according to the methodologies contained in Chapter 11 of PennDOT's *Publication 46* utilizing the posted speed limit.

We would appreciate your review of the enclosed information. If there are any questions or comments, please call any time.

### Attachments:

Location Map Site Plan





#### ZONING INFORMATION

ZONE:	PROFESSIONAL OFFICE (PO)			
	DAY CARE, ACCESSORY (USE 68) (PE	RMITTED USE)		
ITTOM #	mbi	REQUIREMENTS	PROPOSED	VARIANCE
+	MINIMUM LOT AREA	NONE REQUIRED.	±1.06 ACRES	NO
2	MINIMUM LOT WOTH	150 FEET	1295 PEET	NO
3	MINIMUM LOT DEPTH	NONE REQUIRED	±150 FEET	NO
4	MINIMUM PRONT SETBACK	30 FEET	S30 FEET	NO
5	MINIMUM SIDE SETBACK	25 FEET	<25 FEET	×s.
	MINIMUM REAR SETBACK	50 FEET	<50 FEET	YES
7	MAXMUM BUILDING HEIGHT	35 FEET	<35 FEET	NO
80	WAXIMUW BUILDING COVERAGE	30 PERCENT	#20 PERCENT	NO.
9	MAXIMUM LOT COVERAGE	60 PERCENT	±50 PERCENT	NO:

#### PARKING INFORMATION

ITEM #	mov	REQUIREMENTS	PROPOSED	VARIANCE	
1	BULDING SEE	NONE REQUIRED	8,500 S.F.		
2	PARKING REQUIRED BY TOWNSHIP	1 SPACE FOR EVERY EMPLOYEE ON THE LANCEST SHIFT. + 1 SPACE: FOR EVERY 6 STADENTS, AND DIY-STREET WATING SPACES TO ACCOMMEDIATE AT LAST SIX AUTO, BEAM. + 8 (ASSIAN: 48 STADENTS) + 6 WATING = 22 SPACES	22 SPACES	NO	
30	MINIMUM PARKING DIMENSIONS	9.5 FEET X 18 FEET	9.5 FEET X 18 FEET	NO.	
•	MINIMUM AISLE WOTH	24 FEET	24 FEET	NO	
50.	PARKING SETBACK	PARKING IS PROHEITED IN THE MINIMUM BUILDING SETBACKS	PARKING IS SHOWN WITHIN SIDE AND REAR SETBACKS	YES	



PROPOSED DEVELOPMENT
OLD SCHOOL HOUSE LANE
UPPER ALLEN TOWNSHIP, CUMBERLAND COUNTY,

0.JH A.JS. T = 33 23-0000X 09070222

CONCEPT

2601 Martet Place Salte 250 Harristang, PA 17110 (717) 943-1665 (717) 051-9658 Fax

PA

TRAFFIC PLANNING AND DESIGN, INC.

www.TrafficPD.com | 1.877.873.9739 | TPD@TrafficPD.com

FIGURE 2

SITE PLAN

# **Appendix B:** Existing Roadway Conditions



Direction / Road: NB Old Schoolhouse Lane (West)

Approach / Departure: Approach

50 feet Distance:



**Direction / Road:** NB Old Schoolhouse Lane (West)

Approach / Departure: Approach

> 200 Feet Distance:



Direction / Road: EB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



Direction / Road:EB Cumberland Pkwy

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

Distance: 50 feet



**Direction / Road:** WB Cumberland Pkwy

Approach / Departure: Approach

> 200 Feet Distance:

## **INTERSECTION WORKSHEET**

Traffic Planning and Design, Inc.

TPD Project #	
Date	
Analyst	

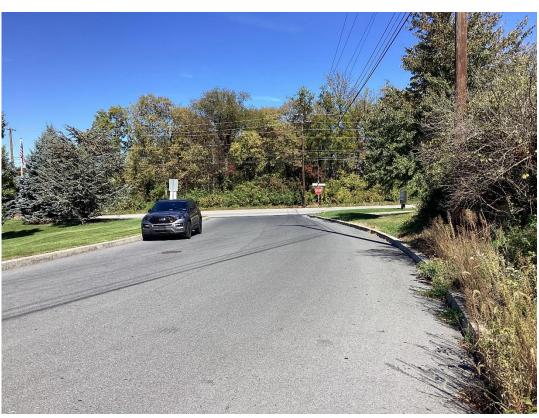
Osignalized O	Un-signalized OTwo-V	Way Stop Control O	All-Way Stop Control	Ooffset Oother
Area Type: Urb	an Suburban Rural	CBD		
Streets: (N-S)		(E-W	()	
	Westbound	Eastbound		Northbound
	L T R	L T R	L T R	L T R
No. Lanes	9 1	<b>₽</b>		$\longleftrightarrow$
Lane Width	1			
Storage Length				
Grade (approaching intersection) + uphill, -downhill				
Channelized Right?				
If so, is lane >				
75'?				
Shoulder width Pavement				
condition*				
Lane marking condition*				
Posted speed limit				
Driveways on				
approach?	D #	D	D #	D #
Bus Stops?	Route #:	Route #:	Route #:	Route #:
Parking? Pedestrian Curb				
Ramps?	/	/	/	/
Sidewalk?	/	/	/	/
Crosswalks?				
	U	nsignalized Intersecti	ons:	
Sign Control		<b></b>		
Sight Distance*				
		70 10 17 1		
No Turn on Red		Signalized Intersectio	ns:	
posted?				
Ped Button?				
Left Turn Phase	7		/	
Actuated Lanes				//
Comments (please be	as specific as possible):	CONTRACTOR		
	_	<u> </u>		
<b>\</b>	- ~ <del>\</del>		ا ار	)  (
LTR	LTR	L 1	. T R 1<0	L T R 1 2 1
Actuated Lanes	as specific as possible):		T R 1<0	LTR 121



Direction / Road: NB Old Schoolhouse Lane (East)

Approach / Departure: Approach

50 feet Distance:



Direction / Road: NB Old Schoolhouse Lane (East)

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: EB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



**Direction / Road:** EB Cumberland Pkwy

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



**Direction / Road:** WB Cumberland Pkwy

Approach / Departure: Approach

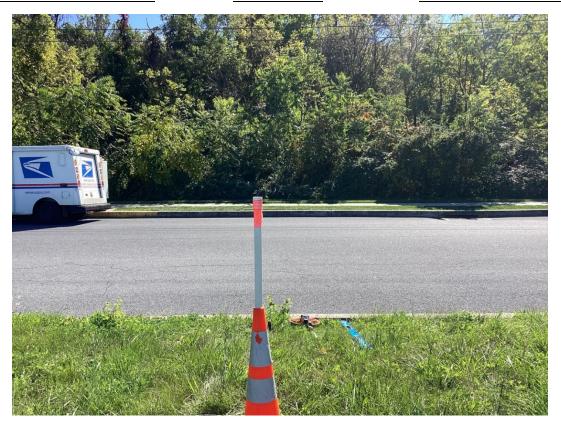
200 Feet Distance:

## **INTERSECTION WORKSHEET**

Traffic Planning and Design, Inc.

TPD Project # _	
Date	
Analyst	

33	8	,		
Osignalized O	Un-signalized OTwo	o-Way Stop Control O	All-Way Stop Co	ontrol Ooffset Oother
Area Type: Urba	an Suburban Rural	CBD		
Streets: (N-S)		(E-W	()	
	Westbound	Eastbound		Northbound
	L T R	L T R	L T	R L T R
No. Lanes			L I	R L I K
	9			
Lane Width				
Storage Length				
Grade (approaching intersection) + uphill, -downhill				
Channelized Right?				
If so, is lane > 75'?				
Shoulder width				
Pavement				
condition*				
Lane marking condition*				
Posted speed limit				
Driveways on				
approach?				
Bus Stops?	Route #:	Route #:	Route #	#: Route #:
Parking?				
Pedestrian Curb Ramps?	/	/	/	/
Sidewalk?	/	/	/	/
Crosswalks?				
		<b>Unsignalized Intersecti</b>	ons:	
Sign Control				
Sight Distance*				
		Signalized Intersection	ns:	
No Turn on Red				
posted?		And the state of t		
Ped Button?				
Left Turn Phase			/	
Actuated Lanes				and the second second
Comments (please be	as specific as possible):			
1 1				
	- -	<b>&gt;</b>	4//	
	LTR	 L 1	T R 1<0	'
L T R 0>1<0	0>0<0			



Approach / Departure:

**Direction / Road:** Center Driveway - Looking Out

Distance:



Approach / Departure:

**Direction / Road:** Center Driveway – Looking In

Distance:



Approach / Departure:

**Direction / Road:** Center Driveway – Looking Right

Distance:



**Direction / Road:** Approach / Departure: Center Driveway – Looking Left

Distance:

## **DRIVEWAY SIGHT DISTANCE MEASUREMENTS**

(FOR LOCAL ROADS, USE PENNDOT PUB 70)

APPLICANT_			APPLICATION NO	
S.R	SEG	OFFSET	LEGAL SPEED LIMI	Γ
MEASURED B	BY		DATE	
FOR DEPART	MENT USE ONLY: S	afe-Running Speed	85th Percentile Speed _	
Λ				
A			,	je.
		<b>«···</b> ,	GRADE%	0']
	3.501	GRADE%	Sign Line	
	CE REQUIRED	Sight Line	DRIVER'S EYE 10'  DISTANCE REQUIRED  FSD=	EDGE OF TRAVEL LANE
P			/HICH A DRIVER AT A DRIVEWAY LOC LE APPROACHING ON THE ROADWA	
			<b>←</b>	
			GRADE%  3.50' Sight Line	
=======				
		Ť (	DISTANCE REQUIRED FSD=	
col	NTINUOUSLY SEE THE	REAR OF A VEHICLE WHIC	WHICH A DRIVER ON THE ROADWAY CH IS LOCATED IN THE DRIVER'S TRA A LEFT TURN INTO A DRIVEWAY.	~
С				
		Sight Line	3.50	
3.50'	GRADE	======================================	<u> </u>	=========
		÷	DISTANCE REC	

THE MAXIMUM LENGTH OF ROADWAY ALONG WHICH A DRIVER OF A VEHICLE INTENDING TO MAKE A LEFT TURN INTO A DRIVEWAY CAN CONTINUOUSLY SEE A VEHICLE APPROACHING FROM THE OPPOSITE DIRECTION.

## **DRIVEWAY SIGHT DISTANCE MEASUREMENTS**

(FOR LOCAL ROADS, USE PENNDOT PUB 70)

		· ·		
APPLICANT_			APPLICATION NO	
S.R	SEG	OFFSET	LEGAL SPEED LIMI	Γ
MEASURED B	BY		DATE	
FOR DEPART	MENT USE ONLY: S	afe-Running Speed	85th Percentile Speed _	
Λ				
A			,	je.
		<b>«···</b> ,	GRADE%	0']
	3.501	GRADE%	Sign Line	
	CE REQUIRED	Sight Line	DRIVER'S EYE 10'  DISTANCE REQUIRED  FSD=	EDGE OF TRAVEL LANE
P			/HICH A DRIVER AT A DRIVEWAY LOC LE APPROACHING ON THE ROADWA	
			<b>←</b>	
			GRADE%  3.50' Sight Line	
=======				
		Ť (	DISTANCE REQUIRED FSD=	
col	NTINUOUSLY SEE THE	REAR OF A VEHICLE WHIC	WHICH A DRIVER ON THE ROADWAY CH IS LOCATED IN THE DRIVER'S TRA A LEFT TURN INTO A DRIVEWAY.	~
С				
		Sight Line	3.50	
3.50'	GRADE	======================================	<u> </u>	=========
		÷	DISTANCE REC	

THE MAXIMUM LENGTH OF ROADWAY ALONG WHICH A DRIVER OF A VEHICLE INTENDING TO MAKE A LEFT TURN INTO A DRIVEWAY CAN CONTINUOUSLY SEE A VEHICLE APPROACHING FROM THE OPPOSITE DIRECTION.

# **Appendix C:**Manual Turning Movement Counts



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 1

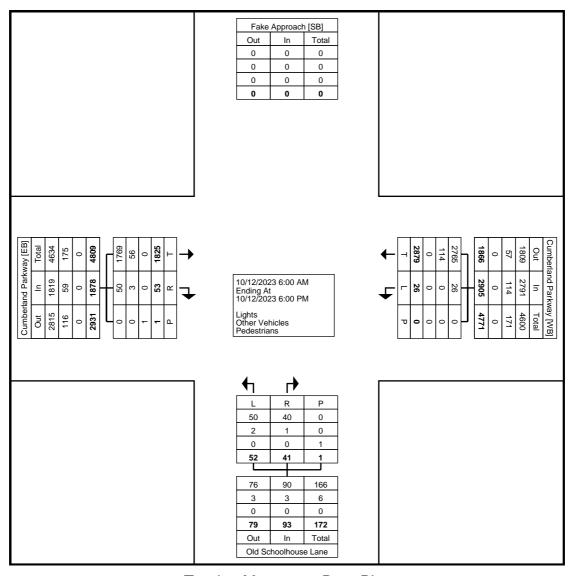
**Turning Movement Data** 

		Cumberlan	d Parkway	Ī	3	VIOVEM Cumberlar	nd Parkway	Î	0				
		Eastb	•				bound			Northbound			
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
6:00 AM	37	0	0	37	1	44	0	45	0	0	0	0	82
6:15 AM	42	1	0	43	0	35	0	35	1	0	0	1	79
6:30 AM	49	1	0	50	1	59	0	60	3	0	0	3	113
6:45 AM	65	1	0	66	1	77	0	78	0	0	0	0	144
Hourly Total	193	3	0	196	3	215	0	218	4	0	0	4	418
7:00 AM	56	4	0	60	1	80	0	81	0	2	0	2	143
7:15 AM	85	5	0	90	4	105	0	109	3	1	0	4	203
7:30 AM	81	6	0	87	0	120	0	120	1	1	0	2	209
7:45 AM	103	7	0	110	4	117	0	121	4	2	0	6	237
Hourly Total	325	22	0	347	9	422	0	431	8	6	0	14	792
8:00 AM	60	0	0	60	4	102	0	106	3	1	0	4	170
8:15 AM	89	1	0	90	1	111	0	112	2	2	0	4	206
8:30 AM	80	1	0	81	0	91	0	91	1	1	0	2	174
8:45 AM	67	4	0	71	3	105	0	108	1	1	0	2	181
Hourly Total	296	6	0	302	8	409	0	417	7	5	0	12	731
*** BREAK ***	-		-	-	-		-	_	-		-	-	-
3:00 PM	91	3	0	94	1	143	0	144	1	4	0	5	243
3:15 PM	85	0	0	85	0	128	0	128	1	4	0	5	218
3:30 PM	76	3	0	79	1	133	0	134	7	5	0	12	225
3:45 PM	92	2	0	94	0	141	0	141	1	2	0	3	238
Hourly Total	344	8	0	352	2	545	0	547	10	15	0	25	924
4:00 PM	85	0	0	85	0	155	0	155	3	0	0	3	243
4:15 PM	79	0	1	79	1	163	0	164	1	2	. 1	3	246
4:30 PM	95	3	0	98	0	163	0	163	3	2	0	5	266
4:45 PM	90	1	0	91	0	178	0	178	3	5	0	8	277
Hourly Total	349	4	1	353	1	659	0	660	10	9	1	19	1032
5:00 PM	89	2	0	91	1	146	0	147	5	3	0	8	246
5:15 PM	74	1	0	75	2	155	0	157	3	1	0	4	236
5:30 PM	80	3	0	83	0	177	0	177	0	2	0	2	262
5:45 PM	75	4	0	79	0	151	0	151	5	0	0	5	235
Hourly Total	318	10	0	328	3	629	0	632	13	6	0	19	979
Grand Total	1825	53	1	1878	26	2879	0	2905	52	41	1	93	4876
Approach %	97.2	2.8	-	-	0.9	99.1	-	-	55.9	44.1	-	-	-
Total %	37.4	1.1	-	38.5	0.5	59.0	-	59.6	1.1	0.8	-	1.9	-
Lights	1769	50	-	1819	26	2765	-	2791	50	40	-	90	4700
% Lights	96.9	94.3	-	96.9	100.0	96.0	_	96.1	96.2	97.6	-	96.8	96.4
Other Vehicles	56	3	-	59	0	114	-	114	2	1	-	3	176
% Other Vehicles	3.1	5.7	-	3.1	0.0	4.0	-	3.9	3.8	2.4	-	3.2	3.6
Pedestrians	-	-	1	-	-		0	-	-	-	1	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	100.0	-	-



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 2



**Turning Movement Data Plot** 



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 3

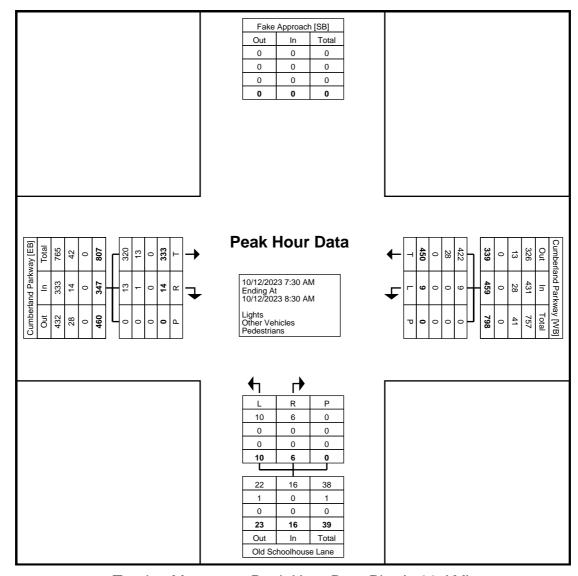
Turning Movement Peak Hour Data (7:30 AM)

			. •						,				
		Cumberlan	d Parkway	_		Cumberlan	d Parkway	,	Old Schoolhouse Lane (West)				
Ot and Time a	Eastbound					Westbound			Northbound				
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
7:30 AM	81	6	0	87	0	120	0	120	1	1	0	2	209
7:45 AM	103	7	0	110	4	117	0	121	4	2	0	6	237
8:00 AM	60	0	0	60	4	102	0	106	3	1	0	4	170
8:15 AM	89	1	0	90	1	111	0	112	2	2	0	4	206
Total	333	14	0	347	9	450	0	459	10	6	0	16	822
Approach %	96.0	4.0	-	-	2.0	98.0	-	-	62.5	37.5	-	-	-
Total %	40.5	1.7	-	42.2	1.1	54.7	-	55.8	1.2	0.7	-	1.9	-
PHF	0.808	0.500	-	0.789	0.563	0.938	-	0.948	0.625	0.750	-	0.667	0.867
Lights	320	13	-	333	9	422	-	431	10	6	-	16	780
% Lights	96.1	92.9	-	96.0	100.0	93.8	-	93.9	100.0	100.0	-	100.0	94.9
Other Vehicles	13	1	-	14	0	28	-	28	0	0	-	0	42
% Other Vehicles	3.9	7.1	-	4.0	0.0	6.2	-	6.1	0.0	0.0	-	0.0	5.1
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	_		_	_			_	_	_				_



