







Transportation Impact StudyGenius Kids Child Development Center *Upper Allen Township, Cumberland County*

For Submission To: Upper Allen Township

GENIUS KIDS CHILD DEVELOPMENT CENTER TRANSPORTATION IMPACT STUDY

FOR SUBMISSION TO:

Upper Allen Township, Cumberland County, PA

Prepared For:

BL Companies

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EXECUTIVE SUMMARY

The purpose of this study is to examine the potential traffic impact associated with the proposed Genius Kids Child Development Center along Old Schoolhouse Lane in Upper Allen Township, Cumberland County, PA. Based on this evaluation, the following conclusions were reached:

- 1. This report has been prepared in accordance with PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's *Publication 282*, *Appendix A*, dated September 2022 and with Upper Allen Township's Subdivision and Land Development Ordinance § 220-3.7.F.
- 2. The project scope and the extent of the study area were based on the contents of the TIS Scoping Memo and the associated feedback from representatives of Upper Allen Township. The approved study area intersections included in this TIA are as follows:
 - » Cumberland Parkway & Old Schoolhouse Lane (West);
 - » Cumberland Parkway & Old Schoolhouse Lane (East);
 - » Old Schoolhouse Lane & Proposed Site Driveways.
- 3. The proposed development is located on the northern side of Old Schoolhouse Lane, in the vacant parcel east of the Wylie Dental Group building. The project involves construction of an 8,500 square foot Genius Kids Child Development Center.
- 4. Access to the development is proposed via two (2) full-movement driveway connections to Old Schoolhouse Lane.
- 5. The available sight distances at the proposed site driveways to Old Schoolhouse Lane exceed PennDOT's sight distance criteria.
- 6. Based on ITE trip generation data, the proposed development is anticipated to generate **53 new vehicle-trips** during the weekday A.M. peak hour and **53 new vehicle-trips** during the weekday P.M. peak hour.
- 7. Capacity analyses were conducted to determine the quality of operation (LOS) at the study area intersections for the 2023 existing, 2025 and 2035 base (no-build), and 2025 and 2035 projected (build) conditions. The capacity analyses were conducted in accordance with the standards contained in Appendix A Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT *Publication 282*, dated September 2022.
- 8. Levels of Service (LOS) for the study area intersections have been summarized in matrix form. **Table**I details the overall intersection LOS for each study area intersection.

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TABLE I
OVERALL INTERSECTION LEVEL OF SERVICE SUMMARY

	_	Ov	м					
Intersection	Time Period	Time		dout Year	2035 Horizon Year		Meets LOS Requirements?	
	renou	Existing	Base	Projected	Base	Projected	Requirements:	
Cumberland Parkway &	A.M.	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	YES	
Old Schoolhouse Lane (West)	P.M.	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	YES	
Cumberland Parkway &	A.M.	A (0.3)	A (0.3)	A (0.8)	A (0.3)	A (0.8)	YES	
Old Schoolhouse Lane (East)	P.M.	A (0.4)	A (0.4)	A (0.8)	A (0.4)	A (0.8)	YES	
Old Schoolhouse Lane &	A.M.			A (4.0)		A (3.9)	YES	
Proposed Site Driveway (West)	P.M.			A (3.8)		A (3.8)	YES	
Old Schoolhouse Lane &	A.M.			A (1.3)		A (1.3)	YES	
Proposed Site Driveway (East)	P.M.			A (1.4)		A (1.3)	YES	

Base = No-Build scenario

Projected = Build scenario with development

- Under 2025 and 2035 projected (build) conditions, the study intersection will operate in accordance with the standards contained in Appendix A - Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT *Publication 282*, dated September 2022.
- 10. Traffic Planning and Design Inc. (TPD) recommends the following roadway improvements as outlined at the study area intersections:

Cumberland Parkway & Old Schoolhouse Lane (West)

» No improvements are necessary.

Cumberland Parkway & Old Schoolhouse Lane (East)

» No improvements are necessary.

Old Schoolhouse Lane & Proposed Site Driveways

- » Design the site driveways as full-movement driveways in accordance with Township standards.
- » Provide one entering lane and one exiting lane.
- » Provide a "Stop" sign, (PennDOT designation R1-1) on exiting driveway approaches.
- » Provide and perpetually maintain required sight lines per Township standards.
- » Provide sidewalk along the property frontage to Old Schoolhouse Lane and provide crosswalks and ADA ramps at the proposed site driveways.

As part of the Township's Land Development process, the applicant will coordinate and fund the implementation of the recommended roadway improvements. Additionally, all improvements will be constructed to accommodate non-motorized access/circulation and be ADA-compliant unless otherwise approved by the Township.

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INTRODUCTION

Traffic Planning and Design, Inc. (TPD) has completed a Transportation Impact Study (TIS) for the proposed Genius Kids Child Development Center in Upper Allen Township, Cumberland County, Pennsylvania. As shown in **Figure 1**, the proposed development is located on the northern side of Old Schoolhouse Lane, in the vacant parcel east of the Wylie Dental Group building. As shown in **Figure 2**, the project involves construction of an 8,500 square foot Genius Kids Child Development Center. Access to the development is proposed via two (2) full movement driveways to Old Schoolhouse Lane.

This report has been prepared in accordance with PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's Publication 282, Appendix A, dated September 2022 and in accordance with Upper Allen Township Subdivision and Land Development Ordinance § 220-3.7.F. The project scope and the extent of the study area were based on the contents of the TIS Scoping Memo and the associated feedback from representatives of Upper Allen Township during the scoping process. All relevant correspondence pertaining to this project has been included in **Appendix A**.

EXISTING ROADWAY NETWORK

A field review of the existing roadway system in the study area was conducted. The existing roadway characteristics within the study area are summarized in **Table 1**.

TABLE 1
ROADWAY CHARACTERISTICS WITHIN STUDY AREA

Roadway	Ownership	Ownership Functional Classification/ Roadway Type		Posted Speed Limit
Cumberland Parkway	Township	Minor Collector	East-West	35 mph
Old Schoolhouse Lane	Township	Local Road	East-West	25 mph

Details of the existing intersection controls, lane configurations, lane widths, and approach grades, as well as photographs of the study area intersections are included in **Appendix B**.

Land Use Context

In Publication 10X (Design Manual Part IX, Appendix B), there is guidance pertaining to defining the land use context(s) for a given area. Based upon review of this information, the land uses surrounding the proposed site best fits the **Suburban Neighborhood** designation, as described below:

Suburban Neighborhood, "Predominantly low-density residential communities with houses typically arranged along a curvilinear system of streets with limited connectivity to regional road networks. Neighborhoods can include community facilities (schools, churches, recreation) and some small businesses or offices."

Roadway Type

In Section 1.2 of the Design Manual, Part 2, there is guidance pertaining to defining the transportation context(s) for a given area. Comparing the existing condition roadway characteristics to the various options presented in Table 1.2 of the Design Manual, Part 2, the study area roadways best fit the following categories, as described below:

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Neighborhood Collector, traffic volumes of 6,000 or less vehicles per day, intersection spacing of 300 to 660 feet, a desired operating speed of 25-35 mph, and a description as follows: "Often similar in appearance to local roadways. Typically classified as "Minor Collector".""

» Cumberland Parkway

Local Road, traffic volumes of 3,000 or less vehicles per day, intersection spacing of 200 to 660 feet, and a desired operating speed of 20-30 mph.

» Old Schoolhouse Lane

Bicycle and Pedestrian Facilities

Based on observations during field visits at the study area, paved shoulders and/or travel lanes currently accommodate pedestrian and bicycle traffic in the vicinity of the development. As part of the proposed development, sidewalks are to be provided along the property frontage of Old Schoolhouse Lane and crosswalks and ADA ramps are to be provided at the proposed site driveways.

Mass Transit Facilities

Bus service in Cumberland County is provided via Rabit Transit, there are no bus stops in the study area. However, Winding Hill Park and Ride, served by route 15N, exists within a mile to the east of the proposed site and provides service between Harrisburg and Gettysburg.

EXISTING TRAFFIC CONDITIONS

Manual Turning Movement Counts

Manual traffic counts were conducted on 15-minute intervals during the weekday morning (6:00 to 9:00 A.M.) and weekday evening (3:00 to 6:00 P.M.) peak periods. Data pertaining to heavy vehicles, pedestrians and transit vehicles were observed during the manual counts. Peak hours and count dates for the study area intersections are identified in **Table 2**.

TABLE 2
MANUAL TRAFFIC COUNT INFORMATION

Intersection	Date of Traffic Counts	Time Period	Intersection Peak Hour ¹
Cumberland Parkway & Old Schoolhouse Lane (West)	Thursday October 12, 2023	Weekday A.M.	7:30 to 8:30 A.M.
	Thursday October 12, 2025	Weekday P.M.	4:15 to 5:15 P.M.
Cumberland Parkway & Old Schoolhouse Lane (East)	Thursday October 12, 2022	Weekday A.M.	7:30 to 8:30 A.M.
	Thursday October 12, 2023	Weekday P.M.	4:30 to 5:30 P.M.
Old Schoolhouse Lane &	Thursday Ostahay 12, 2022	Weekday A.M.	7:45 to 8:45 A.M.
Proposed Site Driveways	Thursday October 12, 2023	Weekday P.M.	4:30 to 5:30 P.M.

¹Peak Hour consists of the four consecutive 15-minute intervals where the highest traffic volumes occur.

The existing condition traffic volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 3**. Manual traffic count data sheets are contained in **Appendix C**.

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BASE (NO-BUILD) CONDITIONS

Annual Background Growth

A background growth factor for the roadways in the study area was developed based on growth factors obtained from the PennDOT Bureau of Planning and Research (BPR) for September 2023 to July 2024. The PennDOT BPR suggests using a background growth trend factor of 0.54% per year in Cumberland County for urban non-interstate roadways. As such, the background growth factor was applied annually to yield an overall growth percentage of 1.01% (0.54 per year for 2 years) for 2025 opening year and of 1.07% (0.54 per year for 12 years) for 2035 horizon year.

Nearby Proposed Developments

No background developments were identified during the scoping process.

Base condition traffic volumes for the opening year 2025 and horizon year 2035 conditions for the weekday A.M. and P.M. peak hours are shown in **Figures 4 and 5**, respectively. Traffic volume development worksheets are contained in **Appendix D**.

SCHEDULED ROADWAY IMPROVEMENTS

Programmed Improvements

There are no proposed improvement projects at the study intersection.

Nearby Development Improvements

No background developments and/or improvements were identified during the scoping process.

PROPOSED SITE ACCESS

Access to the development is proposed via two (2) full-movement driveway connections to Old Schoolhouse Lane.

Sight Distance Analysis

A sight distance analysis was prepared for the proposed site driveway locations. In general, recommended safe sight distances depend upon the posted speed limit and roadway grades. The existing sight distances at the proposed driveway were measured in accordance with PennDOT Publication 282 <u>Highway Occupancy Permit Guidelines</u> and compared to PennDOT's and the Township's safe stopping sight distance standard, which is calculated by the following equation:

$SSSD = 1.47VT + V^2/[30(f\pm q)]$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Vehicle Speed

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements

g = Percent of Roadway Grade Divided by 100

Table 3 shows the measured and PennDOT's/Township's acceptable sight distances (SSSD) for vehicles entering and exiting the site.

TABLE 3 SIGHT DISTANCE ANALYSIS

		Posted		Sight Distances (feet)	
Movement	Direction	Speed (mph)	Grade ¹	SSSD	EXIST
	Old Schoolhouse Lane & Proposed	d Site Access	(West)		
Exiting	To the left	25	-1%	148′	250′
Movements	To the right	25	+1%	145′	500′+
Entering Left	Approaching same direction	25	+1%	145′	500′+
Turns	Approaching opposite direction	25	-1%	148′	280′
	Old Schoolhouse Lane & Propose	d Site Acces	s (East)		
Exiting	To the left	25	-1%	148′	165′
Movements	To the right	25	+1%	145′	500′+
Entering Left	Approaching same direction	25	+1%	145′	500′+
Turns	Approaching opposite direction	25	-1%	148′	165′

EXIST = Existing (measured) Sight Distance SSSD = PennDOT & Township Safe Stopping Sight Distance
1 = Roadway Grade Approaching Access

As shown in **Table 3**, the measured sight distances at the study intersection exceed applicable PennDOT and Township safe stopping sight distance (SSSD). Sight distance measurements are provided in **Appendix B**.

TRIP GENERATION

The trip generation rates for the proposed development were obtained from the manual *Trip Generation*, 11th Edition, an Institute of Transportation Engineers (ITE) Informational Report. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

The data for Land Use Code 565 (Day Care Center) was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour.

The following should be noted with respect to the trip generation methodology:

- » Based on the information provided in the manual *Trip Generation*, not all of the trips generated by the site will be "new" to the nearby roadway system. In addition to the "new trips" generated by the development, there will be "pass-by trips", which are trips that are drawn from the passing traffic stream and do not add trips to the adjacent roadways. Pass-by trip percentages were utilized as specified in the appendices of the manual *Trip Generation*, Eleventh Edition, 2021.
- Published data related to pass-by percentages are not available for Land Use Code #565 (Day Care Center) during the weekday A.M. peak hour. However, given the trip characteristics of a typical Day Care Center with parent drop-off/pick-up to and from there place of employment, etc. it is reasonable to assume that pass-by trips will occur during the weekday A.M. peak hour as well. Since ITE provides a weekday P.M. pass-by rate of 44% it is reasonable to assume that pass-by trips occur at the same rate during the weekday A.M. peak hour.

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Table 4 shows the trip generation rates and directional split for the analyzed time periods.

TABLE 4
ITE TRIP GENERATION DATA

Land Use	ITE#	Time Period	Equation	Entering %	Exiting %	Pass-By %
		Average Weekday	T = 47.62*(X)	50%	50%	
Day Care Center 565	Weekday A.M. Peak Hour	T = 11.00*(X)	53%	47%	44% ¹	
		Weekday P.M. Peak Hour	T = 11.12*(X)	47%	53%	44%

T = number of site-generated vehicular trips
 X = independent variable (ksf of Gross Floor Area)
 1 = ITE pass-by rates not published. See TPD justification above.

The calculated trip generation for the proposed development for the full build-out year is shown in **Table 5**.

TABLE 5
TRIP GENERATION SUMMARY

	Total Trips		Pass-by Trips			New Trips			
Time Period	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit
Average Weekday	405	202	203						
Weekday A.M. Peak Hour	94	50	44	41	22	19	53	28	25
Weekday P.M. Peak Hour	95	44	51	42	20	22	53	24	29

Based on the trip generation analysis summarized in **Table 5**, the proposed development will generate approximately 53 new trips during the weekday A.M. peak hour and 53 new trips during the weekday P.M. peak hour at full build-out. Detailed trip generation calculations are contained in the approved scoping application provided in **Appendix A**.

TRIP DISTRIBUTION

New Trips

Trip distributions were determined based on existing traffic patterns within the study area, the most logical route of travel, and the location/configuration of the proposed site driveways. The new trips for the proposed development were distributed to the local roadway network based on the percentages shown in **Table 6** and presented graphically in **Figure 6A**.

TABLE 6
TRIP DISTRIBUTION PERCENTAGES

Direction - To/From	Assignment (To/From)	Distribution Percentage
East	via Cumberland Parkway	51%
West	via Cumberland Parkway	49%

Pass-By Trips

Pass-by trips for the /retail component of the proposed development were based on the existing traffic patterns in the vicinity of the site and the location and configuration of the site driveways. The pass-by trips for the proposed development were distributed to the local roadway network based on the percentages illustrated in **Figure 7A**.

Calculations for the proposed site trips are provided within the approved scoping memo and associated attachments in **Appendix A**. The distribution and assignment of site-generated trips for the proposed development during the weekday A.M. and weekday P.M. peak hours are shown in **Figures 6B, 7B,** and **8** for primary (new) trips, pass-by trips, and total development trips, respectively.

PROJECTED (BUILD) CONDITION TRAFFIC VOLUMES

The site-generated trips for the proposed development were added to the 2025 and 2035 base (no-build) condition traffic volumes to develop 2025 and 2035 projected (build) condition traffic volumes. Projected condition traffic volumes for full build-out opening year 2025 and horizon year 2035 for the weekday A.M. and weekday P.M. peak hours are shown in **Figures 9** and **10**. Traffic volume development worksheets are contained in **Appendix D**.

LEVELS OF SERVICE FOR AN INTERSECTION

For analysis of intersections, level of service is defined in terms of delay, which is a measure of driver discomfort and frustration, fuel consumption, and lost travel time. LOS criteria is stated in terms of control delay per vehicle for a one-hour analysis period. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The criteria are shown in **Table 7**. Delay, as it relates to level of service, is a complex measure and is dependent upon several variables. For signalized intersections, these variables include the quality of vehicle progression, the cycle length, the green time ratio, and the volume/capacity ratio for the lane group in question. For unsignalized intersections, delay is related to the availability of gaps in the flow of traffic on the major street and the driver's discretion in selecting an appropriate gap for a particular movement from the minor street (straight across, left or right turn).