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 5

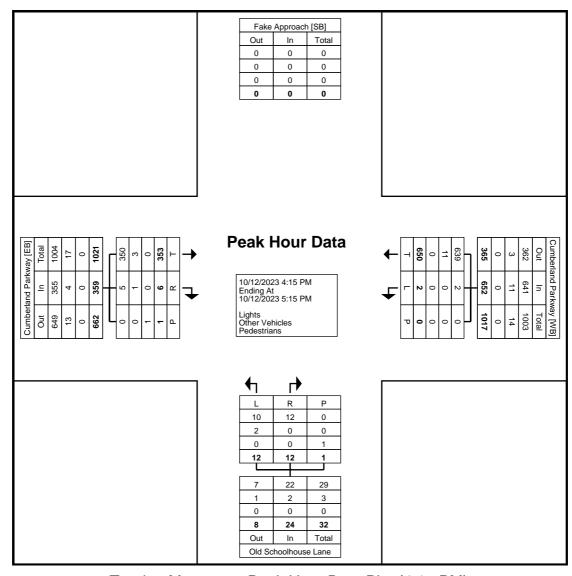
Turning Movement Peak Hour Data (4:15 PM)

				· .				١ ،	,				1
		Cumberlar	nd Parkway			Cumberlan	d Parkway		0				
Start Time		Easth	oound			Westh	oound		Northbound				
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
4:15 PM	79	0	1	79	1	163	0	164	1	2	1	3	246
4:30 PM	95	3	0	98	0	163	0	163	3	2	0	5	266
4:45 PM	90	1	0	91	0	178	0	178	3	5	0	8	277
5:00 PM	89	2	0	91	1	146	0	147	5	3	0	8	246
Total	353	6	1	359	2	650	0	652	12	12	1	24	1035
Approach %	98.3	1.7	-	-	0.3	99.7	-	-	50.0	50.0	-	-	-
Total %	34.1	0.6	-	34.7	0.2	62.8	-	63.0	1.2	1.2	-	2.3	-
PHF	0.929	0.500	-	0.916	0.500	0.913	-	0.916	0.600	0.600	-	0.750	0.934
Lights	350	5	-	355	2	639	-	641	10	12	-	22	1018
% Lights	99.2	83.3	-	98.9	100.0	98.3	-	98.3	83.3	100.0	-	91.7	98.4
Other Vehicles	3	1	-	4	0	11	-	11	2	0	-	2	17
% Other Vehicles	0.8	16.7	-	1.1	0.0	1.7	-	1.7	16.7	0.0	-	8.3	1.6
Pedestrians	-	-	1	-	-	-	0	-	-	-	1	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	100.0	-	-



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:15 PM)



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 1

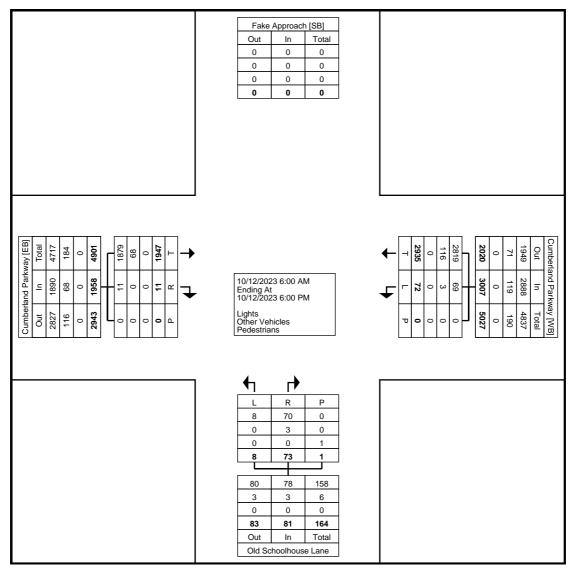
Turning Movement Data

				ΙŲ	irning	Movem	ient D	ata					
		Cumberland	d Parkway			Cumberlan	nd Parkway		0				
Start Time		Eastb			West	bound							
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
6:00 AM	70	0	0	70	1	49	0	50	0	1	0	1	121
6:15 AM	34	0	0	34	1	55	0	56	0	1	0	1	91
6:30 AM	49	0	0	49	4	63	0	67	0	4	0	4	120
6:45 AM	60	0	0	60	0	85	0	85	0	2	0	2	147
Hourly Total	213	0	0	213	6	252	0	258	0	8	0	8	479
7:00 AM	56	1	0	57	3	92	0	95	0	3	0	3	155
7:15 AM	73	0	0	73	4	114	0	118	1	0	0	1	192
7:30 AM	81	1	0	82	3	125	0	128	0	4	0	4	214
7:45 AM	108	0	0	108	6	127	0	133	0	5	0	5	246
Hourly Total	318	2	0	320	16	458	0	474	1	12	0	13	807
8:00 AM	54	2	0	56	3	108	0	111	1	2	0	3	170
8:15 AM	92	2	0	94	1	124	0	125	1	3	0	4	223
8:30 AM	78	2	0	80	6	92	0	98	0	2	0	2	180
8:45 AM	72	0	0	72	3	114	0	117	1	3	0	4	193
Hourly Total	296	6	0	302	13	438	0	451	3	10	0	13	766
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	123	0	0	123	2	138	0	140	0	3	0	3	266
3:15 PM	90	1	0	91	2	137	0	139	0	2	0	2	232
3:30 PM	94	1	0	95	1	127	0	128	1	4	0	5	228
3:45 PM	95	0	0	95	6	139	0	145	0	3	0	3	243
Hourly Total	402	2	0	404	11	541	0	552	1	12	0	13	969
4:00 PM	94	0	0	94	2	136	0	138	0	4	0	4	236
4:15 PM	88	0	0	88	4	148	0	152	0	3	1	3	243
4:30 PM	95	0	0	95	6	163	0	169	1	2	0	3	267
4:45 PM	99	0	0	99	3	168	0	171	1	3	0	4	274
Hourly Total	376	0	0	376	15	615	0	630	2	12	1	14	1020
5:00 PM	99	0	0	99	7	141	0	148	0	9	0	9	256
5:15 PM	82	1	0	83	0	155	0	155	0	7	0	7	245
5:30 PM	82	0	0	82	2	178	0	180	0	2	0	2	264
5:45 PM	79	0	0	79	2	157	0	159	1	1	0	2	240
Hourly Total	342	1	0	343	11	631	0	642	1	19	0	20	1005
Grand Total	1947	11	0	1958	72	2935	0	3007	8	73	1	81	5046
Approach %	99.4	0.6	-	-	2.4	97.6	-	-	9.9	90.1	-	-	-
Total %	38.6	0.2	-	38.8	1.4	58.2	-	59.6	0.2	1.4	-	1.6	-
Lights	1879	11	-	1890	69	2819	-	2888	8	70	-	78	4856
% Lights	96.5	100.0	-	96.5	95.8	96.0	-	96.0	100.0	95.9	-	96.3	96.2
Other Vehicles	68	0	-	68	3	116	-	119	0	3	-	3	190
% Other Vehicles	3.5	0.0	-	3.5	4.2	4.0	-	4.0	0.0	4.1	-	3.7	3.8
Pedestrians	-	-	0	-	-	-	0	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 2



**Turning Movement Data Plot** 



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 3

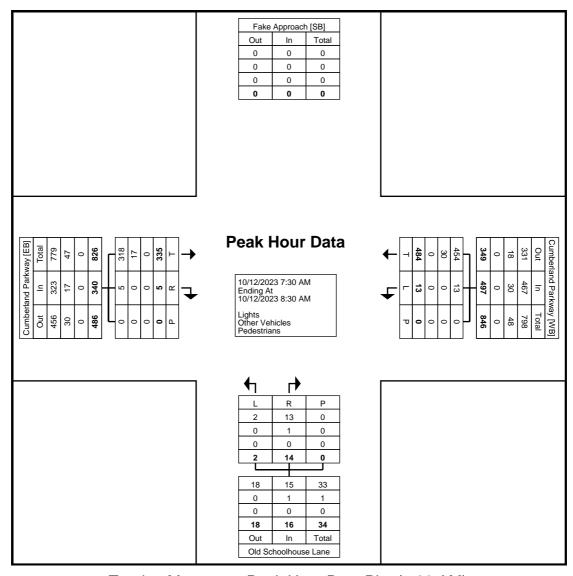
Turning Movement Peak Hour Data (7:30 AM)

								٠, ,					
		Cumberlar	nd Parkway		Cumberland Parkway				C				
Start Time		Easth	oound			West	oound		Northbound				
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
7:30 AM	81	1	0	82	3	125	0	128	0	4	0	4	214
7:45 AM	108	0	0	108	6	127	0	133	0	5	0	5	246
8:00 AM	54	2	0	56	3	108	0	111	1	2	0	3	170
8:15 AM	92	2	0	94	1	124	0	125	1	3	0	4	223
Total	335	5	0	340	13	484	0	497	2	14	0	16	853
Approach %	98.5	1.5	-	-	2.6	97.4	-	-	12.5	87.5	-	-	-
Total %	39.3	0.6	-	39.9	1.5	56.7	-	58.3	0.2	1.6	-	1.9	-
PHF	0.775	0.625	-	0.787	0.542	0.953	-	0.934	0.500	0.700	-	0.800	0.867
Lights	318	5	-	323	13	454	-	467	2	13	-	15	805
% Lights	94.9	100.0	-	95.0	100.0	93.8	-	94.0	100.0	92.9	-	93.8	94.4
Other Vehicles	17	0	-	17	0	30	-	30	0	1	-	1	48
% Other Vehicles	5.1	0.0	-	5.0	0.0	6.2	-	6.0	0.0	7.1	-	6.3	5.6
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	_	-	-	-	-	-	-	-



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 5

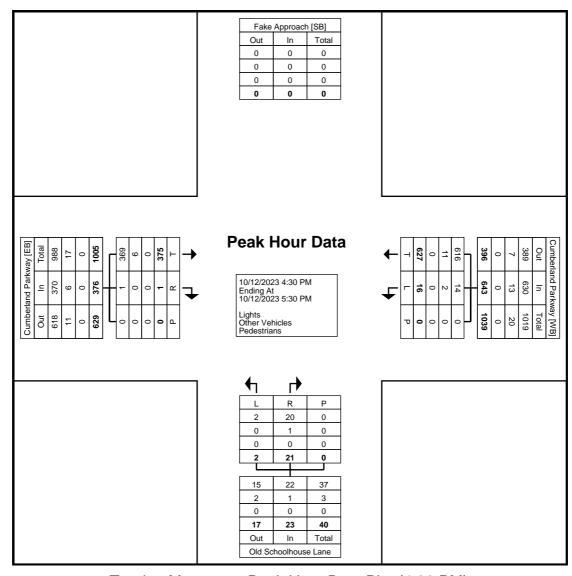
Turning Movement Peak Hour Data (4:30 PM)

				.9		. •		~.~ (					
		Cumberlan	d Parkway			Cumberlan	d Parkway	,	Ó				
		Easth	ound			Westh	ound		Northbound				
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
4:30 PM	95	0	0	95	6	163	0	169	1	2	0	3	267
4:45 PM	99	0	0	99	3	168	0	171	1	3	0	4	274
5:00 PM	99	0	0	99	7	141	0	148	0	9	0	9	256
5:15 PM	82	1	0	83	0	155	0	155	0	7	0	7	245
Total	375	1	0	376	16	627	0	643	2	21	0	23	1042
Approach %	99.7	0.3	-	-	2.5	97.5	-	-	8.7	91.3	-	-	-
Total %	36.0	0.1	-	36.1	1.5	60.2	-	61.7	0.2	2.0	-	2.2	-
PHF	0.947	0.250	-	0.949	0.571	0.933	-	0.940	0.500	0.583	-	0.639	0.951
Lights	369	1	-	370	14	616	-	630	2	20	-	22	1022
% Lights	98.4	100.0	-	98.4	87.5	98.2	-	98.0	100.0	95.2	-	95.7	98.1
Other Vehicles	6	0	-	6	2	11	-	13	0	1	-	1	20
% Other Vehicles	1.6	0.0	-	1.6	12.5	1.8	-	2.0	0.0	4.8	-	4.3	1.9
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	_	_	-	_	-	_	-	_	-	_	-	_	_



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:30 PM)



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 1

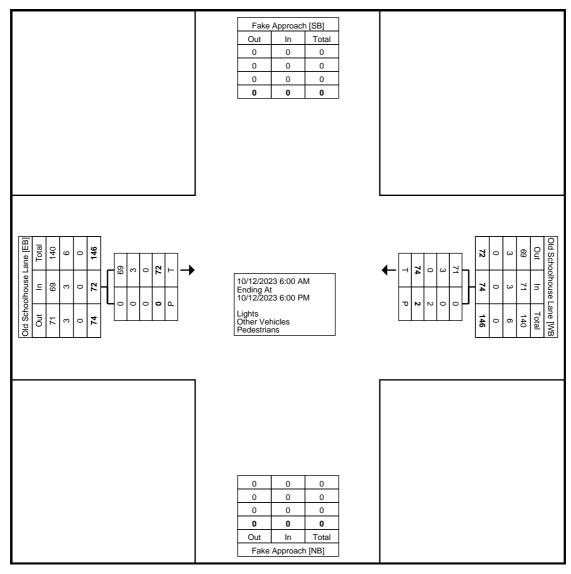
**Turning Movement Data** 

	·	l urnır	ng Movemei	nt Data			i
		Old Schoolhouse La	ne				
Start Time		Eastbound					
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
6:00 AM	1	0	1	1	0	. 1	2
6:15 AM	0	0	0	1	0	1	1
6:30 AM	4	0	4	4	0	4	8
6:45 AM	2	0	2	0	0	0	2
Hourly Total	7	0	7	6	0	6	13
7:00 AM	1	0	1	3	0	3	4
7:15 AM	1	0	1	5	0	5	6
7:30 AM	5	0	5	2	0	2	7
7:45 AM	4	0	4	7	0	7	11
Hourly Total	11	0	11	17	0	17	28
8:00 AM	2	0	2	5	0	5	7
8:15 AM	2	0	2	2	0	2	4
8:30 AM	3	0	3	8	0	. 8	11
8:45 AM	3	0	3	3	0	3	6
Hourly Total	10	0	10	18	0	18	28
*** BREAK ***	-	-	-	-	-	-	-
3:00 PM	3	0	3	2	0	. 2	5
3:15 PM	1	0	1	2	0	2	3
3:30 PM	5	0	5	2	0	2	7
3:45 PM	2	0	2	2	0	2	4
Hourly Total	11	0	11	8	0	8	19
4:00 PM	2	0	2	5	0	5	7
4:15 PM	4	0	4	1	0	1	5
4:30 PM	4	0	4	6	0	6	10
4:45 PM	3	0	3	4	0	4	7
Hourly Total	13	0	13	16	0	16	29
5:00 PM	9	0	9	6	2	6	15
5:15 PM	6	0	6	2	0	2	8
5:30 PM	2	0	2	1	0	1	3
5:45 PM	3	0	3	0	0	0	3
Hourly Total	20	0	20	9	2	9	29
Grand Total	72	0	72	74	2	74	146
Approach %	100.0	-	-	100.0	-	-	-
Total %	49.3	-	49.3	50.7	-	50.7	-
Lights	69	-	69	71	-	71	140
% Lights	95.8	-	95.8	95.9	-	95.9	95.9
Other Vehicles	3	-	3	3	-	3	6
% Other Vehicles	4.2	-	4.2	4.1	-	4.1	4.1
Pedestrians	-	0	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-



## Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 2



**Turning Movement Data Plot** 



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 3

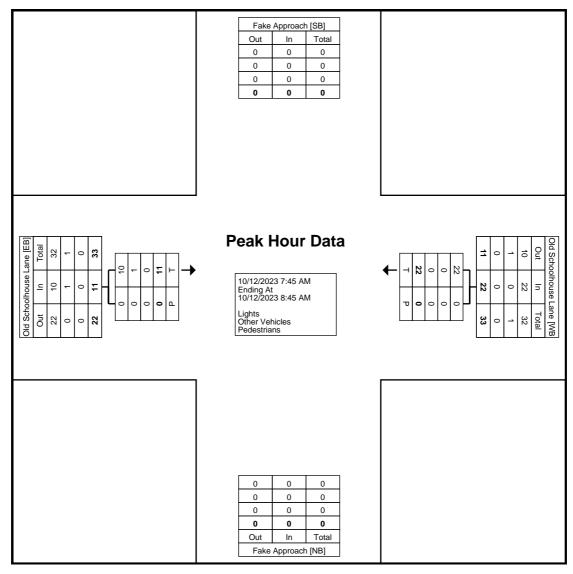
Turning Movement Peak Hour Data (7:45 AM)

		Old Schoolhouse Lar	ne				
Otant Time		Eastbound			Westbound		
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
7:45 AM	4	0	4	7	0	7	11
8:00 AM	2	0	2	5	0	5	7
8:15 AM	2	0	2	2	0	2	4
8:30 AM	3	0	3	8	0	8	11
Total	11	0	11	22	0	22	33
Approach %	100.0	-	-	100.0	-	-	-
Total %	33.3	_	33.3	66.7	-	66.7	-
PHF	0.688	-	0.688	0.688	-	0.688	0.750
Lights	10	-	10	22	-	22	32
% Lights	90.9	-	90.9	100.0	-	100.0	97.0
Other Vehicles	1	-	1	0	-	0	1
% Other Vehicles	9.1	-	9.1	0.0	-	0.0	3.0
Pedestrians	-	0	-	-	0	-	-
% Pedestrians	_	-			-		_



# Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:45 AM)



Counter: MIO: Set up by: KY:

### Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 5

Turning Movement Peak Hour Data (4:30 PM)

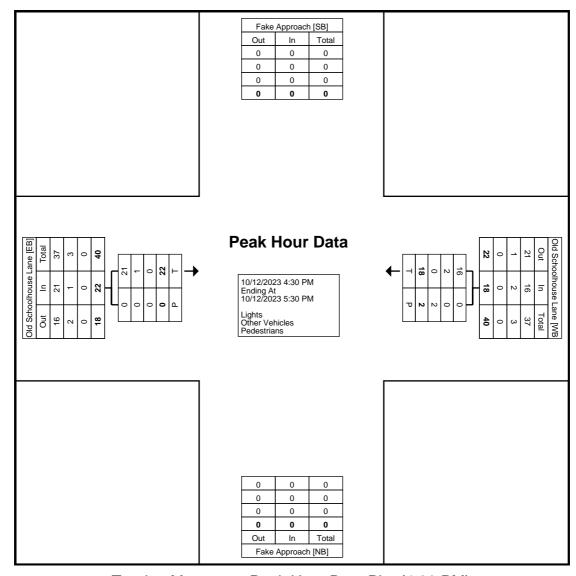
		Old Schoolhouse Lar Eastbound	ne	`	Old Schoolhouse Lar Westbound	ne	
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
4:30 PM	4	0	4	6	0	6	10
4:45 PM	3	0	3	4	0	4	7
5:00 PM	9	0	9	6	2	6	15
5:15 PM	6	0	6	2	0	2	8
Total	22	0	22	18	2	18	40
Approach %	100.0	-	-	100.0	-	-	-
Total %	55.0	-	55.0	45.0	-	45.0	-
PHF	0.611	-	0.611	0.750	-	0.750	0.667
Lights	21	-	21	16	-	16	37
% Lights	95.5	-	95.5	88.9	-	88.9	92.5
Other Vehicles	1	-	1	2	-	2	3
% Other Vehicles	4.5	-	4.5	11.1	-	11.1	7.5
Pedestrians	-	0	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0		-



Counter: MIO: Set up by: KY:

### Traffic Planning and Design, Inc 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:30 PM)

# **Appendix D:**Traffic Volume Development Worksheets

### **TPD# BLC.00127**

11/17/2023

**Traffic Volumes Worksheet** 

Intersection: Synchro Node:

Old Sch	ioolhou	ise Lane	9 & C	umber	land P	arkway	/ (Wes	t)	
Adjacent intersections:	West		East		North		South		

### Time Period: Weekday A.M. Peak Hour

	E	astbour	nd	V	/estbour	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		333	14	9	450		10		6				822
2023 Count Heavy Vehicles		13	1	0	28		0		0				42
2023 Existing Volumes (Balanced)	0	333	14	9	450	0	10	0	6	0	0	0	822
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	5	0	0	0	0	0	0	0	9
2025 Base Volumes	0	337	14	9	455	0	10	0	6	0	0	0	831
New Site Trips			14				12						26
Pass-By Trips		-9	9		-13		11						-2
Total Site Trip Distribution	0	-9	23	0	-13	0	23	0	0	0	0	0	24
2025 Projected Volumes	0	328	37	9	442	0	33	0	6	0	0	0	855
Base Growth (0.54% compounded for 12 yrs)	0	22	1	1	30	0	1	0	0	0	0	0	55
2035 Base Volumes	0	355	15	10	480	0	11	0	6	0	0	0	877
2035 Projected Volumes	0	346	38	10	467	0	34	0	6	0	0	0	901
		•		-	•		-					•	PHF
Existing (Count) Heavy Vehicles %		4%	7%	0%	6%		0%		0%				0.87

### Time Period: Weekday P.M. Peak Hour

	E	Eastbour	ıd	V	/estbour	nd	N	orthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		353	6	2	650		12		12				1035
2023 Count Heavy Vehicles		3	1	0	11		2		0				17
2023 Existing Volumes (Balanced)	0	353	6	2	650	0	12	0	12	0	0	0	1035
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	7	0	0	0	0	0	0	0	11
2025 Base Volumes	0	357	6	2	657	0	12	0	12	0	0	0	1046
New Site Trips			12				14						26
Pass-By Trips		-7	7		-13		14						1
Total Site Trip Distribution	0	-7	19	0	-13	0	28	0	0	0	0	0	27
2025 Projected Volumes	0	350	25	2	644	0	40	0	12	0	0	0	1073
Base Growth (0.54% compounded for 12 yrs)	0	24	0	0	43	0	1	0	1	0	0	0	69
2035 Base Volumes	0	377	6	2	693	0	13	0	13	0	0	0	1104
2035 Projected Volumes	0	370	25	2	680	0	41	0	13	0	0	0	1131
	•												PHF
Existing (Count) Heavy Vehicles %		1%	17%	0%	2%		17%		0%				0.93

### **TPD# BLC.00127**

11/17/2023 Traffic Volumes Worksheet Intersection: Synchro Node:

	Old Sch	oolhou	use Land	e & C	umberla	and Pa	arkway	(East	)	
2	Adjacent intersections:	West		East		North		South		

### Time Period: Weekday A.M. Peak Hour

	E	astbour	ıd	V	/estbour	nd	N	orthboui	nd	So	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		335	5	13	484		2		14				853
2023 Count Heavy Vehicles		17	0	0	30		0		1				48
2023 Existing Volumes (Balanced)	0	335	5	13	484	0	2	0	14	0	0	0	853
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	5	0	0	0	0	0	0	0	9
2025 Base Volumes	0	339	5	13	489	0	2	0	14	0	0	0	862
New Site Trips				14					13				27
Pass-By Trips		-9		13	-13				8				-1
Total Site Trip Distribution	0	-9	0	27	-13	0	0	0	21	0	0	0	26
2025 Projected Volumes	0	330	5	40	476	0	2	0	35	0	0	0	888
Base Growth (0.54% compounded for 12 yrs)	0	22	0	1	32	0	0	0	1	0	0	0	56
2035 Base Volumes	0	357	5	14	516	0	2	0	15	0	0	0	909
2035 Projected Volumes	0	348	5	41	503	0	2	0	36	0	0	0	935
													PHF
Existing (Count) Heavy Vehicles %		5%	0%	0%	6%		0%		7%				0.87

Time Period: Weekday P.M. Peak Hour

	E	astboun	ıd	V	/estbour	nd	N	orthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		375	1	16	627		2		21				1042
2023 Count Heavy Vehicles		6	0	2	11		0		1				20
2023 Existing Volumes (Balanced)	0	375	1	16	627	0	2	0	21	0	0	0	1042
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	7	0	0	0	0	0	0	0	11
2025 Base Volumes	0	379	1	16	634	0	2	0	21	0	0	0	1053
New Site Trips				12					15				27
Pass-By Trips		-7		13	-13				8				1
Total Site Trip Distribution	0	-7	0	25	-13	0	0	0	23	0	0	0	28
2025 Projected Volumes	0	372	1	41	621	0	2	0	44	0	0	0	1081
Base Growth (0.54% compounded for 12 yrs)	0	25	0	1	42	0	0	0	1	0	0	0	69
2035 Base Volumes	0	400	1	17	669	0	2	0	22	0	0	0	1111
2035 Projected Volumes	0	393	1	42	656	0	2	0	45	0	0	0	1139
													PHF
Existing (Count) Heavy Vehicles %		2%	0%	13%	2%		0%		5%				0.95

### **TPD# BLC.00127**

11/17/2023 Traffic Volumes Worksheet Intersection: Synchro Node:

Old Sc	hoolho	use Lane & I	Proposed Driv	veway (West)	
Adjacent intersections:	West	East	North	South	

### Time Period: Weekday A.M. Peak Hour

	E	astbour	ıd	V	/estbour	nd	N	orthbour	nd	Sc	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		11			22								33
2023 Count Heavy Vehicles		1			0								1
2023 Existing Volumes (Balanced)	0	11	0	0	22	0	0	0	0	0	0	0	33
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	11	0	0	22	0	0	0	0	0	0	0	33
New Site Trips	14				3	11				3		9	40
Pass-By Trips	9					13				8		11	41
Total Site Trip Distribution	23	0	0	0	3	24	0	0	0	11	0	20	81
2025 Projected Volumes	23	11	0	0	25	24	0	0	0	11	0	20	114
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	12	0	0	23	0	0	0	0	0	0	0	35
2035 Projected Volumes	23	12	0	0	26	24	0	0	0	11	0	20	116
													PHF
Existing (Count) Heavy Vehicles %	2%	9%			0%	2%				2%		2%	0.90

Time Period: Weekday P.M. Peak Hour

	E	astbour	ıd	V	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		22			18								40
2023 Count Heavy Vehicles		1			2								3
2023 Existing Volumes (Balanced)	0	22	0	0	18	0	0	0	0	0	0	0	40
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	22	0	0	18	0	0	0	0	0	0	0	40
New Site Trips	12				4	9				4		10	39
Pass-By Trips	7					13				8		14	42
Total Site Trip Distribution	19	0	0	0	4	22	0	0	0	12	0	24	81
2025 Projected Volumes	19	22	0	0	22	22	0	0	0	12	0	24	121
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	23	0	0	19	0	0	0	0	0	0	0	42
2035 Projected Volumes	19	23	0	0	23	22	0	0	0	12	0	24	123
													PHF
Existing (Count) Heavy Vehicles %	2%	5%			11%	2%				2%		2%	0.90

## **Appendix E:**Capacity and Queue Analysis

### **Supporting Calculations**

### **Crtical Headway and Follow-Up Headway Calculations**

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Development: Genius Kids Upper Allen

Location: Upper Allen Township, Cumberland County, PA

Intersection: Cumberland Parkway & Old Schoolhouse Lane (West)

Major Street: Cumberland Parkway

Minor Street: Old Schoolhouse Lane (West)

### **CRITICAL HEADWAY**

Equation:  $t_{c,x} = t_{c,base} + t_{C,HV} P_{HV} + t_{c,G} G - t_{3,LT}$ 

Where:  $t_{c,x} = Critical Headway for Movement X (seconds)$ 

t<sub>c.base</sub> = Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

t<sub>C,HV</sub> = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

 ${
m P_{HV}}$  = Percent of Heavy Vehicles  ${
m t_{c,G}}$  = Roadway Grade Adjustment Factor

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds)
Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

t<sub>3,LT</sub> = Intersection Geometry Adjustment Factor

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

### **CRITICAL HEADWAY**

	Major/Minor:	IIM	NOR	MAJOR
	Street:	Old Schoolho	use Ln (West)	<b>Cumberland Parkway</b>
	Movement:	NB Left	NB Right	WB Left
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>C,HV</sub>	1	1	1
AM	P <sub>HV</sub>	0	0	0
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	-1	-1	1
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	6.2	6.1	4.3
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>C,HV</sub>	1	1	1
PM	$P_{HV}$	0.17	0	0
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	-1	-1	1
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	6.4	6.1	4.3

### FOLLOW-UP HEADWAY

Equation:  $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$ 

Where:  $t_{fx}$  = Follow-up Headway for Movement X (seconds)

t<sub>f,base</sub> = Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

t<sub>f HV</sub> = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

P<sub>HV</sub> = Percent of Heavy Vehicles

### FOLLOW-UP HEADWAY

	FOLLOW-UP HEA	ADWAY		
	Major/Minor:	MI	NOR	MAJOR
	Street:	Old Schoolho	use Ln (West)	<b>Cumberland Parkway</b>
	Movement:	NB Left	NB Right	WB Left
	t <sub>f,base</sub>	3.0	3.1	3.0
	t <sub>f,HV</sub>	0.9	0.9	0.9
AM	P <sub>HV</sub>	0	0	0
Peak				
Hour				
	t <sub>f,x</sub>	3.0	3.1	3.0
	t <sub>f,base</sub>	3.0	3.1	3.0
	t <sub>f,HV</sub>	0.9	0.9	0.9
PM	P <sub>HV</sub>	0.17	0	0
Peak				
Hour				
	t <sub>f,x</sub>	3.2	3.1	3.0

### **Crtical Headway and Follow-Up Headway Calculations**

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Development: Genius Kids Upper Allen

Location: Upper Allen Township, Cumberland County, PA

Intersection: Cumberland Parkway & Old Schoolhouse Lane (East)

Major Street: Cumberland Parkway

Minor Street: Old Schoolhouse Lane (West)

### **CRITICAL HEADWAY**

Equation:  $t_{c,x} = t_{c,base} + t_{C,HV} P_{HV} + t_{c,G} G - t_{3,LT}$ 

Where:  $t_{c,x} = Critical Headway for Movement X (seconds)$ 

t<sub>c,base</sub> = Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

t<sub>C,HV</sub> = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

P<sub>HV</sub> = Percent of Heavy Vehicles

t<sub>c,G</sub> = Roadway Grade Adjustment Factor

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds)
Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

t<sub>3,LT</sub> = Intersection Geometry Adjustment Factor

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

### **CRITICAL HEADWAY**

	Major/Minor:	MII	NOR	MAJOR
	Street:	Old Schoolho	ouse Ln (East)	<b>Cumberland Parkway</b>
	Movement:	NB Left	NB Right	WB Left
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>c,HV</sub>	1	1	1
AM	P <sub>HV</sub>	0	0.07	0
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	-3	-3	2
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	5.8	6.0	4.3
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>c,HV</sub>	1	1	1
PM	$P_{HV}$	0	0.05	0.13
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	-3	-3	2
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	5.8	6.0	4.4

### FOLLOW-UP HEADWAY

Equation:  $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$ 

Where:  $t_{f,x}$  = Follow-up Headway for Movement X (seconds)

 $t_{f,base}$  = Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

t<sub>f HV</sub> = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

P<sub>HV</sub> = Percent of Heavy Vehicles

### FOLLOW-UP HEADWAY

	FOLLOW-UP HEA	ADWAY		
	Major/Minor:	IIM	NOR	MAJOR
	Street:	Old Schoolho	ouse Ln (East)	Cumberland Parkway
	Movement:	NB Left	NB Right	WB Left
	t <sub>f,base</sub>	3.0	3.1	3.0
	$t_{f,HV}$	0.9	0.9	0.9
AM	P <sub>HV</sub>	0	0.07	0
Peak				
Hour				
	t <sub>f,x</sub>	3.0	3.2	3.0
	t <sub>f,base</sub>	3.0	3.1	3.0
	t <sub>f,HV</sub>	0.9	0.9	0.9
PM	P <sub>HV</sub>	0	0.05	0.13
Peak				
Hour				
	t <sub>f,x</sub>	3.0	3.1	3.1

### Crtical Headway and Follow-Up Headway Calculations

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Genius Kids Upper Allen Development:

Upper Allen Township, Cumberland County, PA Location:

Old Schoolhouse Lane & Proposed Driveways Intersection:

Major Street: Old Schoolhouse Lane Minor Street: **Proposed Driveway** 

### **CRITICAL HEADWAY**

Equation:  $t_{c.x} = t_{c.base} + t_{C.HV} P_{HV} + t_{c.G} G - t_{3.LT}$ 

Critical Headway for Movement X (seconds) Where: t<sub>c,x</sub> =

Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

Adjustment Factor for Heavy Vehicles t<sub>c,HV</sub> =

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

P<sub>HV</sub> = Percent of Heavy Vehicles

Roadway Grade Adjustment Factor  $t_{c,G} =$ 

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds) Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

Intersection Geometry Adjustment Factor t<sub>3,LT</sub> =

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

### **CRITICAL HEADWAY**

	Major/Minor:	MII	NOR	MAJOR
	Street:	Propos	ed Dwy	Old Schoolhouse Lane
	Movement:	SB Left	SB Right	EB Left
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>c,HV</sub>	1	1	1
AM	P <sub>HV</sub>	0.02	0.02	0.02
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	0	0	1
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	6.4	6.2	4.3
	t <sub>c,base</sub>	7.1	6.2	4.3
	t <sub>C,HV</sub>	1	1	1
PM	P <sub>HV</sub>	0.02	0.02	0.02
Peak	t <sub>c,G</sub>	0.2	0.1	0
Hour	G	0	0	1
	t <sub>3,LT</sub>	0.7	0	0
	t <sub>c,x</sub>	6.4	6.2	4.3

### FOLLOW-UP HEADWAY

Equation:  $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$ 

Follow-up Headway for Movement X (seconds) Where:  $t_{f,x} =$ 

> $t_{f,base} =$ Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

Percent of Heavy Vehicles

### -----

	FOLLOW-UP HEA	DWAY		
	Major/Minor:	MI	NOR	MAJOR
	Street:	Propos	ed Dwy	Old Schoolhouse Lane
	Movement:	SB Left	SB Right	EB Left
	t <sub>f,base</sub>	3.0	3.1	3.0
	t <sub>f,HV</sub>	0.9	0.9	0.9
AM	P <sub>HV</sub>	0.02	0.02	0.02
Peak				
Hour				
	t <sub>f,x</sub>	3.0	3.1	3.0
	t <sub>f,base</sub>	3.0	3.1	3.0
	t <sub>f,HV</sub>	0.9	0.9	0.9
PM	P <sub>HV</sub>	0.02	0.02	0.02
Peak				
Hour				
	t <sub>e</sub>	3.0	3.1	3.0

### **2023 Existing Conditions Synchro Reports**

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>ĵ</b> ∌	•	ň	<b>†</b>	¥	
Traffic Volume (vph)	333	14	9	450	10	6
Future Volume (vph)	333	14	9	450	10	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1834	0	2035	1902	1988	0
Flt Permitted			0.950		0.970	
Satd. Flow (perm)	1834	0	2035	1902	1988	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	399	0	10	517	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 33.7%			IC	CU Level c	of Service

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		- ሽ		W	
Traffic Vol, veh/h	333	14	9	450	10	6
Future Vol, veh/h	333	14	9	450	10	6
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	_	None	_	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage, #		_	-	0	0	_
Grade, %	-2	_	_	1	-1	_
	87	87	87	87	87	87
Peak Hour Factor						
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	383	16	10	517	11	7
Major/Minor Ma	ajor1	N	/lajor2	N	Minor1	
Conflicting Flow All	0	0	399	0	928	391
Stage 1	-	-	-	-	391	-
Stage 2	-	-	-	-	537	-
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	876	-	331	705
Stage 1	-	-	-	-	799	-
Stage 2	_	_	_	_	685	_
Platoon blocked, %	_	_		_	000	
		_	876		207	705
Mov Cap-1 Maneuver	-			-	327	
Mov Cap-2 Maneuver	-	-	-	-	482	-
Stage 1	-	-	-	-	799	-
Stage 2	-	-	-	-	677	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		11.8	
	U		0.2			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		547	_	_	876	
HCM Lane V/C Ratio		0.034	_		0.012	_
HCM Control Delay (s)		11.8	_	_	9.2	_
HCM Lane LOS						
		B	-	-	A	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>		¥	<b>†</b>	¥	
Traffic Volume (vph)	335	5	13	484	2	14
Future Volume (vph)	335	5	13	484	2	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2025	1775	1800	0
Flt Permitted			0.950		0.994	
Satd. Flow (perm)	1816	0	2025	1775	1800	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	391	0	15	556	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 35.5%			IC	CU Level c	of Service A