TABLE 7

LEVEL OF SERVICE CRITERIA

UNSIGNALIZED AND SIGNALIZED INTERSECTIONS 1

Level of Service	Control Delay Per Vehicle (Seconds)					
Level of Service	Signalized	Unsignalized				
А	< 10	< 10				
В	> 10 and < 20	> 10 and < 15				
С	> 20 and < 35	> 15 and < 25				
D	> 35 and < 55	> 25 and < 35				
E	> 55 and < 80	> 35 and < 50				
F	> 80 or v/c > 1.0	> 50 or v/c > 1.0				

1 = Obtained from Exhibits 19-8 and 20-2 of the Transportation Research Board's Highway Capacity Manual 6th Edition

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CAPACITY ANALYSIS METHODOLOGY

Capacity analyses were conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the 6th edition *Highway Capacity Manual* (HCM) using *Synchro 11* software (Version 11.1, Build 2, Revision 9), a CUBIC/Trafficware product.

The following conditions were analyzed, as applicable:

- » Existing conditions;
- 2025 Base conditions (Build-out year without development);
- 2025 Projected conditions (Build-out year with development);
- 2035 Base conditions (Analysis year without development);
- 2035 Projected conditions (Analysis year with development);

The following should be noted with respect to the capacity analyses:

- » The Pennsylvania default values for two-way stop controlled intersections in a suburban land use context contained in Chapter 10 of PennDOT's *Publication 46* were utilized for the base critical headway and base follow-up headways. Worksheets related to the calculated critical and follow-up headways are included at the beginning of **Appendix E**.
- Per PennDOT standards, a heavy vehicle percentage of 2% was utilized for all turning movements to/from the proposed site access locations.

The capacity analysis worksheets are included in **Appendix E**.

PennDOT Standards

PennDOT's Transportation Impact Study Guidelines outlined in PennDOT's *Policies and Procedures for Transportation Impact Studies*, found in PennDOT's Publication 282, Appendix A, dated September 2022 contain the following criteria regarding levels of service:

- Page 32 of the Guidelines state that if evaluation of the With Development Horizon Year Scenario to the Without Development Horizon Year Scenario indicates that the overall intersection level of service has dropped, the applicant will be required to mitigate the level of service if the increase in overall intersection delay is greater than 10-seconds. If the overall intersection delay increase is less than or equal to 10-seconds, mitigation of the intersection will not be required. If the intersection level of service meets the level of service requirements, applicants may still be required to provide mitigation to address critical lanes or approaches. For locations where the level of service of the design horizon year without the development is LOS F and with development, the delay increases more than 10 seconds, the remedies shall provide an estimated delay which will be no worse than the delay for the design year without the development.
- Page 33 of the Guidelines state that for mitigation scenarios, applicants are expected to mitigate the overall intersection LOS to the original Without Development LOS; the 10-second delay variance is not applied to mitigation scenarios. Applicants may be required to address available storage and queue lengths at critical movements or approaches even if the overall LOS requirements are met.
- » Page 34 of the Guidelines state that if signalization is the preferred alternative for mitigation, overall intersection LOS C in rural areas and LOS D in urban areas is acceptable.

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Page 35 of the Guidelines states new signalized or unsignalized intersections established to serve as access to the development shall be designed to operate at minimum LOS C for rural areas, and minimum LOS D for urban areas.

Upper Allen Township Standards

Upper Allen Township's Transportation Impact Study Guidelines outlined in Upper Allen Township's Subdivision and Land Development Ordinance § 220-3.7.F contain the following criteria regarding levels of service:

Section (2)[c] of the Guidelines state that the study should assess impacts to adjacent roadways and identify improvements that may be required to maintain a level of service rating of D on the affected roadway network. Level of service E or F indicate the study should identify specific improvements to restore LOS to a rating of D or better.

LEVELS OF SERVICE IN THE STUDY AREA

Level of service (LOS) matrices for the study area intersections are shown in **Table 8** for the weekday A.M. and **Table 9** for the weekday P.M. peak hours.

TABLE 8
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY – AM PEAK HOUR

LEVEL OF SERVICE DEEM (SECONDS) SOMMAN AND EMPTOR							
to a constant	Maranasi	Novement 2023 Existing	2025 Ope	ening Year	2035 Horizon Year		
Intersection	Movement		Base	Projected	Base	Projected	
Cumberland Parkway &	WB L/T	Α	А	Α	Α	А	
Old Schoolhouse Lane	NB L/R	В	В	В	В	В	
(West)	ILOS	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)	
Cumberland Parkway &	WB L/T	А	А	А	А	А	
Old Schoolhouse Lane	NB L/R	В	В	В	В	В	
(East)	ILOS	A (0.3)	A (0.3)	A (0.8)	A (0.3)	A (0.8)	
Old Schoolhouse Lane &	EB L/T			Α		А	
Proposed Site Driveway	SB L/R			А		А	
(West)	ILOS			A (4.0)		A (3.9)	
Old Schoolhouse Lane & Proposed Site Driveway	EB L/T			А		А	
	SB L/R			Α		А	
(East)	ILOS			A (1.3)		A (1.3)	

Base = No-Build scenario, Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

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TABLE 9
LEVEL OF SERVICE DELAY (SECONDS) SUMMARY – PM PEAK HOUR

Intersection	Movement 2023		2025 Opening Year		2035 Horizon Year	
intersection	Movement	Existing	Base	Projected	Base	Projected
Cumberland Parkway &	WB L/T	Α	Α	Α	Α	А
Old Schoolhouse Lane	NB L/R	В	В	В	В	В
(West)	ILOS	A (0.3)	A (0.3)	A (0.7)	A (0.3)	A (0.7)
Cumberland Parkway &	WB L/T	Α	Α	Α	Α	А
Old Schoolhouse Lane	NB L/R	В	В	В	В	В
(East)	ILOS	A (0.4)	A (0.4)	A (0.8)	A (0.4)	A (0.8)
Old Schoolhouse Lane &	EB L/T			Α		А
Proposed Site Driveway	SB L/R			Α		А
(West)	ILOS			A (3.8)		A (3.8)
Old Schoolhouse Lane & Proposed Site Driveway	EB L/T			Α		А
	SB L/R			Α		А
(East)	ILOS			A (1.4)		A (1.3)

Base = No-Build scenario, Projected = Build scenario

ILOS = Overall Intersection Level of Service; Unsignalized ILOS calculated in accordance with Figure 5 of Policies and Procedures for Transportation Impact Studies.

As shown in **Tables 8 and 9**, under the 2025 and 2035 projected (build) conditions, the study area intersections will operate in accordance with the standards contained in Appendix A – Policies and Procedures for Transportation Impact Studies Related to Highway Occupancy Permits of PennDOT Publication 282, dated September 2022 and will meet Upper Allen Township SALDO requirements.

95TH PERCENTILE QUEUE ANALYSIS

Queue analyses were conducted at the study area intersections using *Synchro 11* software and reporting the HCM 6th Edition 95th percentile queue lengths. The queue analysis results are summarized in **Table 10** for the weekday AM and weekday PM peak hours.

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TABLE 10
HCM 95TH PERCENTILE QUEUE ANALYSIS

Movement	Available Storage Length ¹	2025 Base 2025 Proje (No-Build) (Build)			2035 Base (No-Build)		2035 Projected (Build)		
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
	Cumberland Parkway & Old Schoolhouse Lane (West)								
WB L/T	300′	0'	0'	0'	0'	0'	0′	0'	0'
NB L/R	500′+	3'	5′	8′	10′	3'	5′	8′	13′
Cumberland Parkway & Old Schoolhouse Lane (East)									
WB L/T	460′	3'	3'	5'	5′	3'	3′	5′	5'
NB L/R	200'+	3'	3'	5′	5′	3'	3′	5′	5′
Old Schoolhouse Lane & Proposed Site Driveway (West)									
EB L/T	250′+			3'	3'			3'	3'
SB L/R	100′+			3′	3'			3'	3′
Old Schoolhouse Lane & Proposed Site Driveway (East)									
EB L/T	75′+			0'	0'		1	0'	0'
SB L/R	100′+			0′	0′			0′	0′

Base = No-Build Scenario Pro

Projected = Build Scenario

Existing Storage/Proposed Storage

1 = Available storage measured to nearest public intersection.

As shown in **Table 10**, the 2025 and 2035 projected (build) condition queues will be accommodated within the existing/proposed storage length. Queue analysis worksheets are included with the capacity analysis worksheets provided in **Appendix E**.

AUXILIARY TURN LANE ANALYSIS

Methodology

TPD evaluated auxiliary turn lane warrants at the site access intersection of Old Schoolhouse Lane and the Proposed Western Site Driveway. The warrant analysis methodology contained within Chapter 11 of PennDOT's *Publication 46*, Section 11.17 was utilized for this evaluation. The auxiliary turn lane warrant analysis worksheets are contained in **Appendix F**.

Findings

Table 11 summarizes the results of the auxiliary turn lane analysis at the site access intersections.

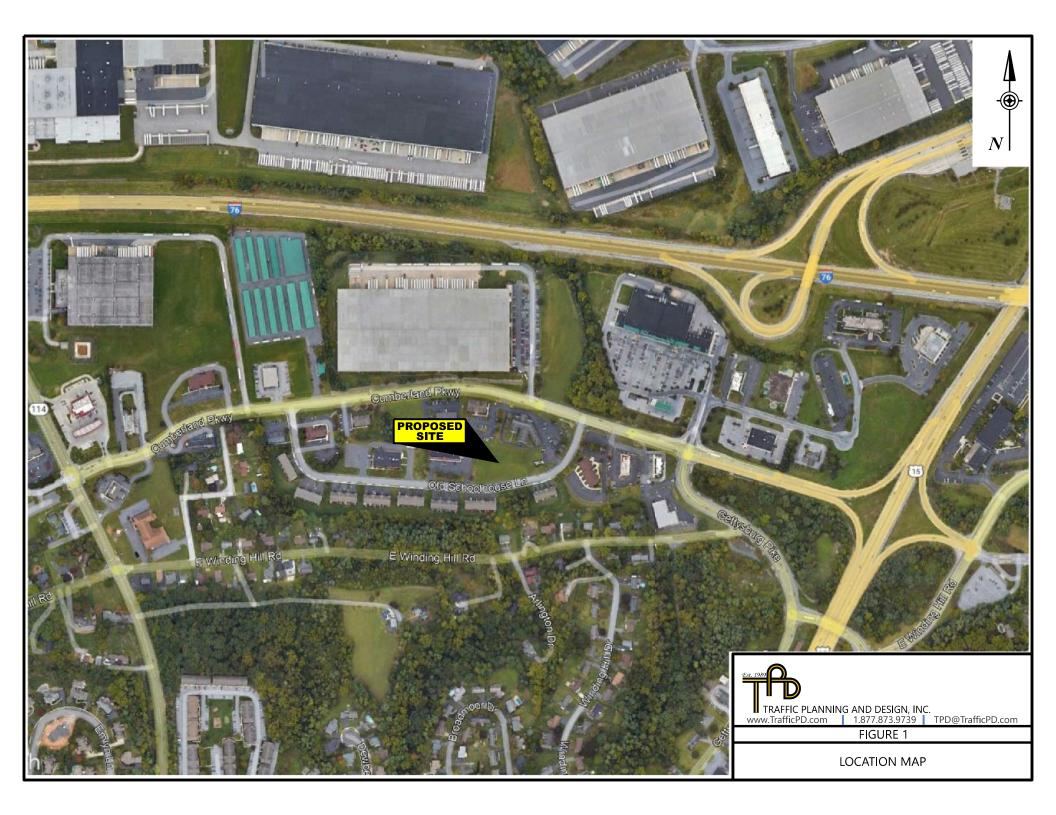
TABLE 11
AUXILIARY TURN LANE ANALYSIS SUMMARY

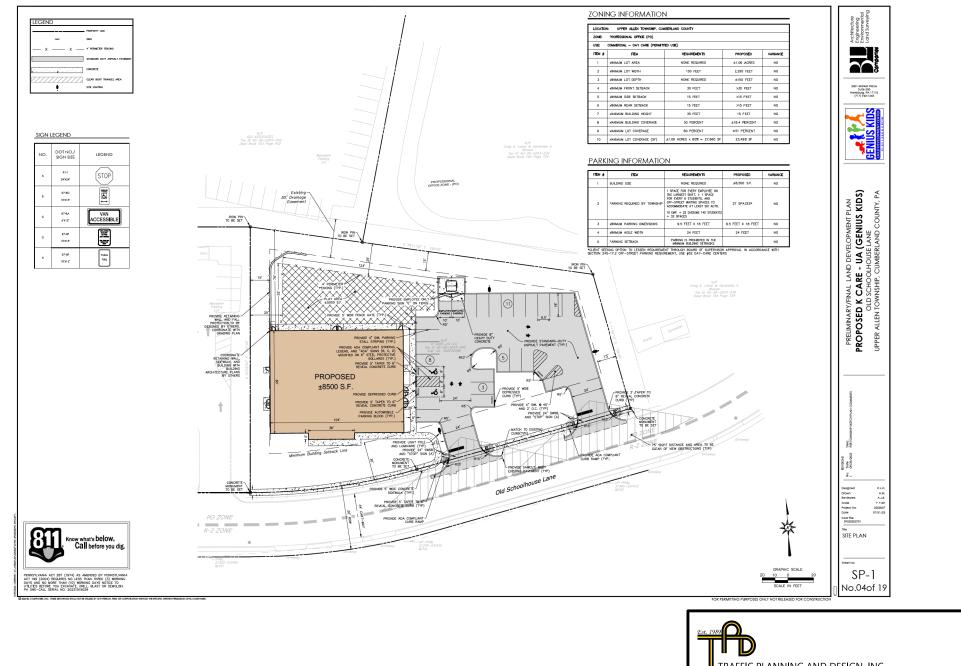
Intersection	Auxiliary Lane	Peak Hour	Warrant Satisfied?	Required Lane Length	Proposed Lane Length
	WB Right-Turn Lane	A.M.	No		
Old Schoolhouse Lane &		P.M.	No		
Proposed Site Driveway (West)	EB Left-Turn Lane	A.M.	No		
		P.M.	No		
		A.M.	No		
Old Schoolhouse Lane &	WB Right-Turn Lane	P.M.	No		
Proposed Site Driveway (East)		A.M.	No		
	EB Left-Turn Lane	P.M.	No		

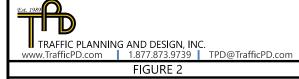
RECOMMENDATIONS AND CONCLUSIONS

The recommendations and conclusions for this Transportation Impact Study (TIS) are listed within the Executive Summary of this report.

Page 13— www.TrafficPD.com

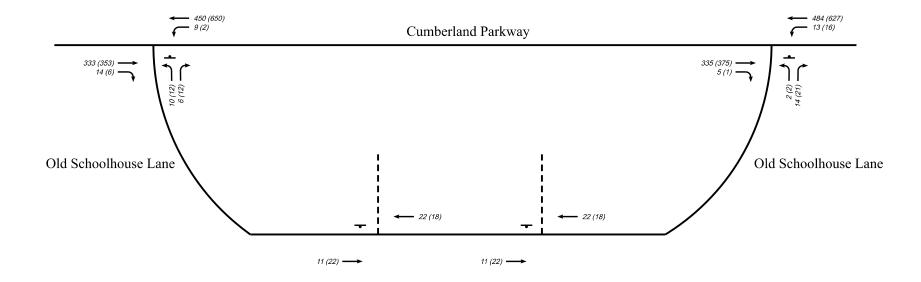




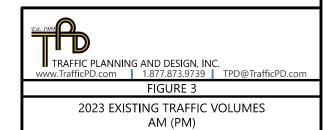


SITE PLAN

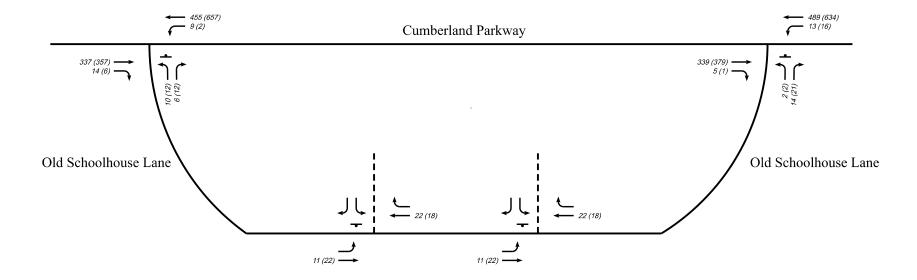


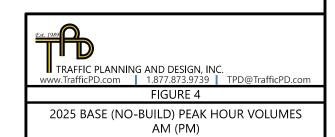




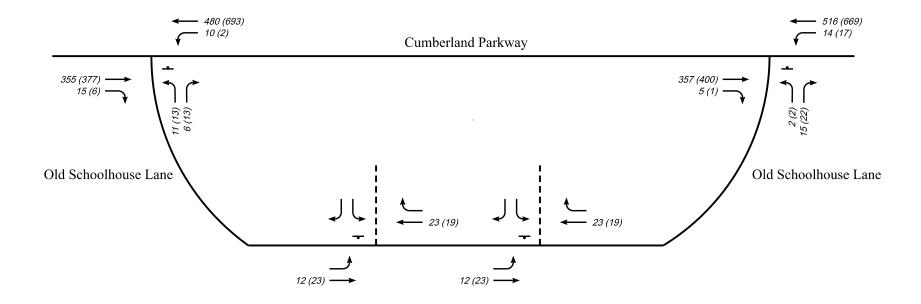








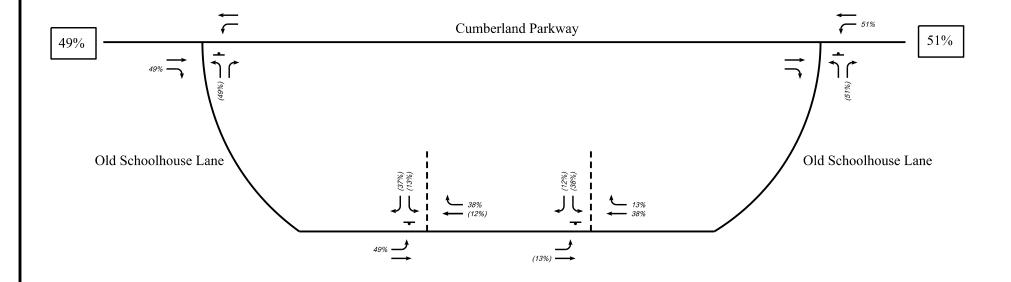






2035 BASE (NO-BUILD) PEAK HOUR VOLUMES AM (PM)





KEY: STOP CONTROLLED - · PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE



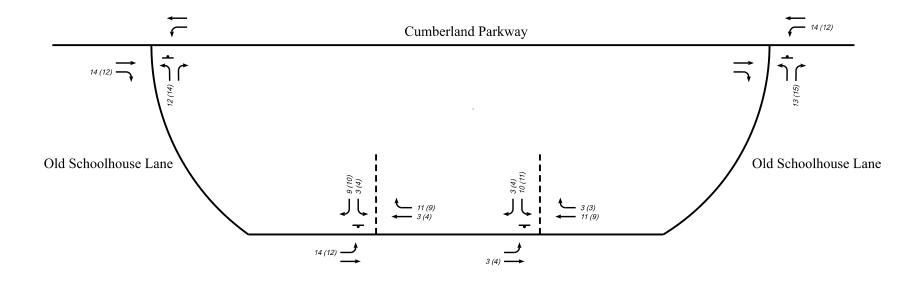
TRAFFIC PLANNING AND DESIGN, INC. www.TrafficPD.com | 1.877.873.9739 |

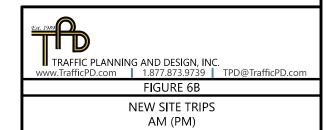
TPD@TrafficPD.com

FIGURE 6A

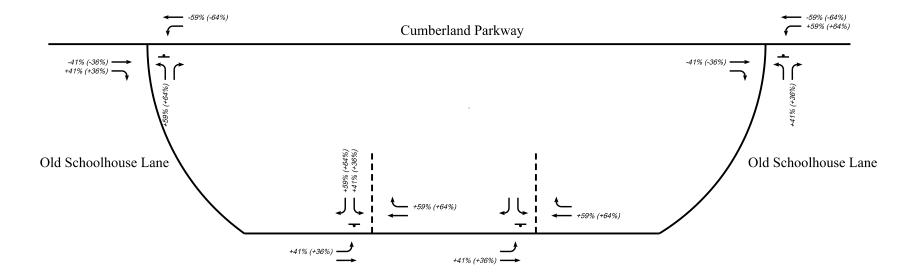
SITE TRIP DISTRIBUTION PERCENTAGES ENTER (EXIT)

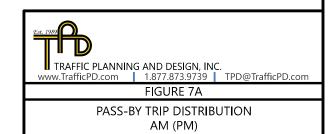




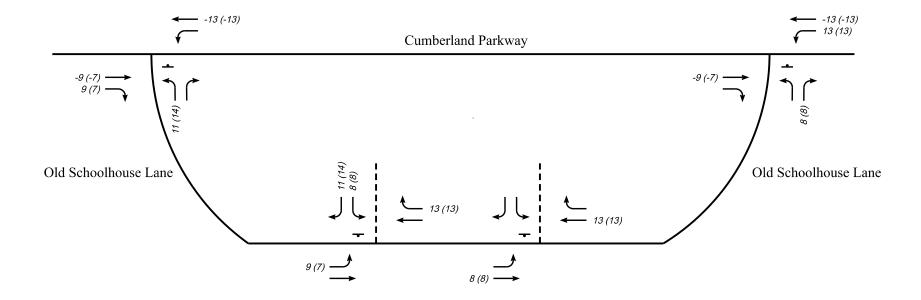


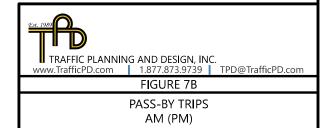




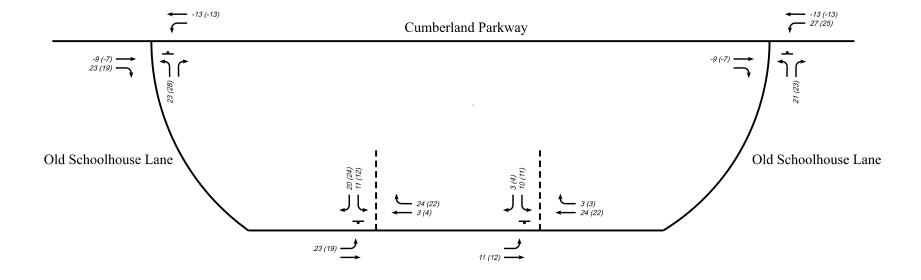








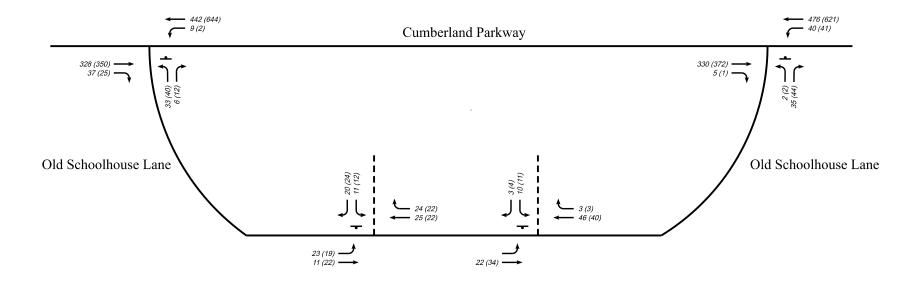


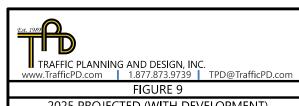




TOTAL PEAK HOUR SITE GENERATED TRIPS AM (PM)

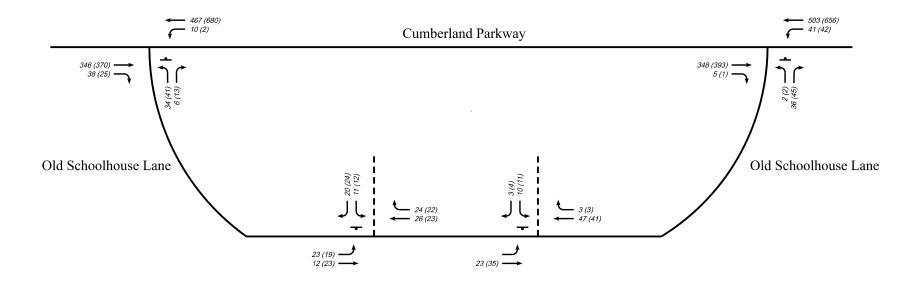


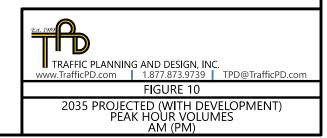




2025 PROJECTED (WITH DEVELOPMENT) PEAK HOUR VOLUMES AM (PM)







Appendix A:Project Correspondence



WWW.TRAFFICPD.COM

February 5, 2024

Ms. Jennifer Boyer, AICP – Community Development Director/Planner Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

Re: Response to Township's 1/19/24 Traffic Review Comments

Genius Kids Child Development Center

TPD# BLC.00127

Mrs. Boyer:

On behalf of the Applicant (Genius Kids Child Development), Traffic Planning and Design, Inc. (TPD) is resubmitting this Traffic Impact Study (TIS) package related to the proposed Genius Kids Child Development Center in Upper Allen Township.

Response to Township's January 19, 2024 review letter

For the discussion below, the Township's traffic comments from the 01/19/24 review letter are shown in italics, with the corresponding TPD responses in bold type.

Traffic

9. The sight distance looking left from the proposed site access (east) must be verified. It appears that there are two (2) evergreen trees on the adjacent property that may impact the sight distance at this driveway.

Sight distances were verified at the proposed site driveway intersection as applicable per Township Ordinance requirements (PennDOT's SSSD). While the evergreen trees on the adjacent property limit sight distances at the easternmost driveway, the measured available sight distances at the site driveway exceed applicable sight distance requirements.

- 10. The following changes shall be made to the study for clarity and accuracy, although the changes will have no impact on the results of the traffic study.
 - a. The P.M. peak critical headways were utilized for the A.M. Peak hour in the capacity analysis.
 - b. The turn lane analysis worksheets show "known" for Cycles per hour (assumed) when it should show "60" per PennDOT guidelines.

The capacity analysis has been revised as requested.

Sincerely,

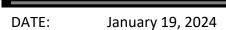
TRAFFIC PLANNING AND DESIGN, INC.

Jarred Neal P.E.

Senior Project Manager

JNeal@TrafficPD.com





TO: Upper Allen Township Planning Commission

Wayne Willey, Chair

FROM: Jennifer M. Boyer, AICP

Community Development Director/Planner

RE: Plan Name: K Care – UA (Genius Kids)

Plan Type: Prelim/Final Land Development

UAT File No.: 23-08-01

Property Parcel ID: 42-26-0243-045

Property Address: 151 Old Schoolhouse Lane, Mechanicsburg, PA 17055

Zoning District: Professional Office (PO)

The proposed project is for the construction of an 8,500 square foot daycare center with a 5,000 square foot fenced-in outdoor play area within the Professional Office (PO) Zoning District. The property is a vacant parcel of land on Old Schoolhouse Lane, approximately 1.06 acres in size.

Ordinance 828 was adopted on October 4, 2023 to update various regulations for child care and daycare facilities, including permitting child care centers in the PO Zoning District. The proposed use of the subject property is consistent with the Upper Allen Township's Zoning Ordinance and its Comprehensive Plan.

The Applicant has requested the following modifications:

1. Modify the requirements of Section 220-5.3.B.(5). The Applicant is proposing a portion of the sidewalk be outside the right-of-way as there is not enough space to provide a five-foot wide sidewalk and a five-foot wide grass area from the existing curb within the right-of-way.

Staff Comment: There is approximately 7-8 feet between the existing curb and edge of right-of-way to the property. Staff has no issues with the Applicant still providing a five-foot wide sidewalk, even if part of it does go onto private property. Staff had requested the Applicant provide a pedestrian access easement across their property wide enough to cover the sidewalk area and at least three feet of additional space alongside the sidewalk. The Applicant provided a six-foot wide access easement within their property to accommodate the sidewalk and excess area. The sidewalk must be constructed in accordance with our standards, as defined in Section 220-5.3.B.(2) and the Township's Construction and Material Specifications Manual.

2. Modify the requirements of Section 220-5.4.B.(3)(g) to provide a rounded tangential arc with a minimum radius of 15 feet instead of the required 30 feet.

Staff Comment: A lesser amount can typically be considered unless there is heavy tractor trailer use. This access drive is not expected to have heavy tractor trailer use. The existing buildings at 101 Old Schoolhouse Lane (western intersection of Old Schoolhouse Lane and Gettysburg Pike), 176 Cumberland Parkway, and 200 Cumberland Parkway all have access drives with 15' radii and operate with no issues. The access drives for 161 and 131 Old Schoolhouse Lane also appear to have the same design.

PARKING SPACES

Per Section 245-17.2, Table XVII-1 and the latest zoning ordinance amendment adopted October 4, 2023, Ordinance 828, there is not enough parking being provided to meet the minimum off-street parking standards.

Previous Parking Re	<u>egulations</u>	New Parking Regulations		
1 space for every employee	16	1 space for every employee on	16	
on largest shift		largest shift		
1 space for every 6 students	23 (140 students)	1 space for every 6 students	23 (140 students)	
6 waiting spaces	6			
TOTAL SPACES REQUIRED	45	TOTAL SPACES REQUIRED	39	
TOTAL SPACES PROVIDED	27	TOTAL SPACES PROVIDED	27	

The Applicant is proposing 27 off-street parking spaces, instead of the required 39 spaces. The Applicant may alter the design to provide for the required number of spaces, enter into a joint parking agreement with neighboring properties per Section 245-17.4, or seek authorization of a parking reduction from the Board of Commissioners per Section 245-17.5 of the Zoning Ordinance.

The Applicant is seeking approval to reduce the parking space requirement. Section 245-17.5.A. allows the Board of Commissioners, in consultation with the Township Engineer and Township Zoning Officer, to consider authorizing a reduction of minimum parking requirements for a non-residential use in the PO zoning district where the Applicant can justify a reduction and still provide adequate parking facilities to serve the use. The Applicant's attorney has provided a narrative for their request (see attached).

The Applicant references their existing daycare facility in Silver Spring Township. This facility may have a maximum of 144 students and has a peak parking demand of 23 spaces. Per Silver Spring Township's Code, one space for every six persons enrolled in the facility is required, so a maximum of 24 spaces would be required for their existing site. This facility on Old Schoolhouse

K Care-UA (Genius Kids) P/F LD Plan UAT File No. 23-08-01 PC Meeting: January 29, 2024

Lane would be similar in size and operation, with 27 parking spaces, which is four (4) additional spaces than the facility in Silver Spring Township.

KinderCare, located on Cumberland Parkway (near Wendy's), is an existing daycare facility that was constructed in the late 1990s/early 2000's. Parking requirements in effect at that time required one space for every ten students plus one space for every faculty/staff member. According to the land development plan, the building was designed to be certified for 179 students and 21 faculty/staff, with 80% of faculty/staff on site at any given time, thus requiring a total of 46 parking spaces. To staff's knowledge, there are no issues with on-site parking at KinderCare. The parking lot is rarely full, and the heaviest usage would be during special events. Staff also had an opportunity to speak to a parent who uses KinderCare as their daycare provider, and they said the parking lot is never full.

While the 27 parking spaces appear to be sufficient to accommodate the normal anticipated traffic for a typical day, the Applicant should provide an alternative parking plan for times when all parking spaces are filled during pick-up and/or drop-off times, and for special events such as graduation, holiday parties, etc.

TRAFFIC STUDY

Per Section 220-3.7.F. of the Subdivision Land Development Ordinance (SLDO), a traffic impact report is required for any non-residential use where the average daily traffic volume is 250 trips or more, or any mixed-use, or any building in excess of 10,000 square feet. The daycare is expected to generate approximately 405 total trips, according to the ITE Trip Generation Rates. The weekday A.M. Peak Hour total trips would be 94 (with 53 being new trips). The weekday P.M. Peak Hour total trips would be 95 (with 53 being new trips), as shown in Table 5 of the TIS. Vehicular traffic is estimated to be distributed evenly to/from the site from the east and the west side of Cumberland Parkway onto Old Schoolhouse Lane, as shown in Table 6 of the TIS.

The site will include the following improvements to accommodate the proposed daycare facility: two, full movement lanes (one in/one out of the site). A Stop Sign will be placed towards the exit of the site for movement onto Old Schoolhouse Lane. Sidewalks and appropriate ADA ramps will be constructed along the site on Old Schoolhouse Lane. As previously noted above, 53 new vehicles trips will occur during the weekday A.M. peak hours and 53 new vehicle trips will occur during the weekday P.M. peak hour. All intersections are operating at a LOS A and will continue to do so after build-out; therefore, no improvements to any of the intersections with Cumberland Parkway, Old Schoolhouse Lane, and the proposed site driveway are required beyond what the Applicant is already proposing to do.

The Township's Traffic Engineer reviewed the Applicant's traffic study. The Traffic Engineer has stated that the trip generation for the proposed 8,500 square foot building was calculated using the correct land use code. The proposed development is estimated to generate 94 total A.M.

K Care-UA (Genius Kids) P/F LD Plan UAT File No. 23-08-01 PC Meeting: January 29, 2024

peak hour (53 are new trips) and 95 P.M. peak hour trips (53 are new trips). There are a few comments to be addressed, which are noted below (see page 7).

TIMELINE

The following table presents the review period timeline for the above referenced application.

PLAN REVIEW PERIOD	CURRENT DATES
Application Date	09/29/23
Review Period Beginning Date	10/30/23
Time Extension Granted Until 03/29/24	11/17/23
Last Available Planning Commission Meeting	02/26/24
Last Available Board of Commissioners Meeting	03/20/24
Review Period End Date	03/29/24

RECREATION FEE/LAND DEDICATION

The Applicant shall, upon plan approval and prior to plan recording, contribute to the Township's Recreation Land Acquisition and Improvement Fund, in accordance with Section 220-5.15.D.(4) of the Codified Ordinances. The contribution amount shall be \$3,400.00.

OTHER AGENCY REVIEWS

The following agencies were notified on October 6, 2023 that this plan was available for review. Their comments have been included in this report.

AGENCY	SUBMISSION OF COMMENTS
Community Development Department	10/17/23; 01/15/24
Township Engineer (C.S. Davidson, Inc.)	10/20/23; 01/19/24
Traffic Engineer (TRG, Inc.)	10/06/23; 01/17/24
Sewer Department	10/23/23
Police Department	10/09/23 No Comments
Fire Department/Fire Marshal	10/09/23; 01/16/14 No Comments
Cumberland County Planning Commission	10/24/23

PLANNING COMMISSION - SUGGESTED MOTIONS

The Planning Commission should consider recommending approval or denial of the following modifications.