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		<u>ነ</u>	<b>↑</b>	Y	
Traffic Vol, veh/h	335	5	13	484	2	14
Future Vol, veh/h	335	5	13	484	2	14
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,	# 0	_		0	0	_
Grade, %	,, J	_	_	2	-3	_
Peak Hour Factor	87	87	87	87	87	87
	5			6		
Heavy Vehicles, %		0	0		0	7
Mvmt Flow	385	6	15	556	2	16
Major/Minor M	ajor1	N	/lajor2	N	/linor1	
Conflicting Flow All	0	0	391	0	974	388
Stage 1	_	_	-	_	388	-
Stage 2	_	_	_	_	586	_
Critical Hdwy	_	_	4.4	_	5.8	6
					4.8	-
Critical Hdwy Stg 1	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	872	-	365	697
Stage 1	-	-	-	-	837	-
Stage 2	-	-	-	-	694	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	872	_	359	697
Mov Cap-2 Maneuver	_	_	-	_	504	-
Stage 1	_	_	_	_	837	_
		-		-	682	
Stage 2	-	-	-	<del>-</del>	002	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.6	
HCM LOS					В	
Minor Long/Maior M		JDI 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	Γ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		665	-	-	872	-
HCM Lane V/C Ratio		0.028	-	-	0.017	-
HCM Control Delay (s)		10.6	-	-	9.2	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0.1	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	-	$\rightarrow$	•	•	•	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)		7	<b>†</b>	W		
Traffic Volume (vph)	353	6	2	650	12	12	
Future Volume (vph)	353	6	2	650	12	12	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	16	14	16	16	
Grade (%)	-2%			1%	-1%		
Storage Length (ft)		0	50		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1892	0	2035	1977	1814	0	
Flt Permitted			0.950		0.976		
Satd. Flow (perm)	1892	0	2035	1977	1814	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	307			1469	293		
Travel Time (s)	6.0			28.6	8.0		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	386	0	2	699	26	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						П
Control Type: Unsignalize	d						
Intersection Capacity Utiliz	zation 44.2%			IC	CU Level o	of Service A	Α

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
		EDD	14/51	MAIST	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		<u> ነ</u>	<u></u>	¥	
Traffic Vol, veh/h	353	6	2	650	12	12
Future Vol, veh/h	353	6	2	650	12	12
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	380	6	2	699	13	13
Major/Minor M	ajor1	N	//ajor2	N	/linor1	
			386	0	1086	383
Conflicting Flow All	0	0			383	
Stage 1	-	-	-	-		-
Stage 2	-	-	4.0	-	703	- 0.4
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	885	-	254	712
Stage 1	-	-	-	-	750	-
Stage 2	-	-	-	-	530	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	885	-	253	712
Mov Cap-2 Maneuver	-	-	-	-	393	-
Stage 1	-	-	-	-	750	-
Stage 2	-	-	-	-	529	-
Annroach	ЕВ		WB		NB	
Approach						
HCM Control Delay, s	0		0		12.5	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		506		_	885	_
HCM Lane V/C Ratio		0.051	_		0.002	_
HCM Control Delay (s)		12.5	_	_	9.1	_
HCM Lane LOS		В	_	_	A	_
HCM 95th %tile Q(veh)		0.2	_	-	0	_
		J.L				

HCM 6th TWSC Synchro 11 Report XBPM.syn 11/17/2023

	<b>→</b>	$\searrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>f</b>		7	<b></b>	¥	
Traffic Volume (vph)	375	1	16	627	2	21
Future Volume (vph)	375	1	16	627	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1872	0	1792	1844	1823	0
FIt Permitted			0.950		0.996	
Satd. Flow (perm)	1872	0	1792	1844	1823	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	396	0	17	660	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 43.0%			IC	CU Level c	of Service A

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.4					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>}</b>		<u>ነ</u>	<b>^</b>	¥	04
Traffic Vol, veh/h	375	1	16	627	2	21
Future Vol, veh/h	375	1	16	627	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storag	e, # 0	-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	395	1	17	660	2	22
Major/Minor	Major1		Major2		/linor1	
Conflicting Flow All	0	0	396	0	1090	396
Stage 1	-	-	-	-	396	-
Stage 2	-	-	-	-	694	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	_	-	845	_	315	708
Stage 1	_	_	-	_	831	-
Stage 2		_		_	626	
Platoon blocked, %	-	-		-	020	-
			015		200	700
Mov Cap-1 Maneuver		-	845	-	309	708
Mov Cap-2 Maneuver		-	-	-	457	-
Stage 1	-	-	-	-	831	-
Stage 2	-	-	-	-	613	-
Approach	EB		WB		NB	
HCM Control Delay, s			0.2		10.5	
HCM LOS	U		0.2		10.5 B	
					В	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		676	_	_	845	_
HCM Lane V/C Ratio		0.036	_	_	0.02	_
HCM Control Delay (s	)	10.5	_	_	9.3	_
HCM Lane LOS	1	В	_	_	Α.	-
HCM 95th %tile Q(veh	١)	0.1			0.1	
	1)	U. I	-	-	U. I	-

HCM 6th TWSC Synchro 11 Report XBPM.syn 11/17/2023

### 2025 Base (No-Build) Condition Synchro Reports

	<b>→</b>	$\rightarrow$	•	•	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		7	<b>†</b>	W	
Traffic Volume (vph)	337	14	9	455	10	6
Future Volume (vph)	337	14	9	455	10	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1834	0	2035	1902	1988	0
Flt Permitted			0.950		0.970	
Satd. Flow (perm)	1834	0	2035	1902	1988	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	403	0	10	523	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 33.9%			IC	CU Level c	of Service

Lanes, Volumes, Timings
XBAM.syn
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Intersection						
Int Delay, s/veh	0.3					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>\$</b>	4.4	<u></u>	455	<b>Y</b>	^
Traffic Vol, veh/h	337	14	9	455	10	6
Future Vol, veh/h	337	14	9	455	10	6
Conflicting Peds, #/hr	_ 0	_ 0	0	_ 0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	387	16	10	523	11	7
Major/Minor Major/Minor	ajor1	N	//ajor2	N	Minor1	
Conflicting Flow All	0	0	403	0	938	395
Stage 1	-	-	-	-	395	-
Stage 2	_	_	_	_	543	_
Critical Hdwy	_	_	4.3	_	6.4	6.1
Critical Hdwy Stg 1	_	_		_	5.2	-
Critical Hdwy Stg 2	_	_		_	5.2	_
Follow-up Hdwy	_	_	3	_	3.2	3.1
Pot Cap-1 Maneuver	-	_	873	_	326	702
Stage 1	_	_	- 073	-	796	702
	_	-	-	-	681	
Stage 2		-	-	-	001	-
Platoon blocked, %	-	-	072	-	200	700
Mov Cap-1 Maneuver	-	-	873	-	322	702
Mov Cap-2 Maneuver	-	-	-	-	478	-
Stage 1	-	-	-	-	796	-
Stage 2	-	-	-	-	674	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		11.9	
HCM LOS			0.2		В	
110111 200						
NA: 1 /NA: NA (		IDI 4	EDT	<b>EDD</b>	MDI	MOT
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		543	-	-	873	-
HCM Lane V/C Ratio		0.034	-	-	0.012	-
HCM Control Delay (s)		11.9	-	-	9.2	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>∱</b>		ħ	<b>†</b>	W	
Traffic Volume (vph)	339	5	13	489	2	14
Future Volume (vph)	339	5	13	489	2	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2025	1775	1800	0
Flt Permitted			0.950		0.994	
Satd. Flow (perm)	1816	0	2025	1775	1800	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	396	0	15	562	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 35.7%			IC	CU Level c	of Service A

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7		*	<b>†</b>	¥	
Traffic Vol, veh/h	339	5	13	489	2	14
Future Vol, veh/h	339	5	13	489	2	14
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length	-	None -	225	None -	0	NULL
						-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-1	- 07	- 07	2	-3	- 07
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	390	6	15	562	2	16
Major/Minor M	ajor1	N	/lajor2		Minor1	
Conflicting Flow All	0	0	396	0	985	393
Stage 1	-	-	-		393	-
——————————————————————————————————————				-	592	
Stage 2	-	-	-	-		-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	868	-	360	692
Stage 1	-	-	-	-	833	-
Stage 2	-	-	-	-	690	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	868	_	354	692
Mov Cap-2 Maneuver	_	_	-	_	500	-
Stage 1	_	_	_	_	833	_
Stage 2	<u>-</u>	_	_	_	678	_
Olage Z		_			010	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.6	
HCM LOS					В	
		IDI 4	EDT		14/5:	MET
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		660	-	-	868	-
HCM Lane V/C Ratio		0.028	-	-	0.017	-
HCM Control Delay (s)		10.6	-	-	9.2	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0.1	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	-	$\rightarrow$	•	<b>←</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		7	<b>†</b>	W	
Traffic Volume (vph)	357	6	2	657	12	12
Future Volume (vph)	357	6	2	657	12	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1892	0	2035	1977	1814	0
FIt Permitted			0.950		0.976	
Satd. Flow (perm)	1892	0	2035	1977	1814	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	390	0	2	706	26	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 44.6%			IC	CU Level c	of Service

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
			MDI	MOT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		- ሽ		¥	
Traffic Vol, veh/h	357	6	2	657	12	12
Future Vol, veh/h	357	6	2	657	12	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	50	-	0	-
Veh in Median Storage,		_	-	0	0	_
Grade, %	<del>-</del> -2	_	_	1	-1	_
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	384	6	2	706	13	13
Major/Minor M	lajor1	ı	/lajor2	N	/linor1	
						207
Conflicting Flow All	0	0	390	0	1097	387
Stage 1	-	-	-	-	387	-
Stage 2	-	-	-	-	710	-
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	882	-	251	709
Stage 1	_	_	-	_	746	-
Stage 2	_	_	_	_	526	_
			-		520	_
Platoon blocked, %	-	-	000	-	050	700
Mov Cap-1 Maneuver	-	-	882	-	250	709
Mov Cap-2 Maneuver	-	-	-	-	390	-
Stage 1	-	-	-	-	746	-
Stage 2	-	-	-	-	525	-
A	ED		\A/D		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.5	
HCM LOS					В	
Minor Long/Major Marret		JDI 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		503	-	-	882	-
HCM Lane V/C Ratio		0.051	-	-	0.002	-
HCM Control Delay (s)		12.5	-	-	9.1	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.2	-	-	0	-

HCM 6th TWSC Synchro 11 Report XBPM.syn 11/17/2023

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		¥	<b>†</b>	¥	
Traffic Volume (vph)	379	1	16	634	2	21
Future Volume (vph)	379	1	16	634	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1872	0	1792	1844	1823	0
Flt Permitted			0.950		0.996	
Satd. Flow (perm)	1872	0	1792	1844	1823	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	400	0	17	667	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utiliz				IC	CU Level o	of Service A

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.4					
		ED.5	14/51	MOT	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ				N/	
Traffic Vol, veh/h	379	1	16	634	2	21
Future Vol, veh/h	379	1	16	634	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	_	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,		_	-	0	0	_
Grade, %	<del>π</del> -1	_	_	2	-3	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	399	1	17	667	2	22
Major/Minor M	/lajor1	N	Major2	N	/linor1	
			400		1101	400
Conflicting Flow All	0	0		0		400
Stage 1	-	-	-	-	400	-
Stage 2	-	-	-	-	701	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	-	-	842	_	311	705
Stage 1	-	-	-	-	828	-
Stage 2	_	_	_	_	622	_
Platoon blocked, %	_	_		_	<b>V</b>	
Mov Cap-1 Maneuver	_	_	842	_	305	705
Mov Cap-1 Maneuver		_	042	_	454	705
	-					
Stage 1	-	-	-	-	828	-
Stage 2	-	-	-	-	610	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.5	
	U		0.2			
HCM LOS					В	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		673		-	842	-
HCM Lane V/C Ratio		0.036		_	0.02	_
			-	_	9.4	
HCM Control Delay (s)		10.5	-	-		-
HCM Lane LOS		В	-	-	A	-
HCM 95th %tile Q(veh)		0.1	-	-	0.1	-

HCM 6th TWSC Synchro 11 Report XBPM.syn 11/17/2023

### 2035 Base (No-Build) Conditions Synchro Reports

	<b>→</b>	$\searrow$	•	<b>←</b>	1	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	₽		Ť	<b>↑</b>	¥		
Traffic Volume (vph)	355	15	10	480	11	6	
Future Volume (vph)	355	15	10	480	11	6	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	16	14	16	16	
Grade (%)	-2%			1%	-1%		
Storage Length (ft)		0	50		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1834	0	2035	1902	1998	0	
Flt Permitted			0.950		0.969		
Satd. Flow (perm)	1834	0	2035	1902	1998	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	307			1469	293		
Travel Time (s)	6.0			28.6	8.0		
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	425	0	11	552	20	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 35.3%			IC	CU Level o	of Service A	Α

Lanes, Volumes, Timings
XBAM.syn
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Intersection						
Int Delay, s/veh	0.3					
		ED.5	14/51	MAIST	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		- ሽ		W	
Traffic Vol, veh/h	355	15	10	480	11	6
Future Vol, veh/h	355	15	10	480	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mymt Flow	408	17	11	552	13	7
IVIVIIIL I IOVV	700	11	11	002	10	
Major/Minor M	lajor1	<u> </u>	/lajor2		Minor1	
Conflicting Flow All	0	0	425	0	991	417
Stage 1	-	-	-	_	417	-
Stage 2	_	_	_	_	574	_
Critical Hdwy	_	_	4.3	_	6.4	6.1
Critical Hdwy Stg 1	_	_		_	5.2	-
Critical Hdwy Stg 2		_	_	-	5.2	<u>-</u>
			3	-	3.2	3.1
Follow-up Hdwy	-	-				
Pot Cap-1 Maneuver	-	-	858	-	303	682
Stage 1	-	-	-	-	778	-
Stage 2	-	-	-	-	659	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	858	-	299	682
Mov Cap-2 Maneuver	-	-	-	-	458	-
Stage 1	-	-	-	-	778	-
Stage 2	_	_	-	-	650	-
2.0.33 2					300	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		12.2	
HCM LOS					В	
						14/5-
Minor Lane/Major Mvmt		VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		518	-	-	858	-
HCM Lane V/C Ratio		0.038	-	-	0.013	-
HCM Control Delay (s)		12.2	-	-	9.3	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		¥	<b>†</b>	¥	
Traffic Volume (vph)	357	5	14	516	2	15
Future Volume (vph)	357	5	14	516	2	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2025	1775	1799	0
Flt Permitted			0.950		0.995	
Satd. Flow (perm)	1816	0	2025	1775	1799	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	416	0	16	593	19	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utiliz				IC	CU Level o	of Service A

Lanes, Volumes, Timings
XBAM.syn
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Intersection						
Int Delay, s/veh	0.3					
	EDT	EDD	WDI	WDT	MDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		<u> ነ</u>	<u></u>	¥	
Traffic Vol, veh/h	357	5	14	516	2	15
Future Vol, veh/h	357	5	14	516	2	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,	# 0	_		0	0	_
Grade, %	-1	_	_	2	-3	_
Peak Hour Factor	87	87	87	87	87	87
	5			6		
Heavy Vehicles, %		0	0		0	7
Mvmt Flow	410	6	16	593	2	17
Major/Minor M	1ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	416	0	1038	413
Stage 1	-	-	-	-	413	-
Stage 2	-	-	-	-	625	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	854	-	337	675
Stage 1	-	-	-	-	818	-
Stage 2	_	_	_	_	669	_
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver		_	854		331	675
					480	
Mov Cap-2 Maneuver	-	-	-	-		-
Stage 1	-	-	-	-	818	-
Stage 2	-	-	-	-	656	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.8	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
		644		LDIN -	854	-
Capacity (veh/h)			-			
HCM Lane V/C Ratio		0.03	-	-	0.019	-
HCM Control Delay (s)		10.8	-	-	9.3	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0.1	-

HCM 6th TWSC Synchro 11 Report XBAM.syn 11/17/2023

	<b>→</b>	$\searrow$	•	<b>←</b>	4	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.		7	<b>†</b>	¥		
Traffic Volume (vph)	377	6	2	693	13	13	
Future Volume (vph)	377	6	2	693	13	13	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	16	14	16	16	
Grade (%)	-2%			1%	-1%		
Storage Length (ft)		0	50		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1892	0	2035	1977	1814	0	
Flt Permitted			0.950		0.976		
Satd. Flow (perm)	1892	0	2035	1977	1814	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	307			1469	293		
Travel Time (s)	6.0			28.6	8.0		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	411	0	2	745	28	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 46.5%			IC	CU Level c	of Service A	Α

Lanes, Volumes, Timings
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Intersection						
Int Delay, s/veh	0.3					
		EDD	WEL	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ		- ሽ		¥	
Traffic Vol, veh/h	377	6	2	693	13	13
Future Vol, veh/h	377	6	2	693	13	13
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	-2	_	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mymt Flow	405	6	2	745	14	14
IVIVIIIL FIOW	405	О		745	14	14
Major/Minor Ma	ajor1	N	//ajor2		Minor1	
Conflicting Flow All	0	0	411	0	1157	408
	-		411	-	408	400
Stage 1		-				
Stage 2	-	-	-	-	749	- 0.4
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	867	-	230	690
Stage 1	-	-	-	-	730	-
Stage 2	-	-	-	-	504	-
Platoon blocked, %	_	_		-		
Mov Cap-1 Maneuver	_	_	867	_	230	690
Mov Cap-1 Maneuver	_	_	-	_	371	-
	_	-			730	
Stage 1			-	-		-
Stage 2	-	-	-	-	503	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.9	
	U		U			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		483		-	867	-
HCM Lane V/C Ratio		0.058	_		0.002	-
HCM Control Delay (s)				-	9.2	
		12.9	-	-		
HCM Lane LOS HCM 95th %tile Q(veh)		B 0.2	-	-	A 0	-
			_			