MODIFICATIONS

- 1. Move to recommend approval/denial of the modification request for Section 220-5.3.B.(5), allowing the Applicant to provide for a five-foot wide sidewalk along their property, a portion of which will be located outside the public right-of-way. The Applicant will provide for a six-foot wide pedestrian access easement around the portion of sidewalk and adjacent property area that is located within private property. The remainder of the sidewalk will be located within the public right-of-way.
- 2. Move to recommend <u>approval/denial</u> of the modification request for Section 220-5.4.B(.3)(g) to provide a rounded tangential arc with a minimum radius of 15 feet instead of the required 30 feet.

PLAN ACTION

The Planning Commission should consider recommending approval or denial of the subdivision plan. If recommending approval, the conditions listed below should be applied. If recommending denial, the reasons for denial should be given.

It is the recommendation of staff that the Applicant address conditions $\mathbf{1} - \mathbf{13}$ and revise their plans before the plan goes before the Board of Commissioners.

Move to recommend the Board of Commissioners <u>approve/disapprove</u> the preliminary/final land development for K Care – UA (Genius Kids), UAT File # 23-08-01, with the following conditions:

SUBDIVISION, LAND DEVELOPMENT & ZONING

1. Per Section 245-17.2, Table XVII-1 and the latest zoning ordinance amendment adopted October 2023 (Ordinance 828), there is not enough parking being provided to meet the minimum off-street parking standards. The Applicant must either obtain approval of a reduction per Section 245-17.5, enter into a joint parking agreement with neighboring properties per Section 245-17.4, or else meet the minimum standards as identified in Table XVII-1 of Codified Ordinances. If a reduction is granted, a note shall be listed on the Cover Sheet including the date granted.

- a. If a parking reduction is to be granted, the Applicant should provide an alternative parking plan for times when all parking spaces are filled during pick-up and/or drop-off times, and for special events such as graduation, holiday parties, etc., and comply with any other conditions as required by the Board of Commissioners.
- 2. Final plan submission shall include façade drawings, to include elevations, floor plans, lighting, etc., per Section 220-3.5.C.(2)(ff) of the Codified Ordinances.
- 3. The landscaping plan shall be updated to address the following planting information:
 - a. The total length of the property along the street right-of-way was incorrectly identified during the first review reducing the length to 173.61 feet. The length has been confirmed at 304.66 linear feet. The Buffer Yard #2 and street tree planting requirements must be based on the entire length of the property line at 304.66 feet, which would require the following additional plantings per Section 220-5.13.B.(1) of the Codified Ordinances:
 - i. Buffer Yard 2. Eight (8) shade trees, 11 evergreen trees, and 16 deciduous/evergreen shrubs required.
 - b. Landscape islands are to be provided at the end of each parking row, along with appropriate landscaping. The landscaping islands at the northeast corner and the west entrance area appear to not meet the minimum width, length, and/or depth requirements, per Section 220-5.13.B.(2)(a)[2]. We do note, however, that appropriate landscaping has been provided within the island areas.
 - c. Section 245-14.25.E of the Zoning Ordinance states that all outdoor play areas must provide a means of shade, such as shade tree(s) or pavilion(s). It appears the Applicant is providing two Autumn Brilliance Apple Serviceberry trees alongside the outdoor play area. These are small trees that will grow to about 15-25 feet in height and are not typically classified as shade trees, but are smaller trees, per Section 220-5.13.B. More appropriate shade trees should be incorporated into the outdoor play area, or else provide for pavilion(s).

STORMWATER

4. The Applicant shall provide an alternate hatch for bituminous roadway restoration to differentiate between the pavement section being used on-site versus within the public right-of-way. The current hatch infers that roadway restoration will follow the "Bituminous Paving Detail" on Sheet DN-1, which does not meet Township Specifications. Additionally, the trench

- restoration details on Sheet DN-4 need to comply with the Township Construction and Material Specifications.
- 5. The limits of sawcut and roadway restoration for utility installations must comply with the Township Street Cut Ordinance (Chapter 217). Curb removal and roadway restoration associated with the proposed water line connection have not been accounted for on the Plan.
- 6. The revised discharge point does not directly tie into the larger conveyance channel as intended. Extend the outfall pipe to the toe-of-slope at the upper limit of the conveyance channel, elevation 445.00'.
- 7. The Applicant shall revise discrepancies on the Plan regarding the length and slope of the BMP #2 outfall pipe.
- 8. Clarify the size of YD-2.0 and YD-2.1, and whether they are proposed to be Nyloplast Drain Basins as detailed on Sheet DN-6.

TRAFFIC

- 9. The sight distance looking left from the proposed site access (east) must be verified. It appears that there are two (2) evergreen trees on the adjacent property that may impact the sight distance at this driveway.
- 10. The following changes shall be made to study for clarity and accuracy, although the changes will have no impact on the results of the traffic study.
 - a. The P.M. peak critical headways were utilized for the A.M. Peak hour in the capacity analysis.
 - (b.) (The turn lane analysis worksheets shown "known" for Cycles per hour (assumed) when it should show "60" per PennDOT guidelines.

GENERAL

- 11. On the Cover Sheet, the following shall be addressed:
 - a. Cumberland County requires names of signatures to be identified on plans. Please include the printed name of the person who will sign the Certificate of Ownership and Stormwater signature areas on the Cover Sheet.

- b. The waivers and modifications on the Cover Sheet do not clearly define what each request is, as they are both now modification requests. Any approved waiver or modification must clearly be noted as to what type it is, the details of the request, and the date of approval. The Cover Sheet must be updated with more appropriate language, in accordance with Section 220-3.6.B(3) of the Codified Ordinances.
- 12. Within the Zoning Data information on Sheet SP1, list the minimum street frontage requirement. There is currently a section that states minimum lot width, which should be street frontage, as there is no minimum lot width requirement in this district, per Section 245-6.6.D. of the Codified Ordinances.
- 13. Employee parking is proposed in front of the dumpster. The Applicant should demonstrate how this will not create a conflict with trash services.

ADMINISTRATIVE

- 14. A certified, engineered design and details of all retaining walls shall be submitted and approved by the Township prior to the issuance of any permits. The final design must be reviewed and approved prior to recording the land development plan.
- 15. Fence details shall be provided for the dumpster area, per Section 220-5.13.B.(1)(e) of the Codified Ordinances. The detail sheets only include information for the dumpster gate.
- 16. The Applicant must obtain approval of the Erosion and Sediment Control Plan from the Cumberland County Conservation District and furnish to the Township a copy of the required NPDES permit in accordance with the requirements of Section 220-3.5.C(4)(h), Section 220-5.14., and Section 214-19.C of the Codified Ordinances.
- 17. The Applicant shall obtain approval of the planning module for new land development or approval of an exemption from the planning requirements from the Township and PA DEP in accordance with the requirements of Section 220-5.7.A of the Codified Ordinances of Upper Allen Township and pay all applicable application and tapping fees in accordance with the requirements of Chapter 200 of the Codified Ordinances.
- 18. The Applicant must enter into a Reservation of Capacity (ROC) Agreement with the Township and pay the appropriate ROC fees, or, pay tapping fees for the number of approved EDUs.

- 19. The Applicant must enter into a Sewer Extension Agreement with the Township and furnish the required \$1,000.00 escrow for plan and legal review costs, provide plats and legal descriptions for sanitary sewers to be located outside of the public rights-of-way, furnish the required escrow amount for inspection and related costs, and provide appropriate installation financial security for the sanitary sewers.
- 20. The Applicant must provide evidence that the storm drainage and stormwater management facilities has been reviewed and approved by the Township Engineer, in accordance with Sections 220-5.14 and 220-5.18, and Chapter 214 of the Codified Ordinances.
- 21. The Applicant must provide evidence that the sanitary sewer system design has been reviewed and approved by the Township Engineer, in accordance with Section 220-5.7.D(3)(b), and applicable sections of Chapters 199 and 200 of the Codified Ordinances.
- 22. The Applicant shall enter into a Stormwater Best Management Practices Maintenance Operation and Maintenance Agreement with the Township and pay all applicable fees, in accordance with Section 214-33. of the Codified Ordinances.
- 23. The Applicant must contribute to the Township Recreation Land Acquisition and Improvement Fund in the amount of \$3,400.00. This contribution to the Fund shall be paid at the time of approval of the subdivision or land development plan in accordance with the requirements of Section 220-5.15.D.(4) of the Codified Ordinances.
- 24. The Applicant must sign the plan and have the signatures notarized according to Section 220-3.5.C(2)(dd) and 220-3.6.B(1)(a) of the Codified Ordinances.
- 25. The Applicant must have the plan, including any profiles or drawings required under the provisions of Chapter 220, signed and sealed by a licensed surveyor and licensed engineer certifying to the accuracy of the survey and plan and that they are in conforming with the township code and other applicable state regulations, in accordance with Sections 220-3.5.C(2)(e), 220-3.6.B(1)(b), and 220-5.2.N(3) of the Codified Ordinances.
- 26. The Applicant must submit a signed and sealed construction cost estimate for all public improvements, including sanitary sewer work, in accordance with Section 220-4.2. of the Codified Ordinances.
- 27. The Applicant must provide financial security in a form acceptable to the Township and in an amount to be estimated by the applicant and approved by the Township Engineer to ensure construction of the improvements and/or concrete monuments shown on the plan, and the applicant must enter into an agreement with the Township providing for construction and installation of all improvements shown on the plan according to Section

- 220-4.2. of the Codified Ordinances. The financial security shall contain the provision that the Township shall be informed in writing thirty (30) days before the expiration date of any letter of credit or bond provided as a condition of approval.
- 28. The Applicant must also furnish financial security to the Township in an amount equal to the required percentage of the total financial security provided to cover the cost of construction inspection, administrative, and other related costs according to Section 220-7.3.B of the Codified Ordinances.
- 29. The Applicant shall also comply with all fees, taxes, utility rentals, building, police or fire codes, ordinances, resolutions, and regulations as may be in effect from time to time concerning the proposed development.
- 30. The Applicant shall obtain final water main design approval from Veolia Water Company and furnish to the Township an updated design plan.
- 31. The Applicant shall pay such fees as are charged from time to time by Upper Allen Township for other further reviews or permits as may be required concerning the proposed development.
- 32. The Applicant must satisfy all conditions on the approval of the plan and the plan must be recorded within 180 days from the date of written conditional approval by the Board of Commissioners or the plan will be considered disapproved.
- 33. Upon approval of the final plan and prior to obtaining township and county signatures for final plan recording, the Applicant shall provide a CD, a flash drive, or an electronic file submission that includes a .dwg AutoCAD file that includes one drawing of all the lots on the plan, in accordance with Section 220-3.6.A.(14) of the Codified Ordinances of Upper Allen Township. The data shall include all tract and parcel boundaries, lot lines of all lots on the plan, building footprints, street rights-of-way (public and private), curbs, sidewalks, storm sewer infrastructure, sanitary sewer infrastructure, edge of pavement, hydrants, all utility or other easements (public and private), declaration of planned communities/condominium documents (including amendments), and any other data as required by the township and the county. The file shall be with a spatial projection of PA State Plan projections, PA South Zone (3702), NAD83 horizontal datum, NAVD88 vertical datum. Units shall be in US survey foot. A digital copy of the final plan in PDF shall also be submitted. The county Planning Department will not sign final plans until this file has been provided to them.

Thank you.

cc: Commissioner Ginnie M. Anderson Commissioner Eric Fairchild Project File



WWW.TRAFFICPD.COM

November 17, 2023

Upper Allen Township 100 Gettysburg Pike Mechanicsburg, PA 17055

Attention: Ms. Jennifer Boyer, AICP

Re: Response to Upper Allen Township TIS Scope Review

Genius Kids Child Development CenterUpper Allen Township, Cumberland County, PA
TPD# BLC.00127

Jennifer,

On behalf of the Applicant, Traffic Planning and Design, Inc. (TPD) is submitting the Transportation Impact Study (TIS) package for the proposed Genius Kids Child Development Center in Upper Allen Township, Cumberland County, PA.

Response to Upper Allen Township's TIS Scope Submission Review #1

For the discussion below, Transportation Resource Group, Inc. (TRG) comments from the 10/06/2023 TIS scope review letter are shown in *italics*, with the corresponding TPD responses in **bold** type.

1. The trip generation for the proposed 8,500sf Day was calculated using ITE Land Use Code 565 (Day Care Center) and is acceptable for use in the study. According to ITE. The proposed Development is estimated to generate 94 total AM peak hour and 95 total PM peak hour trips. Table 2 shows the total trip generation for the entire site.

Noted.

2. The use of pass-by trips for the PM (44%) for the proposed Day Care is consistent with ITE and is acceptable for use in the study. Although ITE does not have data for the AM peak hour, the use of a 44% pass-by for the AM peak hour is also acceptable for use in the study.

Noted.

- 3. We concur with the following proposed study intersections to be included in the TIS.
 - Old Schoolhouse Lane/Proposed Site Driveway

The Study area should include the intersections.

- Old Schoolhouse Lane / Cumberland Parkway (East)
- Old Schoolhouse Lane / Cumberland Parkway (West)

Noted. The study evaluates all intersections identified above.

- 4. We concur with study time periods:
 - Weekday AM from 6:00 AM to 9:00 AM
 - Weekday PM from 3:00 PM to 6:00 PM

Noted.

5. We concur with the 0.54% yearly growth rate for use in the study, consistent with PennDOT's current Growth Factors.

Noted.

6. There are no planned developments in the area to be included in the study.

Noted.

7. We concur with the proposed trip distribution methodology for the use in the study.

Noted.

8. We concur with the use of HCM 6 Methodology for capacity analysis in the study.

Noted.

9. We concur with analysis years of existing 2023 and 2025 with and without development. In accordance with the Upper Allen Township SALDO Section 220-3.7.F(3)(c)[2], future project should consider a tenyear growth period. The study should also include a 2035 analysis year.

Noted. A 2025 opening year and a 2035 horizon year were evaluated as part of the study.

10. We concur with the auxiliary turn lane warrant analysis according to PennDOT's Publication 46.

Noted.

If you have any questions or require additional information to process this application, please call anytime.

Sincerely,

TRAFFIC PLANNING AND DESIGN, INC.

Jarred L. Neal, P.E.
Senior Project Manager
JNeal@Trafficpd.com

From: Jennifer Boyer <jboyer@uatwp.org>
Sent: Friday, October 6, 2023 8:03 AM
To: Neal, Jarred; Wheeler, Jason

Cc: Holtzman, Greg

Subject: RE: Genius Kids (Upper Allen) - TIS Scope Memo **Attachments:** Genuis Kids Scope Review (10-6-2023).pdf

Jarred, Jason -

Attached is a copy of the letter we received from our traffic engineer regarding your TIS scope.

Jen

Jennifer M. Boyer, AICP Community Development Director Upper Allen Township 717.766.0756 www.uatwp.org

From: Neal, Jarred < jneal@trafficpd.com> Sent: Thursday, October 5, 2023 4:03 PM

To: Jennifer Boyer < jboyer@uatwp.org>; Wheeler, Jason < JWheeler@trafficpd.com>; 'cschwab@consulttrg.com'

<cschwab@consulttrg.com>

Cc: Holtzman, Greg <gholtzman@Blcompanies.com>
Subject: RE: Genius Kids (Upper Allen) - TIS Scope Memo

Will do. Thank you Jen.

Jarred Neal, P.E., Senior Project Manager



OFFICES SERVING THE EASTERN UNITED STATES

From: Jennifer Boyer < <u>jboyer@uatwp.org</u>>
Sent: Thursday, October 5, 2023 4:02 PM

To: Wheeler, Jason < JWheeler@trafficpd.com; 'cschwab@consulttrg.com' < cschwab@consulttrg.com>

Cc: Neal, Jarred <ineal@trafficpd.com>; Holtzman, Greg <gholtzman@Blcompanies.com>

Subject: RE: Genius Kids (Upper Allen) - TIS Scope Memo

CAUTION: External email - do not click links or open attachments unless you recognize the sender and know the content is safe.

Jason -

Please have the scope include both intersections of Cumberland Parkway and Old Schoolhouse Lane since all vehicles entering/exiting the daycare must come out onto Cumberland Parkway via one of the two intersections.

Thank you Jen

Jennifer M. Boyer, AICP Community Development Director Upper Allen Township 717.766.0756 www.uatwp.org

From: Wheeler, Jason < JWheeler@trafficpd.com > Sent: Tuesday, September 26, 2023 11:58 AM

To: Jennifer Boyer < jboyer@uatwp.org>; 'cschwab@consulttrg.com' < cschwab@consulttrg.com>

Cc: Neal, Jarred <<u>ineal@trafficpd.com</u>>; Holtzman, Greg <<u>gholtzman@Blcompanies.com</u>>

Subject: Genius Kids (Upper Allen) - TIS Scope Memo

Jen/Chris,

TPD is pleased to submit the Transportation Impact Study (TIS) Scoping Memo for the proposed Genius Kids Child Development Center to be located along Old Schoolhouse Lane in Upper Allen Township, Cumberland County. Please review the attached at your convenience.

Thank you, Jason

Jason Wheeler, PTP, Project Manager



OFFICES SERVING THE EASTERN UNITED STATES

0: 717.234.1430 | D: 717.461.8424 | www.TrafficPD.com | f in 🕝 👸 🔼



PA Office 2 East Market Street Suite 2 York, PA 17401-1206 T: (717) 846-4660

Consulting Engineers and Planners www.consulttrg.com

MD Office 901 Dulaney Valley Road Suite 805 Towson, MD 21204-2624 T: (443) 275-2344

October 6, 2023

Upper Allen Township Attn: Jennifer Boyer, AICP Community Development Director/Planner 100 Gettysburg Pike Mechanicsburg, PA 17055

RE: Genuis Kids Child Development Center Review of Proposed Traffic Impact Study Scope Upper Allen Township, Cumberland County TRG Project No. 517.022.23

Dear Ms. Boyer:

Transportation Resource Group, Inc. has completed the review of the TIS Scope Determination for the proposed Genuis Kids Child Development Center located at 176-198 Old Schoolhouse Ln, Mechanicsburg, PA. The submission dated September 26, 2023, was completed by Traffic Planning and Design, Inc. Based on the review, we offer the following comments:

- 1. The trip generation for the proposed 8,500sf Day was calculated using ITE Land Use Code 565 (Day Care Center) and is acceptable for use in the study. According to ITE, the proposed Development is estimated to generate 94 total AM peak hour and 95 total PM peak hour trips. Table 2 shows the total trip generation for the entire site.
- 2. The use of pass-by trips for the PM (44%) for the proposed Day Care is consistent with ITE and is acceptable for use in the study. Although ITE does not have data for the AM peak hour, the use of a 44% pass-by for the AM peak hour is also acceptable for use in the study.
- 3. We concur with the following proposed study intersections to be included in the TIS.
 - Old Schoolhouse Lane / Proposed Site Driveway

The Study area should include the intersections.

- Old Schoolhouse Lane / Cumberland Parkway (East)
- Old Schoolhouse Lane / Cumberland Parkway (West)

- 4. We concur with study time periods:
 - Weekday AM from 6:00 AM to 9:00 AM
 - Weekday PM from 3:00 PM to 6:00 PM
- 5. We concur with the 0.54% yearly growth rate for use in the study, consistent with PennDOT's current Growth Factors.
- 6. There are no planned developments in the area to be included in the study.
- 7. We concur with the proposed trip distribution methodology for the use in the study.
- 8. We concur with the use of HCM 6 Methodology for capacity analysis in the study.
- 9. We concur with analysis years of existing 2023 and 2025 with and without development. In accordance with the Upper Allen Township SALDO Section 220-3.7.F(3)(c)[2], future project should consider a ten-year growth period. The study should also include a 2035 analysis year.
- 10. We concur with the auxiliary turn lane warrant analysis according to PennDOT's Publication 46.

If you have any questions regarding the above review comments, please feel free to give me a call.

Very truly yours,

Transportation Resource Group, Inc.

Christopher E. Schwab, P.E.

Christopher & School

Senior Associate

From: Wheeler, Jason

Sent: Tuesday, September 26, 2023 11:58 AM

To: AICP Jennifer Boyer (jboyer@uatwp.org); 'cschwab@consulttrg.com'

Cc: Neal, Jarred; Holtzman, Greg

Subject: Genius Kids (Upper Allen) - TIS Scope Memo

Attachments: 2023-09-26 (Genuis Kids - Upper Allen) TIS Scope Memo_attach.pdf

Jen/Chris,

TPD is pleased to submit the Transportation Impact Study (TIS) Scoping Memo for the proposed Genius Kids Child Development Center to be located along Old Schoolhouse Lane in Upper Allen Township, Cumberland County. Please review the attached at your convenience.

Thank you, Jason

Jason Wheeler, PTP, Project Manager



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O: 717.234.1430 | D: 717.461.8424 | www.TrafficPD.com | f in 🖸 🖰 🗖



MEMORANDUM

To: Jennifer Boyer – Upper Allen Township (Community Development Director/Planner)

From: Jarred L. Neal, P.E., *Traffic Planning and Design, Inc. (TPD)*

Cc: Greg Holtzman – BL Companies (Project Manager)

Chris Schwab, P.E. – TRG, Inc. (Township Traffic Engineer)

Date: September 26, 2023

Re: Transportation Impact Study (TIS) Scope Determination Request

Genius Kids – Upper Allen

Upper Allen Township, Cumberland County, PA

TPD# BLC.00127

This memo regards a proposed Genius Kids Child Development Center within Upper Allen Township, Cumberland County, PA. The proposed development is located on the north side Old Schoolhouse Lane southwest of the Cumberland Parkway/Old Schoolhouse Lane intersection (easternmost). The project involves construction of an 8,500 s.f. Child Development Center (Infant to Pre-K). Access for the development is proposed via two (2) driveway connections to Old Schoolhouse Lane, ultimately providing connections to Cumberland Parkway.

Prior to preparing the TIS, TPD is requesting the Township's feedback on the scope of the TIS. The following scope parameters are proposed:

TIS Scope

Study Area:

» Old Schoolhouse Lane and Proposed Site Driveway intersections.

Study Time Periods:

- » Weekday AM Peak Hour (6:00-9:00 A.M.);
- » Weekday PM Peak Hour (3:00-6:00 P.M.).

Background Growth Factor & Nearby Developments:

A background growth factor of 0.54% compounded annually will be applied to the existing traffic volumes in accordance with the current PennDOT BPR statistics.

TPD requests that Upper Allen Township, identify any nearby planned developments that will need to be included in this study.

Trip Generation:

The trip generation rates for the proposed development were obtained from the manual *Trip Generation*, Eleventh Edition, 2021, an Institute of Transportation Engineers (ITE) Informational Report. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

The data for Land Use Code 565 (Day Care Center) was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour.

The following should be noted with respect to the trip generation methodology:

- Based on the information provided in the manual *Trip Generation*, not all of the trips generated by the site will be "new" to the nearby roadway system. In addition to the "new trips" generated by the development, there will be "pass-by trips", which are trips that are drawn from the passing traffic stream and do not add trips to the adjacent roadways. Pass-by trip percentages were utilized as specified in the appendices of the manual *Trip Generation*, Eleventh Edition, 2021.
- Published data related to pass-by percentages are not available for Land Use Code #565 (Day Care Center) during the weekday A.M. peak hour. However, given the trip characteristics of a typical Day Care Center with parent drop-off/pick-up to and from there place of employment, etc. it is reasonable to assume that pass-by trips will occur during the weekday A.M. peak hour as well. Since ITE provides a weekday P.M. pass-by rate of 44% it is reasonable to assume that pass-by trips occur at the same rate during the weekday A.M. peak hour.

Table 1 shows the trip generation rates and directional split for the analyzed time periods.

TABLE 1
ITE TRIP GENERATION EQUATIONS

Land Use	ITE#	Time Period	Equations/Rates	Entering %	Exiting %	Pass-By %
Day Care 565	Average Weekday	T = 47.62*(X)	50%	50%		
	Weekday A.M. Peak Hour	T = 11.00*(X)	53%	47%	44% ¹	
		Weekday P.M. Peak Hour	T = 11.12*(X)	47%	53%	44%

T = number of site-generated vehicular trips
 X = independent variable (ksf of Gross Floor Area)
 1 = ITE pass-by rates not published, see TPD justification above

The calculated trip generation for the proposed development is shown in **Table 2**.

TABLE 2 TRIP GENERATION SUMMARY

Time Period	Total Trips		Pass-by Trips			New Trips			
Time Period	Total	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit
	Based on 8,500 s.f. of Gross Floor Area								
Average Weekday	405	202	203						
Weekday A.M. Peak Hour	94	50	44	41	22	19	53	31	25
Weekday P.M. Peak Hour	95	44	51	42	20	22	53	24	29

Trip Distribution:

The distribution and assignment of new trips generated by the development will be based upon the following: (1) existing traffic patterns in the study area; (2) the most logical route of travel; (3) site driveway locations/configurations.

Capacity Analyses:

Capacity analyses will be conducted for the weekday A.M. and P.M. peak hours at the study area intersections. These analyses were conducted according to the methodologies contained in the *Highway Capacity Manual*, 6th Ed. (HCM) using *Synchro 11* software, a Trafficware product:

- » 2023 Existing Conditions;
- » 2025 Base Conditions (opening year without the proposed development);
- » 2025 Projected Conditions (opening year with full build-out of the proposed development).

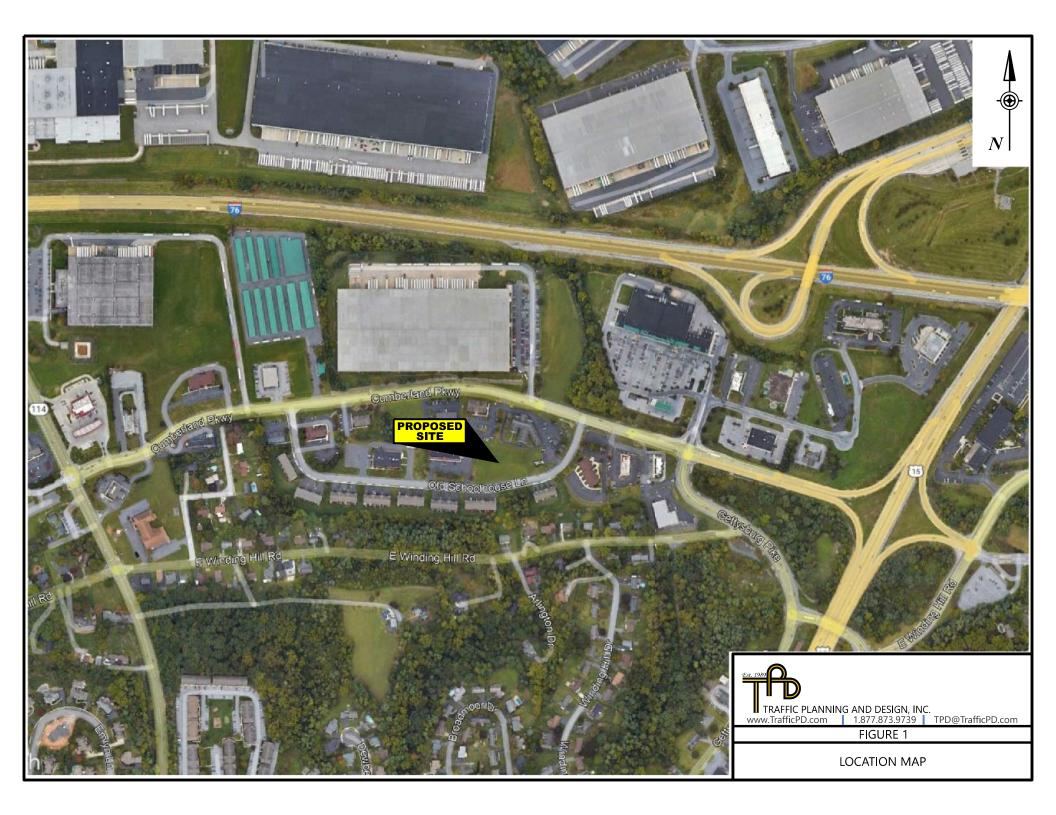
Auxiliary Turn Lane Warrants:

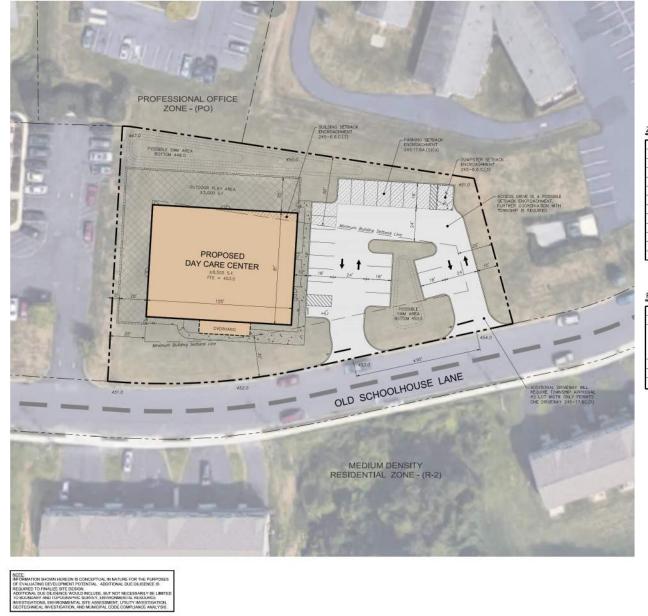
TPD will evaluate auxiliary turn lane warrants on Old Schoolhouse Road approaching the Site Driveway intersections. The warrant analysis was conducted according to the methodologies contained in Chapter 11 of PennDOT's *Publication 46* utilizing the posted speed limit.

We would appreciate your review of the enclosed information. If there are any questions or comments, please call any time.

Attachments:

Location Map Site Plan





ZONING INFORMATION

ZONE:	PROFESSIONAL OFFICE (PC)			
	DAY CARE, ACCESSORY (USE 68) (PE	RMITTED USE)		
ma ø	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
+	MINIMUM LOT AREA	NONE REQUIRED.	±1.06 ACRES	NO
2	MINIMUM LOT WOTH	150 FEET	1295 PEET	NO
3	MINIMUM LOT DEPTH	NONE REQUIRED	±150 FEET	NO
4	MINIMUM PRONT SETBACK	30 FEET	S30 FEET	NO
5	MINIMUM SIDE SETBACK	25 FEET	<25 FEET	×s.
	MINIMUM REAR SETBACK	50 FEET	<50 FEET	YES
7	MAXMUM BUILDING HEIGHT	35 FEET	<35 FEET	NO
80	WAXIMUW BUILDING COVERAGE	30 PERCENT	#20 PERCENT	NO.
9	MAXIMUM LOT COVERAGE	60 PERCENT	±50 PERCENT	NO:

PARKING INFORMATION

ITEM #	mov	REQUIREMENTS	PROPOSED	VARSANCE
1	BULDING SIZE	NONE REQUIRED	6,500 S.F.	NO
2	PARKING REQUIRED BY TOWNSHIP	1 SPACE FOR EVERY EMPLOYEE ON THE LANCEST SHIFT. + 1 SPACE: FOR EVERY 6 STADENTS, AND DIY-STREET WATING SPACES TO ACCOMMEDIATE AT LAST SIX AUTO, BEAM. + 8 (ASSIAN: 48 STADENTS) + 6 WATING = 22 SPACES	EPIFT. + 1 SPACE STUDENTS, AND AN INNE SPACES TO 22 SPACES AT LEAST SIX AUTO, ESLAW: 48 STUDENTS)	
3	MINIMUM PARKING DIMENSIONS	9.5 FEET X 18 FEET	9.5 FEET X 18 FEET	NO.
4	MINIMUM AISLE WIDTH	24 FEET	24 FEET	NO
5.	PARKING SETBACK	PARKING IS PROHEITED IN THE MINIMUM BUILDING SETBACKS	PARKING IS SHOWN WITHIN SIDE AND REAR SETBACKS	YES



PROPOSED DEVELOPMENT
OLD SCHOOL HOUSE LANE
UPPER ALLEN TOWNSHIP, CUMBERLAND COUNTY,

0.JH A.JS. T = 33 23-0000X 09070222

CONCEPT

2601 Martet Place Salte 260 Harristang, PA 17110 (717) 943-1665 (717) 051-9658: Fax

PA

TRAFFIC PLANNING AND DESIGN, INC.

www.TrafficPD.com | 1.877.873.9739 | TPD@TrafficPD.com

FIGURE 2

SITE PLAN

Appendix B: Existing Roadway Conditions



Direction / Road: NB Old Schoolhouse Lane (West)

Approach / Departure: Approach

50 feet Distance:



Direction / Road: NB Old Schoolhouse Lane (West)

Approach / Departure: Approach

> 200 Feet Distance:



Direction / Road: EB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



Direction / Road:EB Cumberland Pkwy

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

Distance: 50 feet



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

> 200 Feet Distance:

INTERSECTION WORKSHEET

Traffic Planning and Design, Inc.