HCM 6th TWSC Synchro 11 Report XBPM.syn 11/17/2023

	<b>→</b>	$\searrow$	•	<b>←</b>	1	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	₽		ň	<b>†</b>	¥		
Traffic Volume (vph)	400	1	17	669	2	22	
Future Volume (vph)	400	1	17	669	2	22	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	14	16	12	16	16	
Grade (%)	-1%			2%	-3%		
Storage Length (ft)		0	225		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1872	0	1792	1844	1823	0	
FIt Permitted			0.950		0.996		
Satd. Flow (perm)	1872	0	1792	1844	1823	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	1469			327	476		
Travel Time (s)	28.6			6.4	13.0		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	422	0	18	704	25	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 45.2%			IC	CU Level o	of Service A	Α

Intersection						
Int Delay, s/veh	0.4					
	EDT	EDD	WDL	\\/DT	NDI	NIDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>}</b>		<u> </u>	<b>↑</b>	À	
Traffic Vol, veh/h	400	1	17	669	2	22
Future Vol, veh/h	400	1	17	669	2	22
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mymt Flow	421	1	18	704	2	23
IVIVIIIL I IUW	42 I	I	10	704		23
Major/Minor M	ajor1	N	//ajor2	ľ	Minor1	
Conflicting Flow All	0	0	422	0	1162	422
Stage 1	-	-	-	-	422	-
Stage 2	_	_	_	_	740	_
			4.4			
Critical Hdwy	-	-		-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	-	-	827	-	288	686
Stage 1	-	-		-	811	-
Stage 2	-	-	-	-	599	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	827	_	282	686
Mov Cap-2 Maneuver	_	_	-	_	434	-
Stage 1	_	_	_	_	811	_
•	_	_			586	
Stage 2	-	-	-	-	200	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.7	
HCM LOS	U		0.2		В	
HOW LOS					В	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		654	_	_	827	_
HCM Lane V/C Ratio		0.039	_		0.022	_
HCM Control Delay (s)		10.7	_		9.4	_
HCM Lane LOS		В			9.4 A	
HCM 95th %tile Q(veh)		0.1	-	-	0.1	-
ncivi som wille Given)		U. I	-	_	U. I	-

## **2025 Projected (Build) Conditions Synchro Reports**

	<b>→</b>	$\rightarrow$	•	←	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	î»		7	<b>†</b>	W	
Traffic Volume (vph)	328	37	9	442	33	6
Future Volume (vph)	328	37	9	442	33	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1814	0	2035	1902	2032	0
Flt Permitted			0.950		0.959	
Satd. Flow (perm)	1814	0	2035	1902	2032	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	420	0	10	508	45	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utiliz				IC	CU Level o	of Service A

Intersection						
Int Delay, s/veh	0.7					
		EDD	14/51	MOT	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>\$</b>	•	<u></u>	<b>†</b>	<b>Y</b>	_
Traffic Vol, veh/h	328	37	9	442	33	6
Future Vol, veh/h	328	37	9	442	33	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	377	43	10	508	38	7
Major/Minor N	/lajor1	N	/lajor2	N	Minor1	
Conflicting Flow All	0	0	420	0	927	399
Stage 1	-	-	420	-	399	J99 -
Stage 2	_	-	_	_	528	_
Critical Hdwy	_	-	4.3		6.2	6.1
Critical Hdwy Stg 1	_		4.5	-	5.2	0.1
Critical Hdwy Stg 2	_	-	-	-	5.2	-
Follow-up Hdwy	-		3	-	3.2	3.1
Pot Cap-1 Maneuver	_	-	861		349	698
Stage 1	_		- 001	<u>-</u>	793	090
Stage 1	-	-	-	_	692	-
Platoon blocked, %	-	-	-	- -	UJZ	-
Mov Cap-1 Maneuver	-	-	861		345	698
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	-	-	001	- -	345 491	098
	-	-	-			
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	684	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		12.7	
HCM LOS					В	
Minor Long /Mailes M		VIDL 4	EDT	EDD	MDI	MDT
Minor Lane/Major Mvmt	ι Γ	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		514	-	-	861	-
HUM Land VIC Datio		0.087	-		0.012	-
HCM Control Dolor (a)		40 -				
HCM Control Delay (s)		12.7	-	-	9.2	-
		12.7 B 0.3	- -	-	9.2 A 0	-

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		ħ	<b>†</b>	W	
Traffic Volume (vph)	330	5	40	476	2	35
Future Volume (vph)	330	5	40	476	2	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2025	1775	1781	0
Flt Permitted			0.950		0.998	
Satd. Flow (perm)	1816	0	2025	1775	1781	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	385	0	46	547	42	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utiliz				IC	CU Level o	of Service A

Intersection						
Int Delay, s/veh	0.8					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽				N/F	
Traffic Vol, veh/h	330	5	40	476	2	35
Future Vol, veh/h	330	5	40	476	2	35
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	_
Veh in Median Storage, #	<del>#</del> 0	_	-	0	0	-
Grade, %	-1	_	_	2	-3	_
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
	379		46	547		40
Mvmt Flow	3/9	6	40	547	2	40
Major/Minor Ma	ajor1	N	//ajor2	N	Minor1	
Conflicting Flow All	0	0	385	0	1021	382
Stage 1	-	-	-	-	382	-
•					639	
Stage 2	-	-	- 4.0	-		-
Critical Hdwy	-	-	4.3	-	5.8	5.9
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	886	-	344	729
Stage 1	-	-	-	-	842	-
Stage 2	-	-	-	-	660	-
Platoon blocked, %	_	-		_		
Mov Cap-1 Maneuver	_	_	886	_	326	729
Mov Cap-2 Maneuver	_	_	-	<u>-</u>	471	-
Stage 1	_		-		842	_
		-	-	-		
Stage 2	-	-	-	-	626	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.7		10.4	
HCM LOS	v		0.1		В	
TIOW LOS					U	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		708	_	_	886	_
HCM Lane V/C Ratio		0.06	_	_	0.052	_
HCM Control Delay (s)		10.4	_	_	9.3	_
HCM Lane LOS		В		<u>-</u>	Α.	_
HCM 95th %tile Q(veh)		0.2	-	<u>-</u>	0.2	_
		U.Z	_	-	0.2	

3: Old Schoolhouse Lane (West)/Old Schoolhouse Lane & Proposed Site Driveway (West)

11111119 1 101111 7 1111								_	
	•	<b>→</b>	<b>←</b>	1	<b>&gt;</b>	1		_	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		4	ĵ»		¥				
Traffic Volume (vph)	23	11	25	24	11	20			
Future Volume (vph)	23	11	25	24	11	20			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Width (ft)	16	16	16	16	12	12			
Grade (%)		1%	-1%		0%				
Satd. Flow (prot)	0	1988	2002	0	1672	0			
Flt Permitted		0.967			0.983				
Satd. Flow (perm)	0	1988	2002	0	1672	0			
Link Speed (mph)		25	30		30				
Link Distance (ft)		948	144		107				
Travel Time (s)		25.9	3.3		2.4				
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90			
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%			
Shared Lane Traffic (%)									
Lane Group Flow (vph)	0	38	55	0	34	0			
Sign Control		Free	Free		Stop				
Intersection Summary									
Area Type:	Other								
Control Type: Unsignalize	ed								
Intersection Capacity Utili	zation 18.5%			IC	CU Level c	of Service	Α		
Analysis Period (min) 15									

Synchro 11 Report 11/17/2023 Lanes, Volumes, Timings PAM.syn

Intersection						
Int Delay, s/veh	4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4		וטיי	SBL ₩	אומט
Traffic Vol, veh/h	23	- <b>원</b> 11	<b>♣</b> 25	24	11	20
Future Vol, veh/h	23	11	25	24	11	20
	23	0	25	0	0	0
Conflicting Peds, #/hr Sign Control	-	Free	Free	Free		
RT Channelized	Free	None		None	Stop	Stop
	-		-		-	None
Storage Length	- ш	-	-	-	0	-
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	26	12	28	27	12	22
Major/Minor M	lajor1	N	/lajor2	N	Minor2	
Conflicting Flow All	55	0		0	106	42
Stage 1	-	-	-	-	42	-
Stage 2	_	_	-	_	64	_
Critical Hdwy	4.3	-	_	-	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	3	_	_	<u>-</u>	3	3.1
Pot Cap-1 Maneuver	1150	_	_	_	1038	1100
Stage 1	-	_	_	<u>-</u>	1146	-
Stage 2		-			1119	
Platoon blocked, %				-	1113	
Mov Cap-1 Maneuver	1150	-		<u>-</u>	1014	1100
Mov Cap-1 Maneuver	-	_	_	<u>-</u>	1014	-
Stage 1		-			1120	
•		•	-		1119	
Stage 2	-	-	-	<del>-</del>	1119	-
Approach	EB		WB		SB	
HCM Control Delay, s	5.5		0		8.5	
HCM LOS					A	
Minor Long/Maior M.		EDI	EDT	WDT	WDD	CDL 4
Minor Lane/Major Mvmt		EBL	EBT	WBT		SBLn1
Capacity (veh/h)		1150	-	-		1068
HCM Lane V/C Ratio		0.022	-	-		0.032
		8.2	0	-	-	8.5
HCM Control Delay (s)						
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		A 0.1	A	-	-	A 0.1

4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

Tilling Flan. 7 Will	oun i loc	11					
	٦	<b>→</b>	<b>←</b>	4	<b>&gt;</b>	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्स	ĵ.		M.		
Traffic Volume (vph)	0	22	46	3	10	3	
Future Volume (vph)	0	22	46	3	10	3	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	1966	2144	0	1740	0	
Flt Permitted					0.962		
Satd. Flow (perm)	0	1966	2144	0	1740	0	
Link Speed (mph)		25	25		25		
Link Distance (ft)		144	476		112		
Travel Time (s)		3.9	13.0		3.1		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	24	54	0	14	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize							
Intersection Capacity Utiliz	zation 13.3%			IC	CU Level of	of Service A	4
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL			WDIX	₩.	SDIX
Lane Configurations	0	4	<b>}</b>	2		2
Traffic Vol, veh/h	0	22	46	3	10	3
Future Vol, veh/h	0	22	46	3	10	3
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	0	24	51	3	11	3
NA - ' /NA'	4.1.4		40		I' O	
	Major1		//ajor2		Minor2	
Conflicting Flow All	54	0	-	0	77	53
Stage 1	-	-	-	-	53	-
Stage 2	-	-	-	-	24	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1151	-	_	-	1080	1084
Stage 1	-	_	_	_	1133	-
Stage 2	_	_	_	_	1169	_
Platoon blocked, %		_	_	_	1100	
Mov Cap-1 Maneuver	1151	_	_	_	1080	1084
				_	1080	1004
Mov Cap-2 Maneuver	-	-	-			
Stage 1	-	-	-	-	1133	-
Stage 2	-	-	-	-	1169	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.4	
HCM LOS	-		- 0		Α	
TIOWI LOO						
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1151	-	-	-	1081
HCM Lane V/C Ratio		-	-	-	-	0.013
HCM Control Delay (s)		0	-	-	-	8.4
HCM Lane LOS		A	-	-	-	Α
HCM 95th %tile Q(veh)		0	_	-	_	0

	-	$\rightarrow$	•	<b>←</b>	4	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)		N.	<b>†</b>	¥		
Traffic Volume (vph)	350	25	2	644	40	12	
Future Volume (vph)	350	25	2	644	40	12	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	16	14	16	16	
Grade (%)	-2%			1%	-1%		
Storage Length (ft)		0	50		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1863	0	2035	1977	1786	0	
Flt Permitted			0.950		0.963		
Satd. Flow (perm)	1863	0	2035	1977	1786	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	307			1469	293		
Travel Time (s)	6.0			28.6	8.0		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	403	0	2	692	56	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 43.9%			IC	CU Level c	of Service	Α

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	LDIX	VVDL		₩.	TIDIN
Traffic Vol, veh/h	350	25	2	<b>T</b> 644	<b>4</b> 0	12
Future Vol, veh/h	350	25	2	644	40	12
Conflicting Peds, #/hr	0	0	0	044	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	Stop -	None
Storage Length	_	-	50	-	0	-
Veh in Median Storage			-	0	0	_
Grade, %	-2	-	_	1	-1	_
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	376	27	2	692	43	13
IVIVIIIL I IUW	310	ZI		032	43	10
Major/Minor	Major1	N	Major2	1	Minor1	
Conflicting Flow All	0	0	403	0	1086	390
Stage 1	-	-	-	-	390	-
Stage 2	-	-	-	-	696	-
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	_	_	-	5.37	-
Critical Hdwy Stg 2	-	-	-	_	5.37	-
Follow-up Hdwy	_	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	873	_	265	706
Stage 1	_	-	-	-	786	-
Stage 2	_	-	_	_	560	-
Platoon blocked, %	-	_		_	- 500	
Mov Cap-1 Maneuver	_	_	873	_	264	706
Mov Cap-1 Maneuver		_	-	_	413	-
Stage 1		_	_	_	786	_
Stage 2		_	_	_	559	_
Staye 2	-	-	<u>-</u>	-	303	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		14	
HCM LOS					В	
NA:		NIDL 4	CDT	EDD	\A/D!	MOT
Minor Lane/Major Mvn	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		457	-	-	873	-
HCM Lane V/C Ratio		0.122	-	-	0.002	-
HCM Control Delay (s	)	14	-	-	9.1	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh	1)	0.4	-	-	0	-

	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		¥	<b></b>	¥	
Traffic Volume (vph)	372	1	41	621	2	44
Future Volume (vph)	372	1	41	621	2	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1872	0	1792	1844	1813	0
Flt Permitted			0.950		0.998	
Satd. Flow (perm)	1872	0	1792	1844	1813	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	393	0	43	654	48	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 42.7%			IC	CU Level o	of Service A

Intersection						
Int Delay, s/veh	0.8					
		===	14/5-	14/5-		NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	-₽				N/	
Traffic Vol, veh/h	372	1	41	621	2	44
Future Vol, veh/h	372	1	41	621	2	44
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,	# 0	_	_	0	0	_
Grade, %	-1	_	_	2	-3	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	392	1	43	654	2	46
IVIVIIIL FIUW	392	I	43	034		40
Major/Minor Major/Minor	ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	393	0	1133	393
Stage 1	-	-	-	-	393	-
Stage 2	_	_	_	<u>-</u>	740	_
		_	4.4		5.8	5.9
Critical Hdwy	-					
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	870	-	299	719
Stage 1	-	-	-	-	833	-
Stage 2	-	-	-	-	599	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	870	-	284	719
Mov Cap-2 Maneuver	-	-	-	-	431	-
Stage 1	-	_	_	_	833	_
Stage 2	_	_	_	_	570	_
Olago Z	_	_			310	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.6		10.5	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		699	-	-	870	-
HCM Lane V/C Ratio		0.069	-	-	0.05	-
HCM Control Delay (s)		10.5	-	-	9.4	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.2	_	_	0.2	_
		J.L			5.2	

Fiming Plan: PM Peak Hour 3: Old Schoolhouse Lane (West)/Old Schoolhouse Lane & Proposed Site Driveway (West)

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4	f)		¥		
Traffic Volume (vph)	19	22	22	22	12	24	
Future Volume (vph)	19	22	22	22	12	24	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	2021	1894	0	1666	0	
FIt Permitted		0.977			0.984		
Satd. Flow (perm)	0	2021	1894	0	1666	0	
Link Speed (mph)		25	30		30		
Link Distance (ft)		948	144		107		
Travel Time (s)		25.9	3.3		2.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	45	48	0	40	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	ation 18.9%			IC	CU Level c	of Service A	4
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EDL			WDK		SDR
Lane Configurations	10	4	<b>}</b>	22	<b>\</b>	24
Traffic Vol, veh/h	19	22	22	22	12	24
Future Vol, veh/h	19	22	22	22	12	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	110110	-		-	
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	21	24	24	24	13	27
Major/Minor	lais=1		/nic=0		line=0	
	lajor1		//ajor2		Minor2	
Conflicting Flow All	48	0	-	0	102	36
Stage 1	-	-	-	-	36	-
Stage 2	-	-	-	-	66	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1156	-	-	-	1044	1108
Stage 1	_	-	_	-	1154	-
Stage 2	-	_	-	_	1117	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1156	_	_	_	1025	1108
Mov Cap-1 Maneuver	-	_	_	_	1025	-
Stage 1				<u>-</u>	1133	
	-	-	-	-		-
Stage 2	-	-	-	-	1117	-
Approach	EB		WB		SB	
HCM Control Delay, s	3.8		0		8.5	
HCM LOS	3.0				A	
					, ,	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	
Capacity (veh/h)		1156	-	-	-	1079
HCM Lane V/C Ratio		0.018	-	-	-	0.037
HCM Control Delay (s)		8.2	0	-	-	8.5
HCM Lane LOS		Α	A	-	-	Α
HCM 95th %tile Q(veh)		0.1	_	_	_	0.1

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		4	ĵ.		W			
Traffic Volume (vph)	0	34	40	3	11	4		
Future Volume (vph)	0	34	40	3	11	4		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	16	16	16	16	12	12		
Grade (%)		1%	-1%		0%			
Satd. Flow (prot)	0	2041	1942	0	1735	0		
Flt Permitted					0.964			
Satd. Flow (perm)	0	2041	1942	0	1735	0		
Link Speed (mph)		25	25		25			
Link Distance (ft)		144	476		112			
Travel Time (s)		3.9	13.0		3.1			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	38	47	0	16	0		
Sign Control		Free	Free		Stop			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalize								
Intersection Capacity Utiliz	zation 13.3%			IC	CU Level of	of Service A	١	
Analysis Period (min) 15								

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	WB1 <b>}</b>	וטא	SBL ₩	אומט
Traffic Vol, veh/h	٥	34	40	3	11	4
	0			3		
Future Vol, veh/h	0	34	40	3	11	4
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	0	38	44	3	12	4
		- 00			12	- 1
Major/Minor I	Major1	N	Major2	N	Minor2	
Conflicting Flow All	47	0	-	0	84	46
Stage 1	-	-	-	-	46	-
Stage 2	_	_	_	_	38	-
Critical Hdwy	4.3	_	_	_	6.42	6.22
Critical Hdwy Stg 1	1.0	<u>-</u>	_	<u>-</u>	5.42	- 0.22
Critical Hdwy Stg 2		_		<u>-</u>	5.42	_
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1157	-	-	-	1070	1094
Stage 1	-	-	-	-	1141	-
Stage 2	-	-	-	-	1151	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1157	-	-	-	1070	1094
Mov Cap-2 Maneuver	-	-	-	-	1070	-
Stage 1	_	-	_	_	1141	_
Stage 2	_	_	_	_	1151	_
Olaye Z		_			1101	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.4	
HCM LOS					Α	
TOW LOO						
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1157	-	-	-	1076
HCM Lane V/C Ratio		_	-	_	_	0.015
HCM Control Delay (s)		0	_	_	_	8.4
HCM Lane LOS		A	_	_	_	Α
HCM 95th %tile Q(veh)		0		_		0
HOW SOUT WILL Q(VEI)		U	_			U