TPD Project #	
Date	
Analyst	

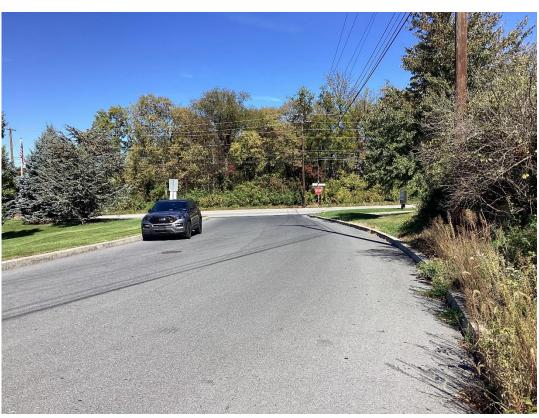
Osignalized Oun-signalized OTwo-Way Stop Control OAll-Way Stop Control OOffset OOther					
Area Type: Urb	an Suburban Rural	CBD			
Streets: (N-S)		(E-W	Y)		
	Westbound	Eastbound		Northbound	
	L T R	L T R	L T R	L T R	
No. Lanes	9	<u></u>		\leftarrow	
Lane Width	1				
Storage Length					
Grade (approaching intersection) + uphill, -downhill					
Channelized Right?					
If so, is lane >					
75'?					
Shoulder width Pavement					
condition*					
Lane marking condition*					
Posted speed limit					
Driveways on					
approach?	D	TD	D #	D "	
Bus Stops?	Route #:	Route #:	Route #:	Route #:	
Parking? Pedestrian Curb					
Ramps?	/	/	/	/	
Sidewalk?	/	/	/	/	
Crosswalks?					
	U	nsignalized Intersecti	ions:		
Sign Control					
Sight Distance*					
		Q. 1. 1			
No Turn on Red		Signalized Intersectio	ns:		
posted?					
Ped Button?					
Left Turn Phase	7		/		
Actuated Lanes	1				
Comments (please be	as specific as possible):	CONTRACTOR OF THE CONTRACTOR O			
\	-)) (
L T R 0>1<0		1	- T R 1<0	L T R 1 2 1	



Direction / Road: NB Old Schoolhouse Lane (East)

Approach / Departure: Approach

50 feet Distance:



Direction / Road: NB Old Schoolhouse Lane (East)

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: EB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



Direction / Road: EB Cumberland Pkwy

Approach / Departure: Approach

Distance: 200 Feet



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

50 feet Distance:



Direction / Road: WB Cumberland Pkwy

Approach / Departure: Approach

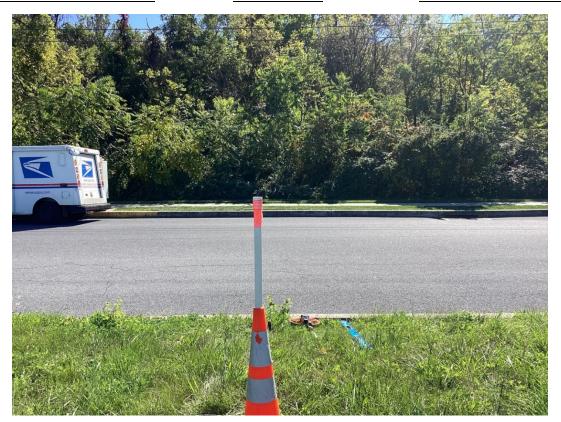
200 Feet Distance:

INTERSECTION WORKSHEET

Traffic Planning and Design, Inc.

TPD Project #	
Date	
Analyst	

Osignalized O	Un-signalized OTwo	-Way Stop Control O	All-Way Stop Control	Ooffset Oother
Area Type: Urba	an Suburban Rural	CBD		
Streets: (N-S)		(E-W	()	
	Westbound	Eastbound	,	Northbound
	L T R	L T R	L T R	L T R
No. Lanes	1	₽		\leftarrow
Lane Width				
Storage Length				
Grade (approaching intersection) + uphill, -downhill				
Channelized Right?				
If so, is lane > 75'?				
Shoulder width				
Pavement condition*				
Lane marking				
condition* Posted speed limit				
Driveways on				
approach?				
Bus Stops?	Route #:	Route #:	Route #:	Route #:
Parking?				
Pedestrian Curb Ramps?	/	/	/	/
Sidewalk?	/	/	/	/
Crosswalks?				
		Unsignalized Intersecti	ons:	·
Sign Control				
Sight Distance*				
		Signalized Intersection	ns:	
No Turn on Red				
posted?				A-T-MARKET TO A
Ped Button?				
Left Turn Phase			/	
Actuated Lanes	L			
Comments (please be	as specific as possible):			
	- ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	•	71	\
L T R 0>1<0	L T R 0>0<0	 L 1	T R 1<0	LTR 121



Approach / Departure:

Direction / Road: Center Driveway - Looking Out

Distance:



Approach / Departure:

Direction / Road: Center Driveway – Looking In

Distance:



Approach / Departure:

Direction / Road: Center Driveway – Looking Right

Distance:



Direction / Road: Approach / Departure: Center Driveway – Looking Left

Distance:

DRIVEWAY SIGHT DISTANCE MEASUREMENTS

(FOR LOCAL ROADS, USE PENNDOT PUB 70)

	•		<u>, , , , , , , , , , , , , , , , , , , </u>	
APPLICANT_			APPLICATION NO.	
S.R	SEG	OFFSET	LEGAL SPEED LIN	MIT
MEASURED	BY		DATE	
FOR DEPAR	TMENT USE ONLY: Sa	ufe-Running Speed	85th Percentile Spee	d
Λ				
A				
		<	GRADE%	3.50'
======	3.50'	GRADE%		==========
	ICE REQUIRED	Sight Line	DRIVER'S EYE 10' DISTANCE REQUIRED FSD=	
R			/HICH A DRIVER AT A DRIVEWAY LO LE APPROACHING ON THE ROADV	
			 	
		[GRADE	ne 3.50'
=======	=============	:========	:======================================	
		· ·	DISTANCE REQUIRED FSD=	
CO	ONTINUOUSLY SEE THE	REAR OF A VEHICLE WHIC	WHICH A DRIVER ON THE ROADWACH IS LOCATED IN THE DRIVER'S TO A LEFT TURN INTO A DRIVEWAY.	
С				
		Sight Line	3.50	
3.50	GRADE	%	<u> </u>	=============
	GIADE _		DISTANCE R FSD=	

THE MAXIMUM LENGTH OF ROADWAY ALONG WHICH A DRIVER OF A VEHICLE INTENDING TO MAKE A LEFT TURN INTO A DRIVEWAY CAN CONTINUOUSLY SEE A VEHICLE APPROACHING FROM THE OPPOSITE DIRECTION.



Direction / Road: Center Driveway - Looking Out

Approach / Departure:
Distance:



Direction / Road: Center Driveway – Looking In

Approach / Departure:
Distance:



Direction / Road: Center Driveway – Looking Right Approach / Departure:

Distance:



Direction / Road: Center Driveway - Looking Left Approach / Departure: Distance:

DRIVEWAY SIGHT DISTANCE MEASUREMENTS

(FOR LOCAL ROADS, USE PENNDOT PUB 70)

	,	•	, , , , , , , , , , , , , , , , , , ,	
APPLICANT			APPLICATION NO	
S.R	SEG	OFFSET	LEGAL SPEED LIN	МІТ
MEASURED BY			DATE	
FOR DEPARTMENT USE ONLY: Safe-Running Speed _			85th Percentile Speed	
Λ				
A				
		<	GRADE%	3.50'
======	3.50'	GRADE%		============
	CE REQUIRED	Sight Line	DRIVER'S EYE 10' DISTANCE REQUIRED FSD=	
P			/HICH A DRIVER AT A DRIVEWAY LO	
			GRADE 9 3.50'	ie 3.50'
co	ONTINUOUSLY SEE THE	REAR OF A VEHICLE WHIC	DISTANCE REQUIRED FSD= WHICH A DRIVER ON THE ROADWA CH IS LOCATED IN THE DRIVER'S TO A LEFT TURN INTO A DRIVEWAY.	
C				
			3.50	*
3.50	GRADE	Sight Line		=======================================
		, i	DISTANCE R FSD=	

THE MAXIMUM LENGTH OF ROADWAY ALONG WHICH A DRIVER OF A VEHICLE INTENDING TO MAKE A LEFT TURN INTO A DRIVEWAY CAN CONTINUOUSLY SEE A VEHICLE APPROACHING FROM THE OPPOSITE DIRECTION.

Appendix C:Manual Turning Movement Counts



Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 1

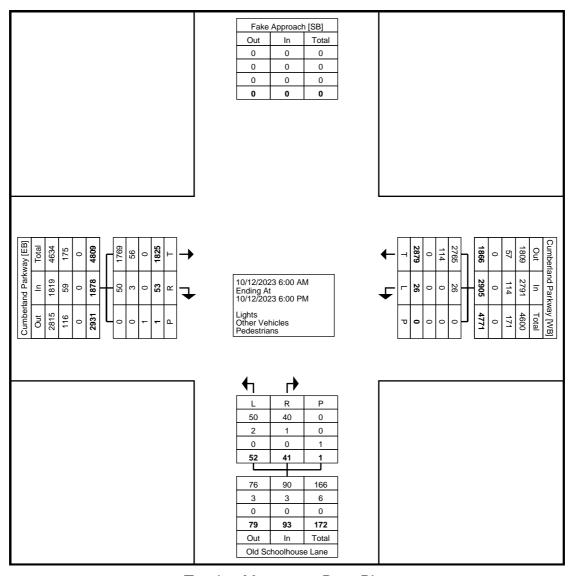
Turning Movement Data

		Cumberlan	nd Parkway	Ī		VIOVEM Cumberlar	nd Parkway		0	ld Schoolhou	se Lane (We	est)	
		Eastb	•				bound				bound	,	
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
6:00 AM	37	0	0	37	1	44	0	45	0	0	0	0	82
6:15 AM	42	1	0	43	0	35	0	35	1	0	0	1	79
6:30 AM	49	1	0	50	1	59	0	60	3	0	0	3	113
6:45 AM	65	1	0	66	1	77	0	78	0	0	0	0	144
Hourly Total	193	3	0	196	3	215	0	218	4	0	0	4	418
7:00 AM	56	4	0	60	1	80	0	81	0	2	0	2	143
7:15 AM	85	5	0	90	4	105	0	109	3	1	0	4	203
7:30 AM	81	6	0	87	0	120	0	120	1	1	0	2	209
7:45 AM	103	7	0	110	4	117	0	121	4	2	0	6	237
Hourly Total	325	22	0	347	9	422	0	431	8	6	0	14	792
8:00 AM	60	0	0	60	4	102	0	106	3	1	0	4	170
8:15 AM	89	1	0	90	1	111	0	112	2	2	0	4	206
8:30 AM	80	1	0	81	0	91	0	91	1	1	0	2	174
8:45 AM	67	4	0	71	3	105	0	108	1	1	0	2	181
Hourly Total	296	6	0	302	8	409	0	417	7	5	0	12	731
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	1
3:00 PM	91	3	0	94	1	143	0	144	1	4	0	5	243
3:15 PM	85	0	0	85	0	128	0	128	1	4	0	5	218
3:30 PM	76	3	0	79	1	133	0	134	7	5	0	12	225
3:45 PM	92	2	0	94	0	141	0	141	1	2	0	3	238
Hourly Total	344	8	0	352	2	545	0	547	10	15	0	25	924
4:00 PM	85	0	0	85	0	155	0	155	3	0	0	3	243
4:15 PM	79	0	1	79	1	163	0	164	1	2	1	3	246
4:30 PM	95	3	0	98	0	163	0	163	3	2	0	5	266
4:45 PM	90	1	0	91	0	178	0	178	3	5	0	8	277
Hourly Total	349	4	1	353	1	659	0	660	10	9	1	19	1032
5:00 PM	89	2	0	91	1	146	0	147	5	3	0	8	246
5:15 PM	74	1	0	75	2	155	0	157	3	1	0	4	236
5:30 PM	80	3	0	83	0	177	0	177	0	2	0	2	262
5:45 PM	75	4	0	79	0	151	0	151	5	0	0	5	235
Hourly Total	318	10	0	328	3	629	0	632	13	6	0	19	979
Grand Total	1825	53	1	1878	26	2879	0	2905	52	41	1	93	4876
Approach %	97.2	2.8	-	-	0.9	99.1	-	-	55.9	44.1	-	-	-
Total %	37.4	1.1	-	38.5	0.5	59.0	-	59.6	1.1	0.8	-	1.9	-
Lights	1769	50	-	1819	26	2765	-	2791	50	40	-	90	4700
% Lights	96.9	94.3	-	96.9	100.0	96.0	-	96.1	96.2	97.6	-	96.8	96.4
Other Vehicles	56	3	-	59	0	114	-	114	2	1	-	3	176
% Other Vehicles	3.1	5.7	-	3.1	0.0	4.0	-	3.9	3.8	2.4	-	3.2	3.6
Pedestrians	-	-	1	-	-	-	0	-	-	-	1	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	100.0	-	-



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Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 3

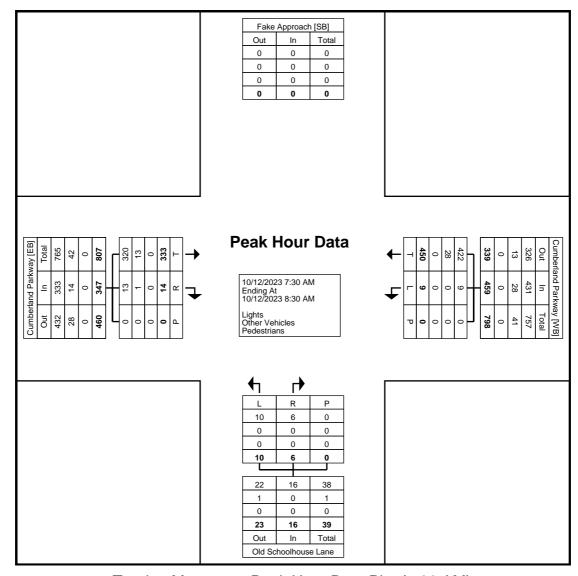
Turning Movement Peak Hour Data (7:30 AM)

			. •					~ (,				
		Cumberlan	d Parkway	_		Cumberlan	d Parkway	,	Ó	ld Schoolhou	se Lane (W	est)	
Ota at Time a		Easth	ound			West	oound			North	bound		
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
7:30 AM	81	6	0	87	0	120	0	120	1	1	0	2	209
7:45 AM	103	7	0	110	4	117	0	121	4	2	0	6	237
8:00 AM	60	0	0	60	4	102	0	106	3	1	0	4	170
8:15 AM	89	1	0	90	1	111	0	112	2	2	0	4	206
Total	333	14	0	347	9	450	0	459	10	6	0	16	822
Approach %	96.0	4.0	-	-	2.0	98.0	-	-	62.5	37.5	-	-	-
Total %	40.5	1.7	-	42.2	1.1	54.7	-	55.8	1.2	0.7	-	1.9	-
PHF	0.808	0.500	-	0.789	0.563	0.938	-	0.948	0.625	0.750	-	0.667	0.867
Lights	320	13	-	333	9	422	-	431	10	6	-	16	780
% Lights	96.1	92.9	-	96.0	100.0	93.8	-	93.9	100.0	100.0	-	100.0	94.9
Other Vehicles	13	1	-	14	0	28	-	28	0	0	-	0	42
% Other Vehicles	3.9	7.1	-	4.0	0.0	6.2	-	6.1	0.0	0.0	-	0.0	5.1
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	_	-		_	_				_			-	-



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Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 5

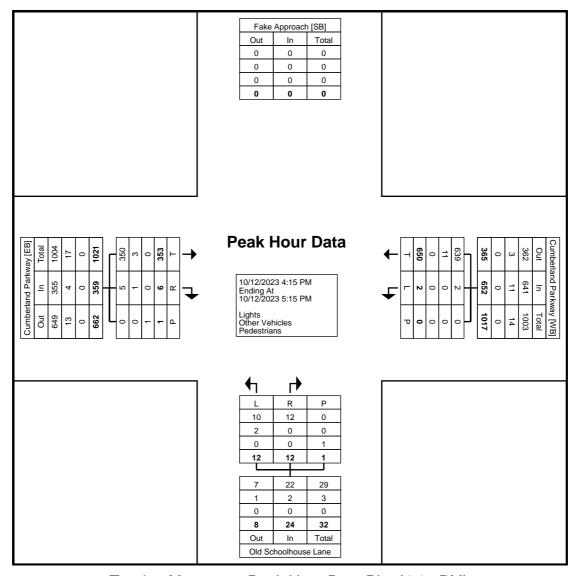
Turning Movement Peak Hour Data (4:15 PM)

								١ ،	1 '				1
	Cumberland Parkway					Cumberlan	d Parkway		0	ld Schoolhou	se Lane (We	est)	
Start Time		Easth	oound			Westh	oound			North	bound		
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
4:15 PM	79	0	1	79	1	163	0	164	1	2	1	3	246
4:30 PM	95	3	0	98	0	163	0	163	3	2	0	5	266
4:45 PM	90	1	0	91	0	178	0	178	3	5	0	8	277
5:00 PM	89	2	0	91	1	146	0	147	5	3	0	8	246
Total	353	6	1	359	2	650	0	652	12	12	1	24	1035
Approach %	98.3	1.7	-	-	0.3	99.7	-	-	50.0	50.0	-	-	-
Total %	34.1	0.6	-	34.7	0.2	62.8	-	63.0	1.2	1.2	-	2.3	-
PHF	0.929	0.500	-	0.916	0.500	0.913	-	0.916	0.600	0.600	-	0.750	0.934
Lights	350	5	-	355	2	639	-	641	10	12	-	22	1018
% Lights	99.2	83.3	-	98.9	100.0	98.3	-	98.3	83.3	100.0	-	91.7	98.4
Other Vehicles	3	1	-	4	0	11	-	11	2	0	-	2	17
% Other Vehicles	0.8	16.7	-	1.1	0.0	1.7	-	1.7	16.7	0.0	-	8.3	1.6
Pedestrians	-	-	1	-	-	-	0	-	-	-	1	-	-
% Pedestrians	-	-	100.0	-	-	-	-	-	-	-	100.0	-	-



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Count Name: (1) AM/PM Old Schoolhouse Lane West x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:15 PM)