## 2035 Projected (Build) Conditions Synchro Reports

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		¥	<b>†</b>	¥	
Traffic Volume (vph)	346	38	10	467	34	6
Future Volume (vph)	346	38	10	467	34	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2035	1902	2032	0
Flt Permitted			0.950		0.959	
Satd. Flow (perm)	1816	0	2035	1902	2032	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	442	0	11	537	46	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 34.6%			IC	CU Level c	of Service A

Intersection						
Int Delay, s/veh	0.7					
		EDD	\\/DI	\\/DT	NDI	NBR
Movement	EBT	EBR	WBL	WBT	NBL	NRK
Lane Configurations	<b>}</b>	00	<u>ነ</u>	107	¥	^
Traffic Vol, veh/h	346	38	10	467	34	6
Future Vol, veh/h	346	38	10	467	34	6
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	398	44	11	537	39	7
	1ajor1		Major2		Minor1	
Conflicting Flow All	0	0	442	0	979	420
Stage 1	-	-	-	-	420	-
Stage 2	-	-	-	-	559	-
Critical Hdwy	-	-	4.3	-	6.2	6.1
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	_	-	-	_	5.2	-
Follow-up Hdwy	_	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	846	_	325	679
Stage 1	_	_	-	_	775	-
Stage 2			_	_	669	_
Platoon blocked, %	_	_	_		003	_
	<del>-</del>		0.40	-	204	670
Mov Cap-1 Maneuver	-	-	846	-	321	679
Mov Cap-2 Maneuver	-	-	-	-	470	-
Stage 1	-	-	-	-	775	-
Stage 2	-	-	-	-	660	-
Approach	EB		WB		NB	
			0.2			
HCM Control Delay, s	0		0.2		13.1	
HCM LOS					В	
		NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Maior Mymt					846	
Minor Lane/Major Mvmt Capacity (veh/h)		493			0.10	
Capacity (veh/h)		493	-	_	0.014	_
Capacity (veh/h) HCM Lane V/C Ratio		0.093	-		0.014	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.093 13.1	-	-	9.3	-
Capacity (veh/h) HCM Lane V/C Ratio		0.093				

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- 1}		¥	<u></u>	¥	
Traffic Volume (vph)	348	5	41	503	2	36
Future Volume (vph)	348	5	41	503	2	36
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1816	0	2025	1775	1781	0
Flt Permitted			0.950		0.998	
Satd. Flow (perm)	1816	0	2025	1775	1781	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	406	0	47	578	43	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 36.5%			IC	CU Level o	of Service A

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EDD	WBL	WBT	NBL	NBR
		EBR				NDK
Lane Configurations	<b>}</b>		<u>ነ</u>	<b>f</b> 02	¥	20
Traffic Vol, veh/h	348	5	41	503	2	36
Future Vol, veh/h	348	5	41	503	2	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	400	6	47	578	2	41
Majay/Minay	NA=1==4		Asia =0		Alime = 4	
	Major1		Major2		Minor1	400
Conflicting Flow All	0	0	406		1075	403
Stage 1	-	-	-	-	403	-
Stage 2	-	-	-	-	672	-
Critical Hdwy	-	-	4.3	-	5.8	5.9
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	871	-	321	710
Stage 1	-	-	-	-	825	-
Stage 2	_	-	-	-	640	-
Platoon blocked, %	_	_		_	J.0	
Mov Cap-1 Maneuver	_	_	871	_	304	710
Mov Cap-1 Maneuver	_	_	-	_	452	7 10
	-	-			825	-
Stage 1	-	-	-	-		
Stage 2	-	-	-	-	605	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.7		10.6	
HCM LOS	U		0.1		В	
TIOWI LOO					D	
Minor Lane/Major Mvn	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		689	-	-	871	-
HCM Lane V/C Ratio		0.063	-	_	0.054	-
HCM Control Delay (s)		10.6	-	-	9.4	-
HCM Lane LOS		В	_	_	A	-
HCM 95th %tile Q(veh	)	0.2	_	_	0.2	_

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	۶	-	<b>←</b>	•	<b>&gt;</b>	4	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4	£		, A		
Traffic Volume (vph)	23	12	26	24	11	20	
Future Volume (vph)	23	12	26	24	11	20	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	1988	2004	0	1672	0	
Flt Permitted		0.968			0.983		
Satd. Flow (perm)	0	1988	2004	0	1672	0	
Link Speed (mph)		25	30		30		
Link Distance (ft)		948	144		107		
Travel Time (s)		25.9	3.3		2.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	39	56	0	34	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	d						
Intersection Capacity Utiliz	zation 18.6%			IC	CU Level of	of Service A	
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	3.9					
		EST	MET	MES	051	055
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	₽		W	
Traffic Vol, veh/h	23	12	26	24	11	20
Future Vol, veh/h	23	12	26	24	11	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage,		0	0	_	0	_
Grade, %	, <del>#</del> -	1	-1	<u> </u>	0	_
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	26	13	29	27	12	22
Major/Minor N	/lajor1	N	//ajor2	N	Minor2	
	_					40
Conflicting Flow All	56	0	-	0	108	43
Stage 1	-	-	-	-	43	-
Stage 2	-	-	-	-	65	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	_	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1149	_	-	-	1035	1098
Stage 1		_	_	_	1145	-
Stage 2	_				1118	_
		-	-		1110	-
Platoon blocked, %	11.10	-	-	-	1011	4000
Mov Cap-1 Maneuver	1149	-	-	-	1011	1098
Mov Cap-2 Maneuver	-	-	-	-	1011	-
Stage 1	-	-	-	-	1119	-
Stage 2	-	_	-	-	1118	-
A	ED		VAID		O.D.	
Approach	EB		WB		SB	
HCM Control Delay, s	5.4		0		8.5	
HCM LOS					Α	
Minor Long/Major Mund		EBL	EDT	MDT	WBR :	CDI ~1
Minor Lane/Major Mvmt			EBT	WBT		
Capacity (veh/h)		1149	-	-		1065
HCM Lane V/C Ratio		0.022	-	-	-	0.032
HCM Control Delay (s)		8.2	0	-	-	8.5
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh)		0.1	-	-	-	0.1
,						

4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

Tilling Flan. Aw	I Cak I loc	(1	T. Old (	3011001110	doc Lario		incuse Earle (East) a 1 reposed one Briveway (East)
	•	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	✓	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्स	₽		N/F		
Traffic Volume (vph)	0	23	47	3	10	3	
Future Volume (vph)	0	23	47	3	10	3	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	1966	2147	0	1740	0	
Flt Permitted					0.962		
Satd. Flow (perm)	0	1966	2147	0	1740	0	
Link Speed (mph)		25	25		25		
Link Distance (ft)		144	476		112		
Travel Time (s)		3.9	13.0		3.1		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	26	55	0	14	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	ization 13.3%			IC	CU Level o	of Service A	A .
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	₽		N/	
Traffic Vol, veh/h	0	23	47	3	10	3
Future Vol, veh/h	0	23	47	3	10	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	_	-	0	-
Veh in Median Storage	.# -	0	0	_	0	_
Grade, %	, <i>''</i>	1	-1	_	0	_
Peak Hour Factor	90	90	90	90	90	90
	2	9	0	2	2	2
Heavy Vehicles, %		26	52		11	3
Mvmt Flow	0	20	52	3		3
Major/Minor N	Major1	Λ	//ajor2	N	Minor2	
Conflicting Flow All	55	0	-	0	80	54
Stage 1	-			-	54	
•	-	-	-			-
Stage 2	4.0	-	-	-	26	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1150	-	-	-	1076	1083
Stage 1	-	-	-	-	1131	-
Stage 2	-	-	-	-	1166	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1150	_	_	-	1076	1083
Mov Cap-2 Maneuver	-	_	_	_	1076	-
Stage 1	_	_	_	_	1131	_
•			_	-	1166	
Stage 2	-	-	-	-	1100	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.4	
HCM LOS	U		- 0		Α	
I IOWI LOO					٨	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1150	-	-	_	1078
HCM Lane V/C Ratio		-	_	_		0.013
HCM Control Delay (s)		0	_	_	_	8.4
HCM Lane LOS		A	_	_	_	A
HCM 95th %tile Q(veh)		0	-			0
HOW SOUT MILE Q(VEIT)		U	-	_	-	U

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>		ħ	<b>†</b>	W		
Traffic Volume (vph)	370	25	2	680	41	13	
Future Volume (vph)	370	25	2	680	41	13	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	16	14	16	16	
Grade (%)	-2%			1%	-1%		
Storage Length (ft)		0	50		0	0	
Storage Lanes		0	1		1	0	
Taper Length (ft)			25		25		
Satd. Flow (prot)	1864	0	2035	1977	1785	0	
FIt Permitted			0.950		0.963		
Satd. Flow (perm)	1864	0	2035	1977	1785	0	
Link Speed (mph)	35			35	25		
Link Distance (ft)	307			1469	293		
Travel Time (s)	6.0			28.6	8.0		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	425	0	2	731	58	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 45.8%			IC	CU Level o	of Service A	Α

Intersection						
Int Delay, s/veh	0.7					
		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	٥٢	<u>*</u>	<b>↑</b>	Y	40
Traffic Vol, veh/h	370	25	2	680	41	13
Future Vol, veh/h	370	25	2	680	41	13
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	398	27	2	731	44	14
Major/Minor Ma	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	425	0	1147	412
Stage 1	-	-	425	-	412	412
Stage 2	_	_		_	735	-
Critical Hdwy	_	_	4.3	_	6.4	6.1
•			4.3			
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	858	-	243	686
Stage 1	-	-	-	-	767	-
Stage 2	-	-	-	-	536	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	858	-	243	686
Mov Cap-2 Maneuver	-	-	-	-	393	-
Stage 1	-	-	-	-	767	-
Stage 2	-	-	-	-	535	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		14.5	
HCM LOS					В	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		438	-	_		_
HCM Lane V/C Ratio		0.133	-	_	0.003	-
HCM Control Delay (s)		14.5	_	_	9.2	_
HCM Lane LOS		В	_	_	A	_
		_				
HCM 95th %tile Q(veh)		0.5	-	_	0	_

	<b>→</b>	$\rightarrow$	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		¥	<b>†</b>	¥	
Traffic Volume (vph)	393	1	42	656	2	45
Future Volume (vph)	393	1	42	656	2	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Satd. Flow (prot)	1872	0	1792	1844	1813	0
Flt Permitted			0.950		0.998	
Satd. Flow (perm)	1872	0	1792	1844	1813	0
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	415	0	44	691	49	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 44.5%			IC	CU Level c	of Service A

Intersection						
Int Delay, s/veh	0.8					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4		<u>ነ</u>	<b>↑</b>	¥	
Traffic Vol, veh/h	393	1	42	656	2	45
Future Vol, veh/h	393	1	42	656	2	45
Conflicting Peds, #/hr	_ 0	0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	414	1	44	691	2	47
Major/Minor	loier1		laier?		liner1	
	/lajor1		Major2		/linor1	445
Conflicting Flow All	0	0	415		1194	415
Stage 1	-	-	-	-	415	-
Stage 2	-	-	-	-	779	-
Critical Hdwy	-	-	4.4	-	5.8	5.9
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	855	-	277	700
Stage 1	-	-	-	-	816	-
Stage 2	-	-	-	-	577	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	855	-	263	700
Mov Cap-2 Maneuver	_	-	-	_	412	-
Stage 1	_	_	_	-	816	-
Stage 2	_	_	_	_	548	_
J.W. 2					0.10	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.6		10.7	
HCM LOS					В	
Minor Lang/Major Mumi		IDI 51	EDT	EDD	\MDI	WPT
Minor Lane/Major Mymt	ı l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		680	-	-	855	-
HCM Lane V/C Ratio		0.073	-	-	0.052	-
HCM Control Delay (s)		10.7	-	-	9.4	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.2	-	-	0.2	-

Fiming Plan: PM Peak Hour 3: Old Schoolhouse Lane (West)/Old Schoolhouse Lane & Proposed Site Driveway (West)

Tilling Flan: Fluit					•		
	۶	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	✓	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્ન	f)		¥		
Traffic Volume (vph)	19	23	23	22	12	24	
Future Volume (vph)	19	23	23	22	12	24	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	2021	1897	0	1666	0	
Flt Permitted		0.978			0.984		
Satd. Flow (perm)	0	2021	1897	0	1666	0	
Link Speed (mph)		25	30		30		
Link Distance (ft)		948	144		107		
Travel Time (s)		25.9	3.3		2.4		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	47	50	0	40	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized	t						
Intersection Capacity Utiliz	ation 18.9%			IC	CU Level c	of Service A	4
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	3.8				·	
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	f)		W	
Traffic Vol, veh/h	19	23	23	22	12	24
Future Vol, veh/h	19	23	23	22	12	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	
Storage Length	_	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	21	26	26	24	13	27
						<u>=</u> :
	Major1		/lajor2		Minor2	
Conflicting Flow All	50	0	-	0	106	38
Stage 1	-	-	-	-	38	-
Stage 2	-	-	-	-	68	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1154	-	-	-	1038	1105
Stage 1	-	-	-	-	1151	-
Stage 2	-	-	-	-	1114	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1154	-	-	-	1019	1105
Mov Cap-2 Maneuver	-	-	-	-	1019	-
Stage 1	-	-	-	-	1130	-
Stage 2	-	-	-	-	1114	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	3.7		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1154	-	_		1075
HCM Lane V/C Ratio		0.018	-	-		0.037
HCM Control Delay (s)		8.2	0	-	_	8.5
HCM Lane LOS		A	A	-	_	A
HCM 95th %tile Q(veh)		0.1	-	-	-	0.1
		<b>J</b> .,				<b>J</b> .,

iming Plan: PM Peak Hour 4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

Tilling Flant. Flyi	Cak Hou	1	0.0	20110011100	add Lario,	014 001100	mode Lane (Last) at reposed one Driveway (Last)
	۶	<b>→</b>	<b>←</b>	•	<b>&gt;</b>	4	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4	ĵ∍		W		
Traffic Volume (vph)	0	35	41	3	11	4	
Future Volume (vph)	0	35	41	3	11	4	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Satd. Flow (prot)	0	2041	1944	0	1735	0	
Flt Permitted					0.964		
Satd. Flow (perm)	0	2041	1944	0	1735	0	
Link Speed (mph)		25	25		25		
Link Distance (ft)		144	476		112		
Travel Time (s)		3.9	13.0		3.1		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	39	49	0	16	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	d						
Intersection Capacity Utiliz	zation 13.3%			IC	CU Level of	of Service A	4
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL			אטא		אמט
Lane Configurations	0	<del>વ</del>	<b>}</b>	2	<b>\</b>	A
Traffic Vol, veh/h	0	35	41	3	11	4
Future Vol, veh/h	0	35	41	3	11	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	0	39	46	3	12	4
	_					
		_				
Major/Minor N	/lajor1	N	//ajor2	N	Minor2	
Conflicting Flow All	49	0	-	0	87	48
Stage 1	-	-	-	-	48	-
Stage 2	-	-	-	-	39	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	_	-	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	3	_	_	_	3	3.1
Pot Cap-1 Maneuver	1155	_	_	_	1065	1091
Stage 1	-	_	_	<u>-</u>	1139	-
Stage 2	_	_	_	_	1150	_
	-				1130	-
Platoon blocked, %	4455	-	-	-	4005	1001
Mov Cap-1 Maneuver	1155	-	-	-	1065	1091
Mov Cap-2 Maneuver	-	-	-	-	1065	-
Stage 1	-	-	-	-	1139	-
Stage 2	-	-	-	-	1150	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.4	
HCM LOS	U		U		0.4 A	
HCIVI LOS					А	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1155	_	-		1072
HCM Lane V/C Ratio		-	_	_		0.016
HCM Control Delay (s)		0	_	_		8.4
HCM Lane LOS		A	_	_	_	Α
		Α.	-		-	
HCM 95th %tile Q(veh)		0	_	_	_	0

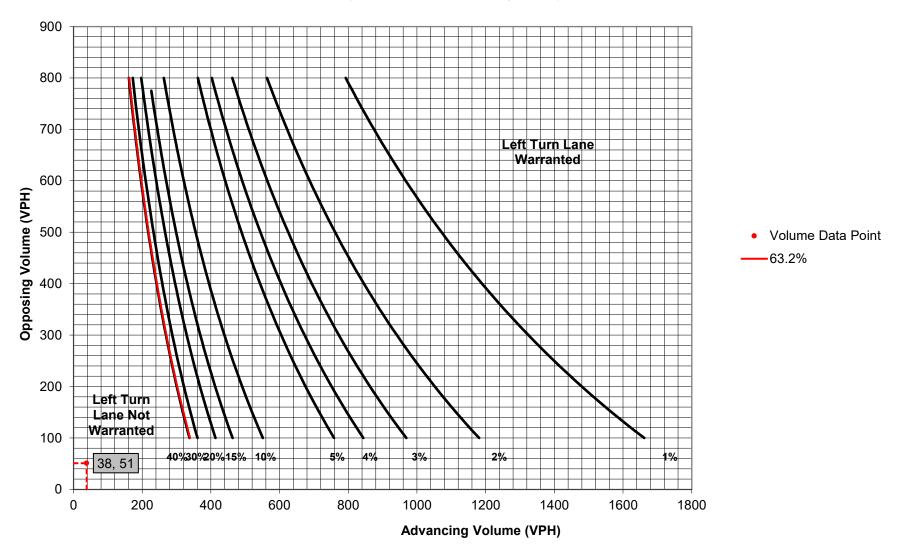
# **Appendix F:**Auxiliary Turn Lane Warrant Analysis