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 1

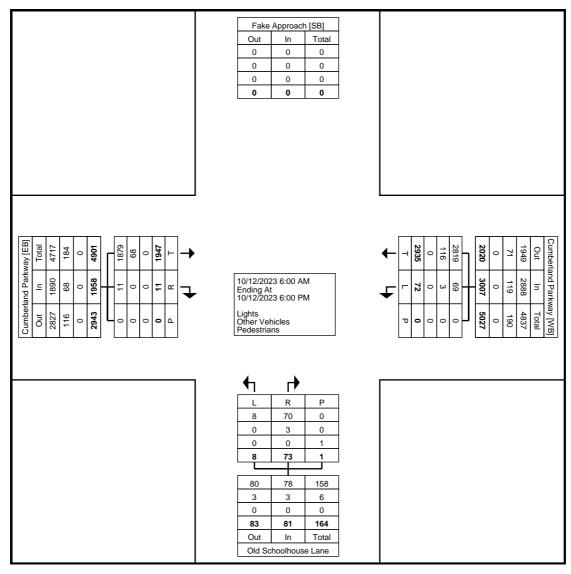
Turning Movement Data

				ΙŲ	ming i	Movem	ient D	ala					
		Cumberland	d Parkway			Cumberlan	nd Parkway		0	ld Schoolhou	ise Lane (Ea	ist)	
Start Time		Eastb	ound			West	bound			North	bound		
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
6:00 AM	70	0	0	70	1	49	0	50	0	1	0	. 1	121
6:15 AM	34	0	0	34	1	55	0	56	0	1	0	. 1	91
6:30 AM	49	0	0	49	4	63	0	67	0	4	0	4	120
6:45 AM	60	0	0	60	0	85	0	85	0	2	0	2	147
Hourly Total	213	0	0	213	6	252	0	258	0	8	0	. 8	479
7:00 AM	56	1	0	57	3	92	0	95	0	3	0	3	155
7:15 AM	73	0	0	73	4	114	0	118	1	0	0	1	192
7:30 AM	81	1	0	82	3	125	0	128	0	4	0	4	214
7:45 AM	108	0	0	108	6	127	0	133	0	5	0	5	246
Hourly Total	318	2	0	320	16	458	0	474	1	12	0	13	807
8:00 AM	54	2	0	56	3	108	0	111	1	2	0	3	170
8:15 AM	92	2	0	94	1	124	0	125	1	3	0	4	223
8:30 AM	78	2	0	80	6	92	0	98	0	2	0	2	180
8:45 AM	72	0	0	72	3	114	0	117	1	3	0	4	193
Hourly Total	296	6	0	302	13	438	0	451	3	10	0	13	766
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	123	0	0	123	2	138	0	140	0	3	0	3	266
3:15 PM	90	1	0	91	2	137	0	139	0	2	0	2	232
3:30 PM	94	1	0	95	1	127	0	128	1	4	0	5	228
3:45 PM	95	0	0	95	6	139	0	145	0	3	0	3	243
Hourly Total	402	2	0	404	11	541	0	552	1	12	0	13	969
4:00 PM	94	0	0	94	2	136	0	138	0	4	0	4	236
4:15 PM	88	0	0	88	4	148	0	152	0	3	1	3	243
4:30 PM	95	0	0	95	6	163	0	169	1	2	0	3	267
4:45 PM	99	0	0	99	3	168	0	171	1	3	0	4	274
Hourly Total	376	0	0	376	15	615	0	630	2	12	1	14	1020
5:00 PM	99	0	0	99	7	141	0	148	0	9	0	9	256
5:15 PM	82	1	0	83	0	155	0	155	0	7	0	7	245
5:30 PM	82	0	0	82	2	178	0	180	0	2	0	2	264
5:45 PM	79	0	0	79	2	157	0	159	1	1	0	2	240
Hourly Total	342	1	0	343	11	631	0	642	1	19	0	20	1005
Grand Total	1947	11	0	1958	72	2935	0	3007	8	73	1	81	5046
Approach %	99.4	0.6	-	-	2.4	97.6	-	-	9.9	90.1	-	-	-
Total %	38.6	0.2	-	38.8	1.4	58.2	-	59.6	0.2	1.4	-	1.6	-
Lights	1879	11	-	1890	69	2819	-	2888	8	70	-	78	4856
% Lights	96.5	100.0	-	96.5	95.8	96.0	-	96.0	100.0	95.9	-	96.3	96.2
Other Vehicles	68	0	-	68	3	116	-	119	0	3	-	3	190
% Other Vehicles	3.5	0.0	-	3.5	4.2	4.0	-	4.0	0.0	4.1	-	3.7	3.8
Pedestrians	-	-	0	-	-	-	0	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 3

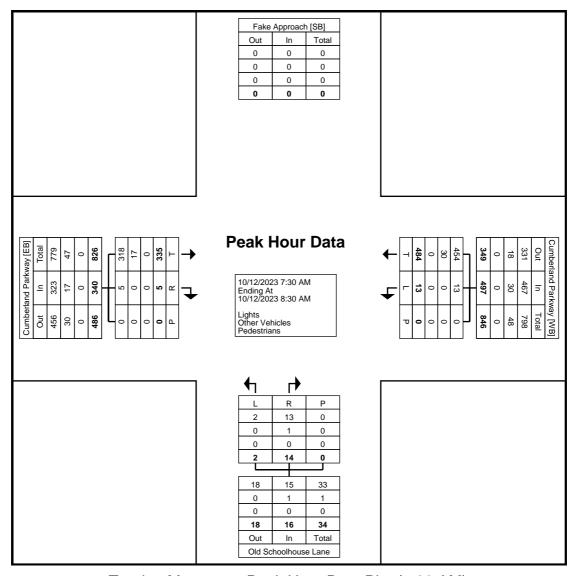
Turning Movement Peak Hour Data (7:30 AM)

							,	1 '					
	Cumberland Parkway					Cumberlan	d Parkway		C	ld Schoolhou	se Lane (Ea	ast)	
Start Time		Easth	oound			West	oound			North	oound		
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
7:30 AM	81	1	0	82	3	125	0	128	0	4	0	4	214
7:45 AM	108	0	0	108	6	127	0	133	0	5	0	5	246
8:00 AM	54	2	0	56	3	108	0	111	1	2	0	3	170
8:15 AM	92	2	0	94	1	124	0	125	1	3	0	4	223
Total	335	5	0	340	13	484	0	497	2	14	0	16	853
Approach %	98.5	1.5	-	-	2.6	97.4	-	-	12.5	87.5	-	-	-
Total %	39.3	0.6	-	39.9	1.5	56.7	-	58.3	0.2	1.6	-	1.9	-
PHF	0.775	0.625	-	0.787	0.542	0.953	-	0.934	0.500	0.700	-	0.800	0.867
Lights	318	5	-	323	13	454	-	467	2	13	-	15	805
% Lights	94.9	100.0	-	95.0	100.0	93.8	-	94.0	100.0	92.9	-	93.8	94.4
Other Vehicles	17	0	-	17	0	30	-	30	0	1	-	1	48
% Other Vehicles	5.1	0.0	-	5.0	0.0	6.2	-	6.0	0.0	7.1	-	6.3	5.6
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-		-	-	-	-	-



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:30 AM)



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 5

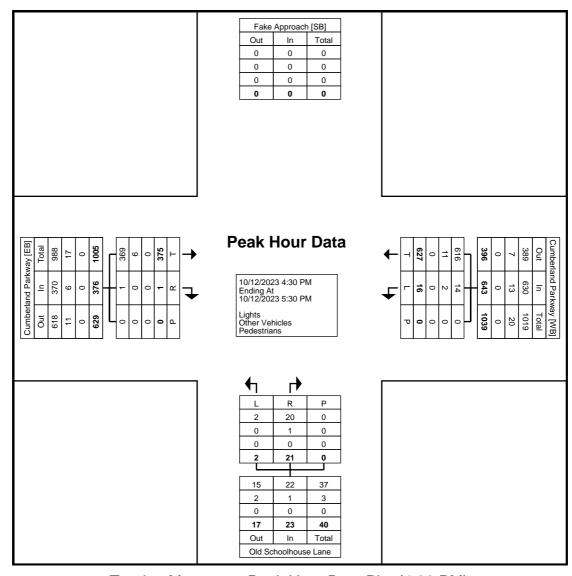
Turning Movement Peak Hour Data (4:30 PM)

	1							١ ،	1 '				
	Cumberland Parkway					Cumberlan	d Parkway		C	old Schoolhou	se Lane (Ea	ast)	
Start Time		Easth	ound			Westh	oound			North	bound		
Start Time	Thru	Right	Peds	App. Total	Left	Thru	Peds	App. Total	Left	Right	Peds	App. Total	Int. Total
4:30 PM	95	0	0	95	6	163	0	169	1	2	0	3	267
4:45 PM	99	0	0	99	3	168	0	171	1	3	0	4	274
5:00 PM	99	0	0	99	7	141	0	148	0	9	0	9	256
5:15 PM	82	1	0	83	0	155	0	155	0	7	0	7	245
Total	375	1	0	376	16	627	0	643	2	21	0	23	1042
Approach %	99.7	0.3	-	-	2.5	97.5	-	-	8.7	91.3	-	-	-
Total %	36.0	0.1	-	36.1	1.5	60.2	-	61.7	0.2	2.0	-	2.2	-
PHF	0.947	0.250	-	0.949	0.571	0.933	-	0.940	0.500	0.583	-	0.639	0.951
Lights	369	1	-	370	14	616	-	630	2	20	-	22	1022
% Lights	98.4	100.0	-	98.4	87.5	98.2	-	98.0	100.0	95.2	-	95.7	98.1
Other Vehicles	6	0	-	6	2	11	-	13	0	1	-	1	20
% Other Vehicles	1.6	0.0	-	1.6	12.5	1.8	-	2.0	0.0	4.8	-	4.3	1.9
Pedestrians	-	-	0	-	-	-	0	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: (2) AM/PM Old Schoolhouse Lane East x Cumberland Pkwy Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:30 PM)



Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 1

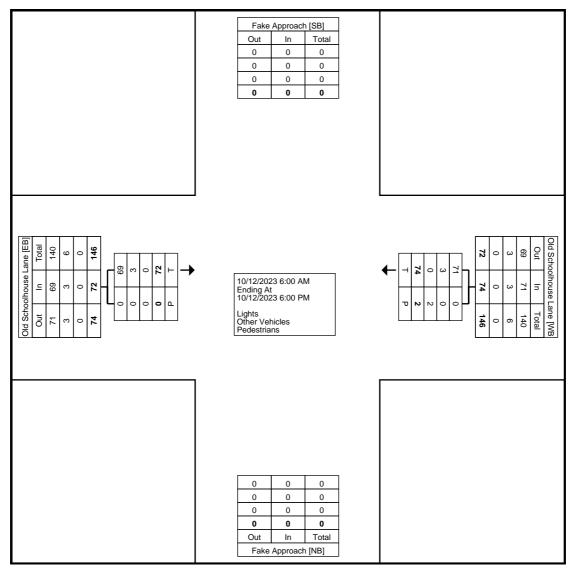
Turning Movement Data

		Turnir	ng Movemei	nt Data			_
		Old Schoolhouse Lar			Old Schoolhouse Lar	ne	
Start Time		Eastbound			Westbound		
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
6:00 AM	1	0	1	1	0	. 1	2
6:15 AM	0	0	0	1	0	1	1
6:30 AM	4	0	4	4	0	4	8
6:45 AM	2	. 0	2	0	. 0	0	2
Hourly Total	7	0	7	6	0	6	13
7:00 AM	1	0	1	3	0	3	4
7:15 AM	1	0	1	5	0	5	6
7:30 AM	5	0	5	2	0	2	7
7:45 AM	4	0	4	7	0	7	11
Hourly Total	11	0	11	17	0	17	28
8:00 AM	2	0	2	5	0	5	7
8:15 AM	2	0	2	2	0	2	4
8:30 AM	3	0	3	8	0	8	11
8:45 AM	3	0	3	3	0	3	6
Hourly Total	10	0	10	18	0	18	28
*** BREAK ***	-	-		-	-	-	-
3:00 PM	3	0	3	2	0	2	5
3:15 PM	1	0	1	2	0	2	3
3:30 PM	5	0	5	2	0	2	7
3:45 PM	2	0	2	2	0	2	4
Hourly Total	11	0	11	8	0	8	19
4:00 PM	2	0	2	5	0	5	7
4:15 PM	4	0	4	1	0	1	5
4:30 PM	4	0	4	6	0	6	10
4:45 PM	3	0	3	4	0	4	7
Hourly Total	13	0	13	16	0	16	29
5:00 PM	9	0	9	6	2	6	15
5:15 PM	6	0	6	2	0	2	8
5:30 PM	2	0	2	1	0	. 1	3
5:45 PM	3	0	3	0	0	0	3
Hourly Total	20	0	20	9	2	9	29
Grand Total	72	0	72	74	2	74	146
Approach %	100.0	-	<u>-</u>	100.0	-	-	-
Total %	49.3	-	49.3	50.7	-	50.7	-
Lights	69	-	69	71	-	71	140
% Lights	95.8	-	95.8	95.9	-	95.9	95.9
Other Vehicles	3	-	3	3	-	3	6
% Other Vehicles	4.2	-	4.2	4.1	-	4.1	4.1
Pedestrians	-	0	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-



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Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 2



Turning Movement Data Plot



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Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 3

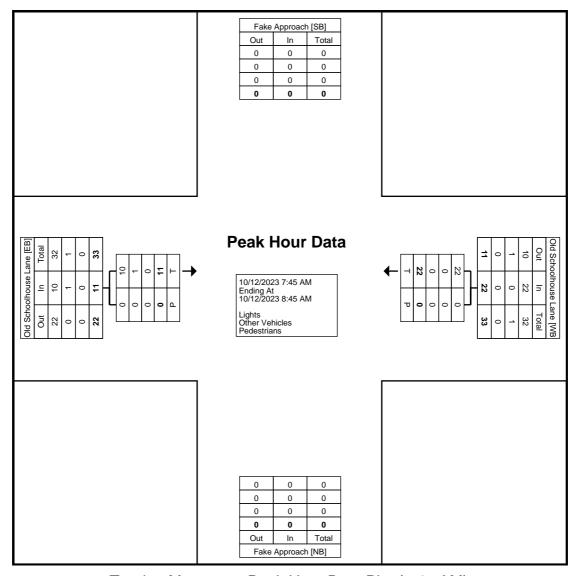
Turning Movement Peak Hour Data (7:45 AM)

		Old Schoolhouse Lar	ne		Old Schoolhouse La	ne	
Otant Time		Eastbound			Westbound		
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
7:45 AM	4	0	4	7	0	7	11
8:00 AM	2	0	2	5	0	5	7
8:15 AM	2	0	2	2	0	2	4
8:30 AM	3	0	3	8	0	8	11
Total	11	0	11	22	0	22	33
Approach %	100.0	-	-	100.0	-	-	-
Total %	33.3	_	33.3	66.7	_	66.7	-
PHF	0.688	-	0.688	0.688	-	0.688	0.750
Lights	10	-	10	22	-	22	32
% Lights	90.9	-	90.9	100.0	-	100.0	97.0
Other Vehicles	1	-	1	0	-	0	1
% Other Vehicles	9.1	-	9.1	0.0	-	0.0	3.0
Pedestrians	-	0	-	-	0	-	-
% Pedestrians	_	-		-	-	_	_



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Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 4



Turning Movement Peak Hour Data Plot (7:45 AM)



Traffic Planning and Design, Inc. 2500 East High Street Suite 650 Pottstown, Pennsylvania, United States 19464 610.326.3100 kyoung@trafficpd.com

Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 5

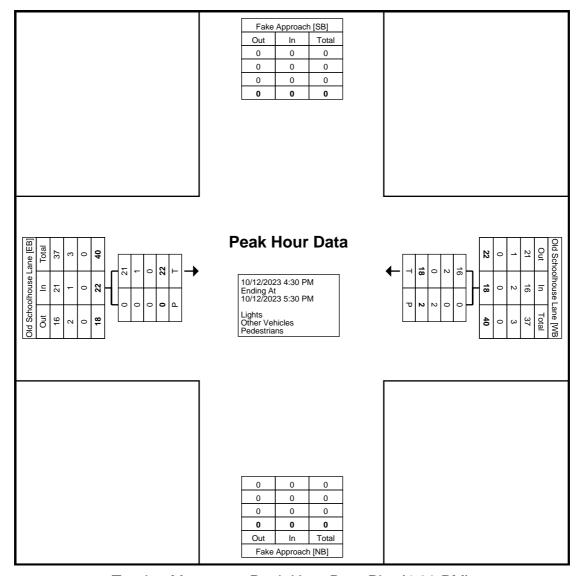
Turning Movement Peak Hour Data (4:30 PM)

		Old Schoolhouse Lar Eastbound	ne	`	ne		
Start Time	Thru	Peds	App. Total	Thru	Peds	App. Total	Int. Total
4:30 PM	4	0	4	6	0	6	10
4:45 PM	3	0	3	4	0	4	7
5:00 PM	9	0	9	6	2	6	15
5:15 PM	6	0	6	2	0	2	8
Total	22	0	22	18	2	18	40
Approach %	100.0	-	-	100.0	-	-	-
Total %	55.0	-	55.0	45.0	-	45.0	-
PHF	0.611	-	0.611	0.750	-	0.750	0.667
Lights	21	-	21	16	-	16	37
% Lights	95.5	-	95.5	88.9	-	88.9	92.5
Other Vehicles	1	-	1	2	-	2	3
% Other Vehicles	4.5	-	4.5	11.1	-	11.1	7.5
Pedestrians	-	0	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0		-