		S <sup>-</sup>	TUDY LOC	ATION A	ND ANALY	SIS INFORM	1ATION				
	Mur	nicipality:		en Township		Analysis		11/17/			
		County:	Cumberl	and County		Conduct		EN			
PennDOT E	ngineerin	g District:		8		Check		TP TP			
					A	ency/Company I	vame:	IP	ט		
Intersection & App	proach De	scription: O	old Schoolhous	se Lane & Pro	posed Site Dri	reway (West) - Ea	astbound Le	ft			
		is Period:	2035 Projected (Build)			Number of Approach Lanes: 1					
		ign Hour:		eak Hour		Undivided o	r Divided H	ighway:	Undivided		
	tersection			gnalized				_			
Posted :	Speed Lim	f Terrain:		25 olling		Left or Right-T	urn Lane Ar		pe of Analys		
	Турс о	· remain.	, and a second	_	E CALCULA		um Lane Ai	larysis:	Cit ruin Lan	•	
					E CALCULA						
			L	.eft Turn La	ne Volume C	alculations					
Movement		Include?	Volume	% Trucks							
	Left	Yes	23	2.0%	24			dvancing Volu		38	
Advancing	Through	-	12	9.0%	14			pposing Volu		51	
	Right	No			N/A		L	eft Turn Volu	me:	24	
	Left	No			N/A						
Opposing	Through Right	- Yes	26 24	0.0% 2.0%	26 25	% Left	· Turns in Ac	dvancing Volu	ma: 6:	3.16%	
	Nigitt	163			ane Volume		i Turris III Ac	avancing void		5.1070	
						,					
Movement	-	Include?	Volume	% Trucks							
	Left	Yes			N/A						
Advancing	Through Right	-			N/A N/A			dvancing Volu		N/A N/A	
	Nigiit				IN/A		KI,	ght Turn Volu	ime:	IN/A	
			TU	RN LANE	WARRAN	<b>FINDINGS</b>					
Left	t Turn La	ne Warrar	nt Findings			Righ	nt Turn Lar	ne Warrant F	indings		
Applicable V	Varrant F	igure:	Figure 1			Applicable Warrant Figure: N/A					
,	Warrant I	Met?:	No				Warrant M	et?: N	I/A		
			TUR	N LANE L	ENGTH CA	LCULATION	S				
l n	torcostion	Control	Uncignalia	and							
				.eu							
-		_									
•	er Hour (If		40		Average	# of Vehicles/Cyc	le:	N/A			
-		- <u>-</u>		Donn DOT D	ublication 46	vhihit 11 G			_		
				י ביוווטטו אנ	-	eed (MPH)					
	<u> </u>	-£ T£5' - ^		25-35		40-45	5	0-60	1		
	Type	or Traffic Con	itroi		Turn D	emand Volume					
			High			Low	High	Low	_		
		Signalized	A	A	B or C	B or C	B or C	B or C	4		
		ınsıgnalized	A	A	C	В	B or C	В	_		
				Left Turn	Lane Storage	Length, Condit	ion A:	N/A	Feet		
						•					
						Condit	ion C:		Feet		
				Requi	ired Left Turn	Lane Storage Le	ength:	N/A	Feet		
							Additio	onal Findings			
								N/A	A		
Additional Comments	s / Justificat	tions:									
Design Hour Volun Cycles Pe	er Hour (H	ing Lane: ssumed): f Known):  of Traffic Con Signalized	Unsignaliz 24 Known 40  http://doi.org/10.1001	PennDOT Pu 25-35  Low A A Left Turn	Average ublication 46, E Sp Turn D High B or C C	# of Vehicles/Cyd xhibit 11-6 eed (MPH) 40-45 emand Volume	High B or C B or C ion A: ion B: ion C: ength:	Does not	Feet Feet Feet		



11/17/2023 Dwy - AM - EB LTL.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

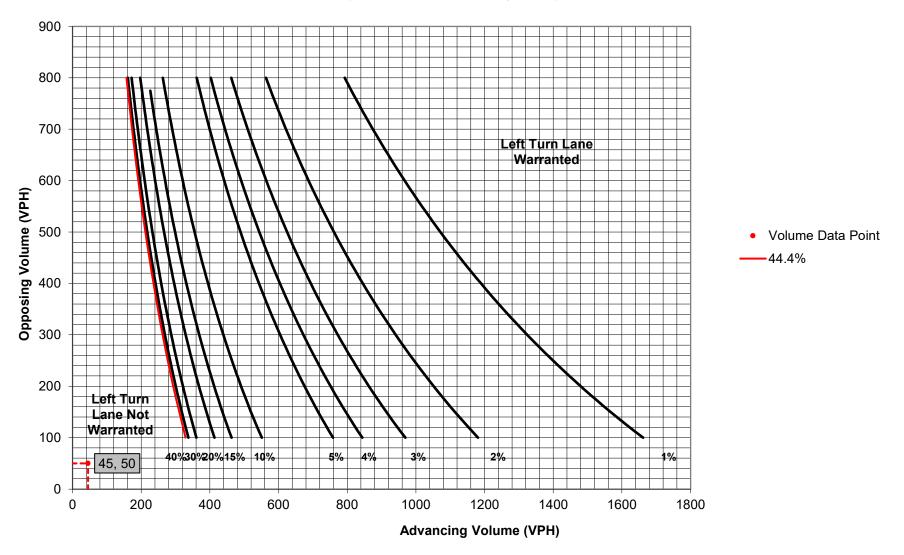


		ST	UDY LOCA	ATION AN	ND ANALY	SIS INFORN	MATION			
	Mur	nicipality:		n Township		Analysis		11/17/		
		County:	Cumberla			Conduct		EN		
PennDOT Er	ngineerin	g District:	8	3			ed By:	TP		
					Ag	ency/Company	Name:	TP	D	
Intersection & App	roach De	scription: Ol	d Schoolhouse	Lane & Prop	osed Site Driv	eway (West) - E	astbound Le	ft		
	-	is Period:		cted (Build)		Number of Approach Lanes: 1				
		ign Hour:		ak Hour		Undivided o	r Divided Hi	ghway:	Undivided	
	tersection			nalized				- T-	and of Ameliania	
Postea S	peed Lim	f Terrain:		5 ling		Left or Right-T	urn Lana An		pe of Analysis eft Turn Lane	
	туре о	i ieiiaiii.					uiii Laile Aii	lalysis:	ert Turri Larie	
				VOLUME	CALCULA	TIONS				
			Le	eft Turn Lan	ie Volume Ca	lculations				
Movement		Include?	Volume	% Trucks	PCEV					
	Left	Yes	19	2.0%	20		Ac	lvancing Volu		
Advancing	Through	-	23	5.0%	25			pposing Volu		
	Right	No			N/A		L	eft Turn Volu	me: 20	
[ L	Left	No			N/A					
Opposing	Through	- Yes	23	11.0% 2.0%	27 23	0/1-5	. T	l	me: 44.44%	
	Right	res			-		t Turns in Ac	lvancing Volu	me: 44.44%	
			Rig	ght Turn La	ne Volume C	alculations				
Movement		Include?	Volume	% Trucks	PCEV					
l	Left	Yes			N/A					
Advancing	Through	-			N/A N/A			Ivancing Volu		
	Right				IN/A		KI	ght Turn Volu	me: N/A	
			TUR	N LANE	WARRAN1	<b>FINDINGS</b>				
Left	Turn La	ne Warrant	t Findings			Rigl	nt Turn Lar	ne Warrant F	indings	
Applicable W	/arrant Fi	gure: F	igure 1			Applicable \	Varrant Fig	ure: N	I/A	
V	Warrant I	Met?:	No				Warrant M	et?: N	I/A	
			TURN	LANE LE	NGTH CA	.CULATION	S			
							<u> </u>			
Int Design Hour Volum	tersection		Unsignalize 20	d						
	r Hour (A	_	Known							
•	er Hour (If		40		Average #	of Vehicles/Cyc	:le:	N/A	1	
				PennDOT Pul	olication 46, E	hibit 11-6				
				J 5011 UI	<u> </u>	ed (MPH)				
	Type	of Traffic Cont	rol	25-35		40-45	5	0-60		
	1.756	unic cont				mand Volume			4	
		Signalizad	High	Low	High P. or C	Low	High	Low B or C	-	
		Signalized Insignalized	A A	A	B or C	B or C	B or C B or C	Borc	1	
				<del>-                                    </del>	<del>_</del>	<del>-                                    </del>	- 5. 5	•	<b>-</b>   <b>-</b>	
				Left Turn	Lane Storage	Length, Condit	ion A:	N/A	Feet	
						Condit	ion B:	N/A	Feet	
						Condit	ion C	N/A	Feet	
				Requir	ed Left Turn	ane Storage Le	ength:	N/A	Feet	
							Additio	nal Findings		
Additional Comments	/ lustificat	ions:						N/A	<u> </u>	
Additional Comments	, sustilled									



11/17/2023 Dwy - PM - EB LTL.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

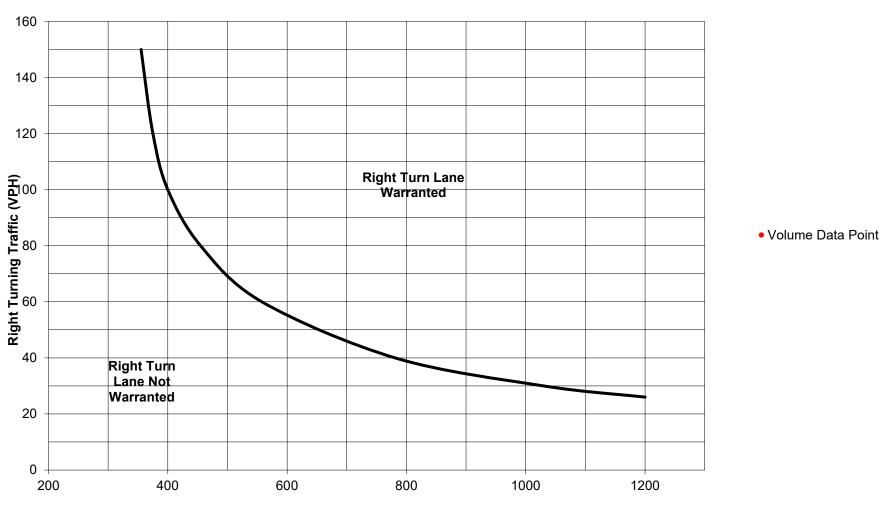


		STI	IDV LOCA	ΔΤΙΩΝ ΔΝ	ΙΟ ΔΝΔΙ Υ	SIS INFORN	ΛΑΤΙΩΝ.			
					- AIVALI			4415-	/2022	
	Mur	nicipality:		n Township		Analysis		11/17, EN		
PennDOT E	ngineerin	County:		nd County 3		Conduct	ed By:	TP		
reliiboi Li	iigiiieeiiii	g District.		<u>,                                      </u>	Ag	ency/Company		TP		
Intersection & App	roach De	scription: Old	Schoolhouse	Lane & Pron	osed Site Driv	away (West) - M	Vesthound R	ight		
intersection & App	noach be	scription. Old	Schoolilouse	calle & FTOP	osed Site Dilv	eway (west) - v	vestbound it	igiit		
		is Period:	2035 Projected (Build)				of Approach		1	
le.	Des tersectior	ign Hour:		ak Hour		Undivided o	or Divided H	ighway:	Undivided	
	Speed Lim			nalized 5				Tv	pe of Analy	reie
. osteu c	•	f Terrain:		ling		Left or Right-1	Turn Lane Ar		ight Turn La	
				VOLUME	CALCULA	TIONS				
				eft Turn Lan						
Movement	: 1	Include?	Volume	% Trucks	PCEV					
	Left	Yes			N/A		Ac	lvancing Volu	ıme:	N/A
Advancing	Through	-			N/A			pposing Volu		N/A
	Right	Yes			N/A		L	eft Turn Volu	ıme:	N/A
	Left	Yes			N/A					
Opposing	Through	- Voc			N/A	% I of	t Turns in As	luancing Valu	ımaı	N/A
Right Yes N/A % Left Turns in Advancing Volume: N/A  Right Turn Lane Volume Calculations										
Movement		Inches 4-2	Volume			uiculatiOH3				
iviovement	Left	Include?	volume	% Trucks	PCEV N/A					
Advancing	Through	-	26	0.0%	26		Ac	dvancing Volu	ıme:	51
	Right	-	24	2.0%	25			ght Turn Volu		25
			THR	NIANEV	NARRANI	FINDINGS				
1.5	T 1 -			III LAIVE			-			
Len	i iurn La	ne Warrant		i		Rig	nt Turn Lar	ne Warrant I		
Applicable W	/arrant Fi	igure:	N/A			Applicable \	Narrant Fig	ure: Fig	ure 9	
•	Warrant I	Met?:	N/A				Warrant M	et?:	No	
			TURN	LANE LE	NGTH CA	CULATION	S			
In	tersection	Control:	Unsignalize	d						
Design Hour Volum	ne of Turn	ing Lane:	25							
	er Hour (A		Known						_	
Cycles Pe	er Hour (If	f Known):	40		Average	of Vehicles/Cy	cle:	N/A		
	_			PennDOT Pub	lication 46, E	chibit 11-6				
						ed (MPH)				
	Туре	of Traffic Contro	ы	25-35		40-45	5	0-60	4	
			High	Low	Turn De	mand Volume Low	High	Low		
		Signalized	A	A	B or C	B or C	B or C	B or C	1	
		Insignalized	А	А	С	В	B or C	В	]	
				Right Turn !	ane Storago	Length, Condit	ion A	N/A	Feet	
				Mgnt Tuill L	une storage	•				
						Condit		N/A	Feet	
						Condit	tion C:	N/A	Feet	
				Required	d Right Turn	Lane Storage L	ength:	N/A	Feet	
							Additio	onal Findings	<u>;:</u>	
Additional Committee	/ luatio	tions:						N/A		
Additional Comments	, Justificat	uons:								



11/17/2023 Dwy - AM - WB RTL.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

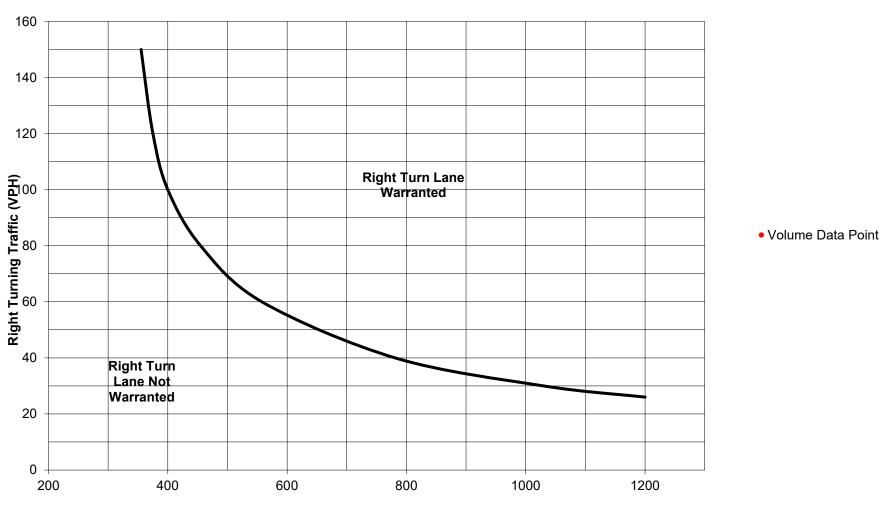


		STI	IDV LOCA	ΔΤΙΩΝ ΔΝ	ΙΟ ΔΝΔΙ Υ	SIS INFORM	ΛΑΤΙΩΝ.				
					ANALI				/2022		
	Mur	nicipality:		n Township nd County		Analysi: Conduct		11/17, EN			
PennDOT E	ngineerin	County:		3			ed By:	TP			
reniiboi Li	igineering	g District.		<u>,                                      </u>	Ag	ency/Company		TP			
Intersection & App	roach De	scription: Old	Schoolhouse	Lane & Pron	osed Site Driv	eway (West) - V	Vesthound R	ight			
intersection & App	il Oacii De	scription. Old	Schoolilouse	calle & FTOP	osed Site Dilv	eway (west) - v	vestbound it	igiit			
		is Period:	2035 Projec			Number of Approach Lanes: 1					
In	Des tersectior	ign Hour:		ak Hour nalized		Undivided (	or Divided H	ighway:	Undivided		
	speed Lim			5				Tv	pe of Analy	rsis	
	•	f Terrain:		ling		Left or Right-1	Turn Lane Ar		ight Turn La		
				VOLUME	CALCULA	TIONS					
					e Volume C						
Movement		Include?	Volume	% Trucks	PCEV						
	Left Yes				N/A		Ad	dvancing Volu	ıme:	N/A	
Advancing	Advancing Through -				N/A			pposing Volu		N/A	
	Right	Yes			N/A		I	eft Turn Volu	ıme:	N/A	
	Left	Yes			N/A						
Opposing	Through Right	- Ves			N/A N/A	% l of	t Turns in A	lvancing Volu	ıme.	N/A	
L	Right Yes N/A % Left Turns in Advancing Volume: N/A  Right Turn Lane Volume Calculations										
Movement		Include?	Volume	% Trucks	PCEV	aioaiationo					
Wovement	Left	No	volume	% ITUCKS	N/A						
Advancing	Through	-	23	11.0%	27		Ad	dvancing Volu	ıme:	50	
	Right	-	22	2.0%	23			ght Turn Volu		23	
			THR	NIANEV	NARRAN1	FINDINGS					
1.50	T 1 -			III LAIVE			-4 T I				
Lett	iurn La	ne Warrant		1		Rig	nt Turn Lai	ne Warrant I	rinaings		
Applicable W	arrant F	igure:	N/A			Applicable \	Narrant Fig	ure: Fig	ure 9		
1	Narrant I	Met?:	N/A				Warrant M	et?:	No		
			TURN	LANE LE	NGTH CA	LCULATION	S				
In	tersection	Control:	Unsignalize	d							
Design Hour Volum	ne of Turn	ing Lane:	23								
		ssumed):	Known						_		
Cycles Pe	er Hour (If	f Known):	40		Average	of Vehicles/Cy	cle:	N/A			
			ı	PennDOT Pub	lication 46, E	khibit 11-6					
					Sp	eed (MPH)					
	Туре	of Traffic Contro	ol	25-35		40-45	5	0-60			
			High	Low	High	emand Volume Low	High	Low	-		
		Signalized	A	A	B or C	B or C	B or C	B or C	1		
		Insignalized	А	А	С	В	B or C	В	]		
				Right Turn !	ano Storaca	Length, Condit	ion A:	N/A	Feet		
				NIGHT FURN L	ane storage	•					
						Condi	ion B:	N/A	Feet		
						Condi	tion C:	N/A	Feet		
				Required	d Right Turn	Lane Storage L	ength:	N/A	Feet		
							Additio	onal Findings	<b>-</b> ::		
							Addition	N/A			
Additional Comments	/ Justificat	tions:									



11/17/2023 Dwy - PM - WB RTL.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

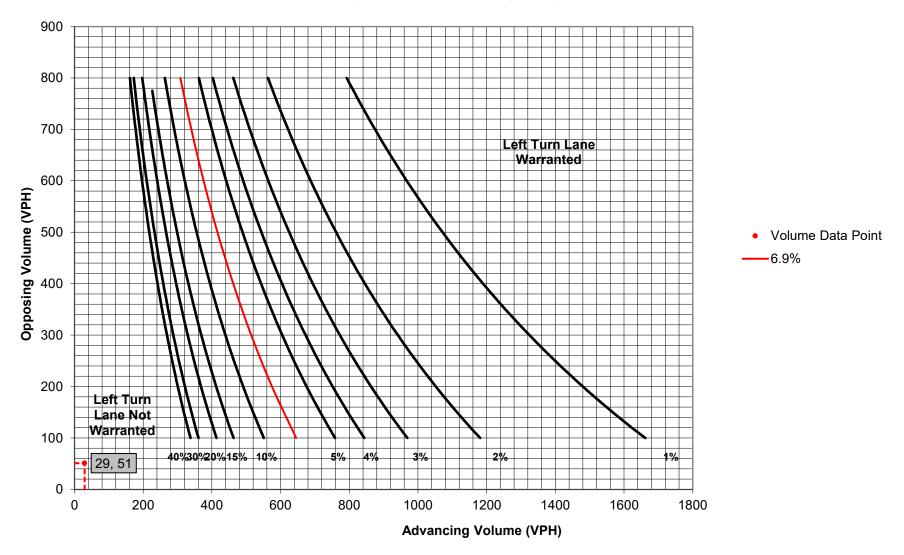


		СТ		ATIONI AN	ID ANALY	SIS INFORM	AATION				
		31			ANALY	SIS INFURIN					
	Mur	nicipality:		n Township		Analysis		11/17/			
		County:		nd County		Conduct		EN			
PennDOT E	ngineerin	g District:		3			ed By:	TPI			
					Ag	ency/Company	Name:	TPI	)		
Intersection & App	oroach De	scription: Old	d Schoolhouse	Lane & Propo	osed Site Driv	eway (East)  - Ea	stbound Left				
	Analys	is Period:	2035 Proje	cted (Build)		Number	of Approach	Lanes:	1		
	Des	ign Hour:	AM Pea	ak Hour		Undivided o	r Divided Hi	ghway:	Undivided		
In	tersection	Control:	Unsigr	nalized							
Posted S	Speed Lim	it (MPH):		5					oe of Analysis		
	Туре о	f Terrain:	Rol	ling		Left or Right-T	urn Lane An	alysis?: Le	eft Turn Lane		
				VOLUME	CALCULA	TIONS					
			Le	eft Turn Land	e Volume Ca	alculations					
Movement	t I	Include?	Volume	% Trucks	PCEV						
	Left	Yes	1	2.0%	2		Ad	vancing Volui	me: 29		
Advancing	Through	-	23	9.0%	27			pposing Volui			
	Right	No			N/A			eft Turn Volui			
	Left	No			N/A						
Opposing Through - 47 0.0% 47											
Right Yes 3 2.0% 4 % Left Turns in Advancing Volume:											
			Rig	ght Turn Lan	e Volume C	alculations					
Movement	t	Include?	Volume	% Trucks	PCEV						
	Left	Yes			N/A						
Advancing	Through	-			N/A			vancing Volu			
	Right	-			N/A		Ri	ght Turn Volui	me: N/A		
			TUR	N LANE V	VARRAN1	FINDINGS					
Left	t Turn La	ne Warrant	Findings			Rial	nt Turn Lar	e Warrant F	indings		
Applicable W			igure 1			Applicable V			/A		
	Warrant I		No				Warrant M		/A		
									,,, <u>,</u>		
			TURN	I LANE LEI	NGTH CA	CULATION	S				
	tersection		Unsignalize	d							
Design Hour Volun	ne of Turn	ing Lane:	2								
•	er Hour (A		Known						<b>-</b>		
Cycles Po	er Hour (If	Known):	40		Average #	of Vehicles/Cyc	cle:	N/A			
			ı	PennDOT Pub					-		
				25-35	Spe	eed (MPH) 40-45	5	0-60	-		
	Туре	of Traffic Conti	rol		Turn De	mand Volume	ı		1		
			High	Low	High	Low	High	Low			
		Signalized	А	А	B or C	B or C	B or C	B or C			
	L	Insignalized	A	A	С	В	B or C	В	_		
				Left Turn L	ane Storage	Length, Condit	ion A:	N/A	Feet		
						Condit		N/A	Feet		
				_		Condit		N/A	Feet		
				Require	ed Left Turn	Lane Storage Le	ength:	N/A	Feet		
							Additio	nal Findings:			
								N/A			
Additional Comments	/ Justificat	tions:							1		



11/17/2023 Dwy - AM - EB LTL.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

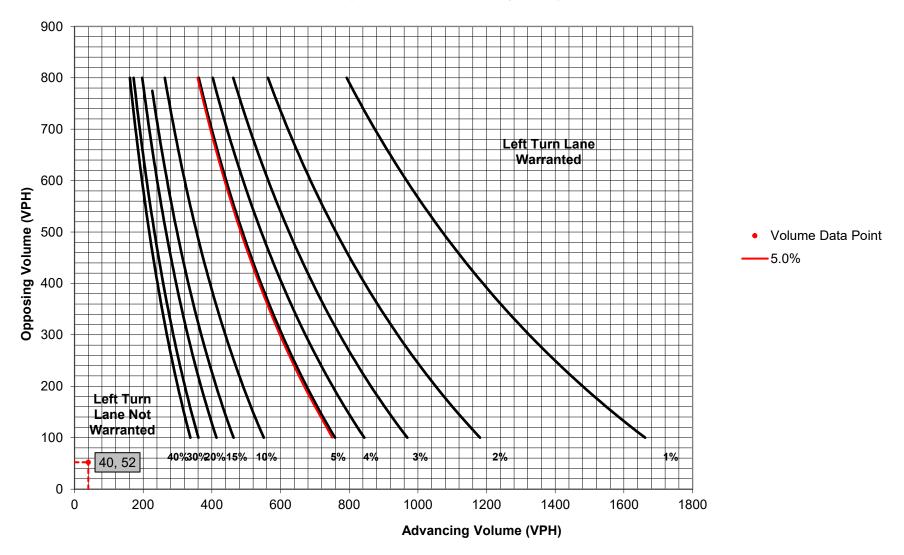


		STU	JDY LOC	ATION AN	ID ANALY	SIS INFORN	/ATION				
	p.4.			n Township				11/17,	/2022		
	iviur	nicipality: County:		nd County		Analysis Conduct		EN			
PennDOT En	ngineering			3			ed By:	TP			
					Ag	ency/Company		TP	D		
Intersection & App	roach Des	scription: Old	Schoolhouse	Lane & Prop	osed Site Driv	eway (East) - Ea	stbound Left	i			
	Analys	is Period:	2035 Proje	cted (Build)		Number	of Approach	Lanes:	1		
		ign Hour:		ak Hour		Undivided o	or Divided H	ighway:	Undivide	d	
		Control:		nalized					of Ana	lunia	
Posteu 3	•	it (MPH): f Terrain:		5 ling		Left or Right-1	Turn Lane Ar		pe of Ana eft Turn La		
	•				CALCULA			•			
					e Volume C						
Movement		Include?	Volume	% Trucks	PCEV						
ovement	Left	Yes	1	2.0%	2		Ac	dvancing Volu	ıme:	40	
Advancing	Advancing Through -			5.0%	38			pposing Volu		52	
	Right	No			N/A		L	eft Turn Volu	ıme:	2	
	Left	No			N/A						
Opposing	Through	- Yes	41 3	11.0% 2.0%	48	0/1-4	4 T 4 a			5.00%	
	Right	res					t Turns in Ac	dvancing Volu	ime:	3.00%	
					ne Volume C	aiculations					
Movement	Left	Include?	Volume	% Trucks	PCEV						
Advancing	Left Through	Yes -			N/A N/A		۸	lvancing Volu	ıma:	N/A	
Advancing	Right	-			N/A			ght Turn Volu		N/A	
	<u> </u>	·	<b>-</b>		,	FINIDINGS		9		,	
			TUR	IN LANE V	WARRANT	FINDINGS					
Left	Turn La	ne Warrant	Findings			Rig	ht Turn Lar	ne Warrant F	Findings		
Applicable W	arrant Fi	igure: Fig	gure 1			Applicable Warrant Figure: N/A					
V	Varrant I	Met?:	No				Warrant M	et?: N	I/A		
			TURN	LANE LE	NGTH CA	CULATION	S				
Int	tersection	Control:	Unsignalize	d							
Design Hour Volum			2								
Cycles Pe			Known						_		
Cycles Pe	er Hour (If	f Known):	40		Average	of Vehicles/Cy	cle:	N/A			
				PennDOT Pub	olication 46, E	chibit 11-6					
					Spe	ed (MPH)	,				
	Туре	of Traffic Contro	ol	25-35	T.una D.	40-45	5	0-60	4		
			High	Low	High	mand Volume Low	High	Low			
		Signalized	A	A	B or C	B or C	B or C	B or C	1		
	U	Insignalized	A	А	С	В	B or C	В			
				Left Turn I	ane Storage	Length, Condit	ion A:	N/A	Feet		
				zere ruini t	Jiorage	•					
						Condit		N/A	Feet		
						Condit	tion C:	N/A	Feet		
				Require	ed Left Turn	Lane Storage L	ength:	N/A	Feet		
			_				Additio	onal Findings	<u>:</u>		
Additional Comments	/ luctific	tions.						N/A	A		
Additional Comments	, Justificat	uons:									



11/17/2023 Dwy - PM - EB LTL.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

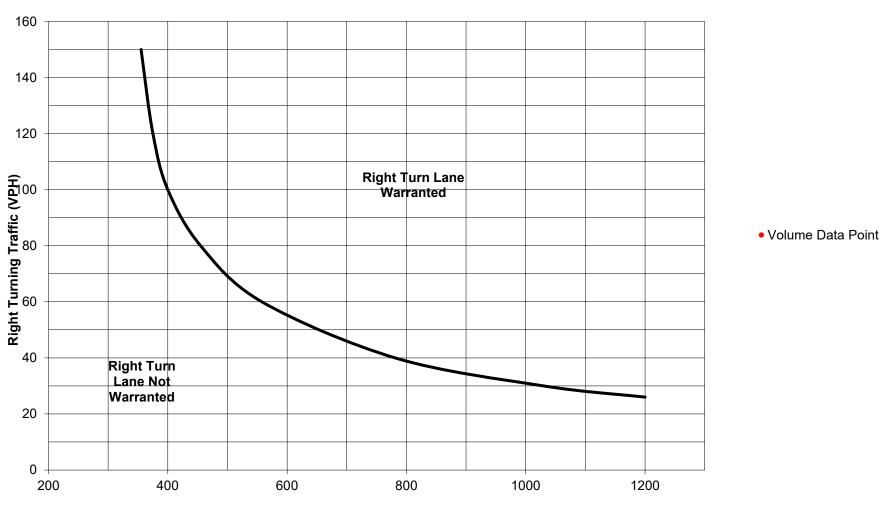


		STI	IDV LOCA	ΔΤΙΩΝ ΔΝ	ΙΟ ΔΝΔΙ Υ	SIS INFORM	ΛΑΤΙΩΝ.				
					ANALI				/2022		
	Mur	nicipality:		n Township		Analysi		11/17, EN			
PennDOT E	ngineering	County:		nd County 8		Conduct	ed By:	TP			
reliiboi Li	igineering	g District.		<u> </u>	Ag	ency/Company		TP			
Intersection & App	roach De	scription: Old	Schoolhouse	lane & Pron	osed Site Driv	eway (Fast) - W	esthound Ric	zht			
intersection & App	ioacii De	scription. Old	Schoolilouse	E Lane & Frop	osed Site Dilv	eway (Last) - w	estbouria Mg	3111			
		is Period:	2035 Projected (Build)				of Approach		1		
le.	Des tersection	ign Hour:		ak Hour		Undivided o	or Divided H	ighway:	Undivided		
	tersection Speed Lim			nalized !5				Tv	pe of Analys	eie	
. osteu c	•	f Terrain:		ling		Left or Right-1	Turn Lane Ar		ight Turn Lar		
				VOLUME	CALCULA	TIONS					
					e Volume C						
Movement	: 1	Include?	Volume	% Trucks	PCEV						
	Left Yes				N/A		Ac	lvancing Volu	ıme:	N/A	
Advancing	Through	-			N/A			pposing Volu		N/A	
	Right	Yes		_	N/A		L	eft Turn Volu	ıme:	N/A	
	Left	Yes			N/A						
Opposing	Through Right	- Ves			N/A N/A	% l of	t Turns in Ac	lvancing Volu	ıma:	N/A	
LL	Right Yes N/A % Left Turns in Advancing Volume: N/A  Right Turn Lane Volume Calculations										
Movement	. 1	Include?	Volume	% Trucks	PCEV	raicaiations					
iviovement	Left	No	volume	% Trucks	N/A						
Advancing	Through	-	47	0.0%	47		Ac	dvancing Volu	ıme:	51	
	Right	-	3	2.0%	4			ght Turn Volu		4	
			THE	NIANEV	NΔRRΔNI	FINDINGS					
1.5	Turna La	no Monant					ht Turre I ar	\A/	Finalin are		
		ne Warrant		1				ne Warrant I			
Applicable W	/arrant Fi	igure:	N/A			Applicable \	Narrant Fig	ure: Fig	ure 9		
1	Narrant I	Met?:	N/A				Warrant M	et?:	No		
			TURN	LANE LE	NGTH CA	LCULATION	S				
In	tersection	Control:	Unsignalize	ed							
Design Hour Volum	ne of Turn	ing Lane:	4								
	er Hour (A		Known								
Cycles Pe	er Hour (If	f Known):	40		Average	f of Vehicles/Cy	cle:	N/A			
				PennDOT Pub	lication 46, E				_		
				25.25	Sp	eed (MPH)		0.50			
	Туре	of Traffic Contro	ol	25-35	Turn Dr	40-45 emand Volume	<u> </u>	0-60	-		
			High	Low	High	Low	High	Low			
		Signalized	A	A	B or C	B or C	B or C	B or C	1		
	U	Insignalized	А	A	С	В	B or C	В			
				Right Turn I	.ane Storage	Length, Condit	ion A:	N/A	Feet		
						Condit		N/A	Feet		
						Condi	tion C:	N/A	Feet		
				Require	d Right Turn	Lane Storage L	ength:	N/A	Feet		
			-				Additio	onal Findings			
Additional Comments	/ luckitic	tions.						N/A	A		
Additional Comments	/ Justificat	uons:									



11/17/2023 Dwy - AM - WB RTL.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



		ST	TUDY LOCA	ATION A	AND	ANALY:	SIS INFORM	IATION			
	Mur	nicipality:	Upper Alle				Analysis		11/17/		
		County:		nd County			Conduct		EN		
PennDOT En	ngineering	g District:		8			Check		TP TP		
						Age	ency/Company I	vame:	IP	D	j
Intersection & App	roach Des	scription: Ol	ld Schoolhouse	e Lane & Pr	roposed	d Site Drive	eway (East) - We	estbound Rig	ght		
		is Period:	2035 Proje		l)			of Approach		1	
Int	es tersection	ign Hour:		ak Hour nalized			Undivided o	r Divided H	ignway:	Undivided	]
	peed Lim			!5					Tv	pe of Analysis	1
	•	f Terrain:		ling			Left or Right-T	urn Lane Ar		ght Turn Lane	
				VOLUM	ΛΕ CA	ALCULA	TIONS				•
							Iculations				
D. 2		In alcordan									
Movement	Left	Include? Yes	Volume	% Truck	.S	PCEV N/A		Λ.	dvancing Volu	me: N/A	
Advancing	Through	-				N/A			pposing Volu		
	Right	Yes				N/A			eft Turn Volu		
	Left	Yes				N/A		_		,,,,	
Opposing	Through	-				N/A					
	Right	Yes									
			Riç	ght Turn L	Lane V	/olume C	alculations				
Movement		Include?	Volume	% Truck	s	PCEV					
Advancing	Left	No -	41	11.0%		N/A 48		Λ.	duancina Valu	me: 52	
Advancing	Through Right	-	3	2.0%		40			dvancing Volu ght Turn Volu		
					- 11/4		EINIDINIOS		B		
				IN LANE	: WA	RKANI	FINDINGS				
Left	Turn La	ne Warran	t Findings				Righ	nt Turn Lar	ne Warrant F	indings	
Applicable W	arrant Fi	igure:	N/A				Applicable V	Varrant Fig	ure: Figu	ure 9	
V	Varrant I	Met?:	N/A				,	Warrant M	et?:	No	
			TURN	I LANE I	LENG	TH CAL	CULATION:	S			
Int	tersection	Control:	Unsignalize	ed							
Design Hour Volum			4								
Cycles Pe	•		Known							-	
Cycles Pe	er Hour (If	Known):	40			Average #	of Vehicles/Cyc	ie:	N/A	J	
			1	PennDOT F	Publica	tion 46, Ex	hibit 11-6			_	
						•	ed (MPH)				
	Туре	of Traffic Cont	trol	25-35			40-45	5	0-60	-	
			High	Lo	w I	High	mand Volume Low	High	Low		
		Signalized	A	A		B or C	B or C	B or C	B or C	†	
		Insignalized	A	А		С	В	B or C	В	]	
				Diaht T		Charren	Lamath Carrier	ion Ar	NI/A	Tr	
				Kignt Tur	n Lane	estorage	Length, Condit		N/A	Feet	
						Condition B:			N/A	Feet	
							Condit	ion C:	N/A	Feet	
				Requi	ired Ri	ght Turn L	ane Storage Le	ength:	N/A	Feet	
								Additio	onal Findings:	<del>-</del> :	
									N/A		
Additional Comments	/ Justificat	tions:									



11/17/2023 Dwy - PM - WB RTL.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)