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Count Name: (3) AM/PM Proposed Driveway x Old Schoolhouse Lane Site Code: Start Date: 10/12/2023 Page No: 6



Turning Movement Peak Hour Data Plot (4:30 PM)

Appendix D:Traffic Volume Development Worksheets

11/17/2023

Traffic Volumes Worksheet

Intersection: Synchro Node:

Old Schoolhouse Lane & Cumberland Parkway (West)											
Adjacent intersections:	West		East		North		South				

Time Period: Weekday A.M. Peak Hour

	E	astbour	nd	٧	/estbour	nd	Ν	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		333	14	9	450		10		6				822
2023 Count Heavy Vehicles		13	1	0	28		0		0				42
2023 Existing Volumes (Balanced)	0	333	14	9	450	0	10	0	6	0	0	0	822
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	5	0	0	0	0	0	0	0	9
2025 Base Volumes	0	337	14	9	455	0	10	0	6	0	0	0	831
New Site Trips			14				12						26
Pass-By Trips		-9	9		-13		11						-2
Total Site Trip Distribution	0	-9	23	0	-13	0	23	0	0	0	0	0	24
2025 Projected Volumes	0	328	37	9	442	0	33	0	6	0	0	0	855
Base Growth (0.54% compounded for 12 yrs)	0	22	1	1	30	0	1	0	0	0	0	0	55
2035 Base Volumes	0	355	15	10	480	0	11	0	6	0	0	0	877
2035 Projected Volumes	0	346	38	10	467	0	34	0	6	0	0	0	901
		•		-	•		-				•	•	PHF
Existing (Count) Heavy Vehicles %		4%	7%	0%	6%		0%		0%				0.87

	E	astbour	nd	V	/estbour	nd	N	orthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		353	6	2	650		12		12				1035
2023 Count Heavy Vehicles		3	1	0	11		2		0				17
2023 Existing Volumes (Balanced)	0	353	6	2	650	0	12	0	12	0	0	0	1035
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	7	0	0	0	0	0	0	0	11
2025 Base Volumes	0	357	6	2	657	0	12	0	12	0	0	0	1046
New Site Trips			12				14						26
Pass-By Trips		-7	7		-13		14						1
Total Site Trip Distribution	0	-7	19	0	-13	0	28	0	0	0	0	0	27
2025 Projected Volumes	0	350	25	2	644	0	40	0	12	0	0	0	1073
Base Growth (0.54% compounded for 12 yrs)	0	24	0	0	43	0	1	0	1	0	0	0	69
2035 Base Volumes	0	377	6	2	693	0	13	0	13	0	0	0	1104
2035 Projected Volumes	0	370	25	2	680	0	41	0	13	0	0	0	1131
		-											PHF
Existing (Count) Heavy Vehicles %		1%	17%	0%	2%		17%		0%				0.93

11/17/2023 Traffic Volumes Worksheet Intersection: Synchro Node:

	Old Sch	oolhou	use Lane	e & C	umberlar	nd Pa	arkway	(East)	
2	Adjacent intersections:	West		East	N	orth		South		

Time Period: Weekday A.M. Peak Hour

	E	astbour	ıd	V	/estbour	nd	N	orthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		335	5	13	484		2		14				853
2023 Count Heavy Vehicles		17	0	0	30		0		1				48
2023 Existing Volumes (Balanced)	0	335	5	13	484	0	2	0	14	0	0	0	853
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	5	0	0	0	0	0	0	0	9
2025 Base Volumes	0	339	5	13	489	0	2	0	14	0	0	0	862
New Site Trips				14					13				27
Pass-By Trips		-9		13	-13				8				-1
Total Site Trip Distribution	0	-9	0	27	-13	0	0	0	21	0	0	0	26
2025 Projected Volumes	0	330	5	40	476	0	2	0	35	0	0	0	888
Base Growth (0.54% compounded for 12 yrs)	0	22	0	1	32	0	0	0	1	0	0	0	56
2035 Base Volumes	0	357	5	14	516	0	2	0	15	0	0	0	909
2035 Projected Volumes	0	348	5	41	503	0	2	0	36	0	0	0	935
													PHF
Existing (Count) Heavy Vehicles %		5%	0%	0%	6%		0%		7%				0.87

	E	astboun	ıd	V	/estbour	nd	N	orthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		375	1	16	627		2		21				1042
2023 Count Heavy Vehicles		6	0	2	11		0		1				20
2023 Existing Volumes (Balanced)	0	375	1	16	627	0	2	0	21	0	0	0	1042
Base Growth (0.54% compounded for 2 yrs)	0	4	0	0	7	0	0	0	0	0	0	0	11
2025 Base Volumes	0	379	1	16	634	0	2	0	21	0	0	0	1053
New Site Trips				12					15				27
Pass-By Trips		-7		13	-13				8				1
Total Site Trip Distribution	0	-7	0	25	-13	0	0	0	23	0	0	0	28
2025 Projected Volumes	0	372	1	41	621	0	2	0	44	0	0	0	1081
Base Growth (0.54% compounded for 12 yrs)	0	25	0	1	42	0	0	0	1	0	0	0	69
2035 Base Volumes	0	400	1	17	669	0	2	0	22	0	0	0	1111
2035 Projected Volumes	0	393	1	42	656	0	2	0	45	0	0	0	1139
													PHF
Existing (Count) Heavy Vehicles %		2%	0%	13%	2%		0%		5%				0.95

11/17/2023 Traffic Volumes Worksheet Intersection: Synchro Node:

Old Sc	Old Schoolhouse Lane & Proposed Driveway (West)												
Adjacent intersections:	West	East	North	South									

Time Period: Weekday A.M. Peak Hour

	Е	astbour	ıd	V	/estbour	nd	N	orthbour	nd	Sc	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		11			22								33
2023 Count Heavy Vehicles		1			0								1
2023 Existing Volumes (Balanced)	0	11	0	0	22	0	0	0	0	0	0	0	33
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	11	0	0	22	0	0	0	0	0	0	0	33
New Site Trips	14				3	11				3		9	40
Pass-By Trips	9					13				8		11	41
Total Site Trip Distribution	23	0	0	0	3	24	0	0	0	11	0	20	81
2025 Projected Volumes	23	11	0	0	25	24	0	0	0	11	0	20	114
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	12	0	0	23	0	0	0	0	0	0	0	35
2035 Projected Volumes	23	12	0	0	26	24	0	0	0	11	0	20	116
													PHF
Existing (Count) Heavy Vehicles %	2%	9%			0%	2%				2%		2%	0.90

	E	astbour	ıd	V	/estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		22			18								40
2023 Count Heavy Vehicles		1			2								3
2023 Existing Volumes (Balanced)	0	22	0	0	18	0	0	0	0	0	0	0	40
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	22	0	0	18	0	0	0	0	0	0	0	40
New Site Trips	12				4	9				4		10	39
Pass-By Trips	7					13				8		14	42
Total Site Trip Distribution	19	0	0	0	4	22	0	0	0	12	0	24	81
2025 Projected Volumes	19	22	0	0	22	22	0	0	0	12	0	24	121
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	23	0	0	19	0	0	0	0	0	0	0	42
2035 Projected Volumes	19	23	0	0	23	22	0	0	0	12	0	24	123
													PHF
Existing (Count) Heavy Vehicles %	2%	5%			11%	2%				2%		2%	0.90

2/5/2024

Traffic Volumes Worksheet

Intersection:	
Synchro Node:	

Old Schoolho	ouse L	ane &	Propos	ed Driv	veway	(East)	

4 Adjacent intersections: West East North South

Time Period: Weekday A.M. Peak Hour

	E	astboun	ıd	V	√estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		11			22								33
2023 Count Heavy Vehicles		1			0								1
2023 Existing Volumes (Balanced)	0	11	0	0	22	0	0	0	0	0	0	0	33
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	11	0	0	22	0	0	0	0	0	0	0	33
New Site Trips		3			11	3				10		3	30
Pass-By Trips		8			13								21
Total Site Trip Distribution	0	11	0	0	24	3	0	0	0	10	0	3	51
2025 Projected Volumes	0	22	0	0	46	3	0	0	0	10	0	3	84
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	12	0	0	23	0	0	0	0	0	0	0	35
2035 Projected Volumes	0	23	0	0	47	3	0	0	0	10	0	3	86
		•		•			•	•		•		•	
Existing (Count) Heavy Vehicles %	0%	9%			0%	0%				0%		0%	<u>PHF</u>
2025 Heavy Vehicle %	2%	5%			0%	2%				2%		2%	0.90

Existing (Count) Heavy Vehicles %	0%	9%		0%	0%		0%	0%	PHF
2025 Heavy Vehicle %	2%	5%		0%	2%		2%	2%	0.00
2035 Heavy Vehicle %	2%	4%		0%	2%		2%	2%	0.90

	Е	astboun	ıd	٧	√estbour	nd	N	orthbour	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
2023 Count Volumes		22			18								40
2023 Count Heavy Vehicles		1			2								3
2023 Existing Volumes (Balanced)	0	22	0	0	18	0	0	0	0	0	0	0	40
Base Growth (0.54% compounded for 2 yrs)	0	0	0	0	0	0	0	0	0	0	0	0	0
2025 Base Volumes	0	22	0	0	18	0	0	0	0	0	0	0	40
New Site Trips		4			9	3				11		4	31
Pass-By Trips		8			13								21
Total Site Trip Distribution	0	12	0	0	22	3	0	0	0	11	0	4	52
2025 Projected Volumes	0	34	0	0	40	3	0	0	0	11	0	4	92
Base Growth (0.54% compounded for 12 yrs)	0	1	0	0	1	0	0	0	0	0	0	0	2
2035 Base Volumes	0	23	0	0	19	0	0	0	0	0	0	0	42
2035 Projected Volumes	0	35	0	0	41	3	0	0	0	11	0	4	94

Existing (Count) Heavy Vehicles %	0%	5%		11%	0%		0%	0%	PHF
2025 Heavy Vehicle %	2%	3%		5%	2%		2%	2%	0.90
2035 Heavy Vehicle %	2%	3%		5%	2%		2%	2%	0.90

Appendix E:Capacity and Queue Analysis

Supporting Calculations

Crtical Headway and Follow-Up Headway Calculations

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Development: Genius Kids Upper Allen

Location: Upper Allen Township, Cumberland County, PA

Intersection: Cumberland Parkway & Old Schoolhouse Lane (West)

Major Street: Cumberland Parkway

Minor Street: Old Schoolhouse Lane (West)

CRITICAL HEADWAY

Equation: $t_{c,x} = t_{c,base} + t_{C,HV} P_{HV} + t_{c,G} G - t_{3,LT}$

Where: $t_{c,x} = Critical Headway for Movement X (seconds)$

t_{c,base} = Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

t_{C,HV} = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

P_{HV} = Percent of Heavy Vehicles

t_{c,G} = Roadway Grade Adjustment Factor

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds)
Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

t_{3,LT} = Intersection Geometry Adjustment Factor

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

CRITICAL HEADWAY

	Major/Minor:	IIM	NOR	MAJOR
	Street:	Old Schoolho	use Ln (West)	Cumberland Parkway
	Movement:	NB Left	NB Right	WB Left
	t _{c,base}	7.1	6.2	4.3
	t _{C,HV}	1	1	1
AM	P _{HV}	0	0	0
Peak	t _{c,G}	0.2	0.1	0
Hour	G	-1	-1	1
	t _{3,LT}	0.7	0	0
	t _{c,x}	6.2	6.1	4.3
	t _{c,base}	7.1	6.2	4.3
	t _{C,HV}	1	1	1
PM	P _{HV}	0.17	0	0
Peak	t _{c,G}	0.2	0.1	0
Hour	G	-1	-1	1
	t _{3,LT}	0.7	0	0
	t _{c,x}	6.4	6.1	4.3

FOLLOW-UP HEADWAY

Equation: $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$

Where: t_{fx} = Follow-up Headway for Movement X (seconds)

t_{f,base} = Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

t_{f HV} = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

P_{HV} = Percent of Heavy Vehicles

FOLLOW-UP HEADWAY

	FOLLOW-UP HEA	ADWAY		
	Major/Minor:	MI	NOR	MAJOR
	Street:	Old Schoolho	use Ln (West)	Cumberland Parkway
	Movement:	NB Left	NB Right	WB Left
	t _{f,base}	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
AM	P _{HV}	0	0	0
Peak				
Hour				
	t _{f,x}	3.0	3.1	3.0
	t _{f,base}	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
PM	P _{HV}	0.17	0	0
Peak				
Hour				
	t _{f,x}	3.2	3.1	3.0

Crtical Headway and Follow-Up Headway Calculations

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Development: Genius Kids Upper Allen

Location: Upper Allen Township, Cumberland County, PA

Intersection: Cumberland Parkway & Old Schoolhouse Lane (East)

Major Street: Cumberland Parkway

Minor Street: Old Schoolhouse Lane (West)

CRITICAL HEADWAY

Equation: $t_{c,x} = t_{c,base} + t_{C,HV} P_{HV} + t_{c,G} G - t_{3,LT}$

Where: $t_{c,x} = Critical Headway for Movement X (seconds)$

t_{c.base} = Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

t_{C,HV} = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

P_{HV} = Percent of Heavy Vehicles

t_{c,G} = Roadway Grade Adjustment Factor

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds)
Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

t_{3,LT} = Intersection Geometry Adjustment Factor

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

CRITICAL HEADWAY

	Major/Minor:	MII	NOR	MAJOR
	Street:	Old Schoolho	ouse Ln (East)	Cumberland Parkway
	Movement:	NB Left	NB Right	WB Left
	t _{c,base}	7.1	6.2	4.3
	t _{c,HV}	1	1	1
AM	P _{HV}	0	0.07	0
Peak	t _{c,G}	0.2	0.1	0
Hour	G	-3	-3	2
	t _{3,LT}	0.7	0	0
	t _{c,x}	5.8	6.0	4.3
	t _{c,base}	7.1	6.2	4.3
	t _{c,HV}	1	1	1
PM	P_{HV}	0	0.05	0.13
Peak	t _{c,G}	0.2	0.1	0
Hour	G	-3	-3	2
	t _{3,LT}	0.7	0	0
	t _{c,x}	5.8	6.0	4.4

FOLLOW-UP HEADWAY

Equation: $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$

Where: $t_{f,x}$ = Follow-up Headway for Movement X (seconds)

t_{f,base} = Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

t_{f HV} = Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

P_{HV} = Percent of Heavy Vehicles

FOLLOW-UP HEADWAY

	Major/Minor:	MI	NOR	MAJOR
	Street:	Old Schoolh	ouse Ln (East)	Cumberland Parkway
	Movement:	NB Left	NB Right	WB Left
	t _{f,base}	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
AM	P _{HV}	0	0.07	0
Peak				
Hour				
	t _{f,x}	3.0	3.2	3.0
	$t_{f,base}$	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
PM	P _{HV}	0	0.05	0.13
Peak				
Hour				
	t _{f v}	3.0	3.1	3.1

Crtical Headway and Follow-Up Headway Calculations

Source: Highway Capacity Manual, 6th Edition, Volume 3, Chapter 20.

Genius Kids Upper Allen Development:

Upper Allen Township, Cumberland County, PA Location:

Old Schoolhouse Lane & Proposed Driveways Intersection:

Major Street: Old Schoolhouse Lane Minor Street: **Proposed Driveway**

CRITICAL HEADWAY

Equation: $t_{c.x} = t_{c.base} + t_{C.HV} P_{HV} + t_{c.G} G - t_{3.LT}$

Critical Headway for Movement X (seconds) Where: t_{c,x} =

Base Critical Headway (from PennDOT Publication 46, Exhibit 10-11)

Adjustment Factor for Heavy Vehicles $t_{C,HV} =$

Street with One Lane in Each Direction = 1.0 (seconds)

Street with Two or Three Lanes in Each Direction = 2.0 (seconds)

P_{HV} = Percent of Heavy Vehicles

Roadway Grade Adjustment Factor $t_{c,G} =$

Minor Street Lefts and Throughs = 0.2 (seconds)

Minor Street Rights = 0.1 (seconds) Major Street Lefts = 0.0 (seconds)

G = Percent Grade of Roadway Approach (Expressed as an Interger)

Intersection Geometry Adjustment Factor t_{3,LT} =

(Three Leg Intersection Minor Street Left = 0.7 seconds, all others = 0.0 seconds)

CRITICAL HEADWAY

	Major/Minor:	MII	NOR	MAJOR
	Street:	Propos	ed Dwy	Old Schoolhouse Lane
	Movement:	SB Left	SB Right	EB Left
	t _{c,base}	7.1	6.2	4.3
	t _{c,HV}	1	1	1
AM	P_{HV}	0.02	0.02	0.02
Peak	t _{c,G}	0.2	0.1	0
Hour	G	0	0	1
	t _{3,LT}	0.7	0	0
	t _{c,x}	6.4	6.2	4.3
	t _{c,base}	7.1	6.2	4.3
	t _{C,HV}	1	1	1
PM	P_{HV}	0.02	0.02	0.02
Peak	t _{c,G}	0.2	0.1	0
Hour	G	0	0	1
	t _{3,LT}	0.7	0	0
	t _{c,x}	6.4	6.2	4.3

FOLLOW-UP HEADWAY

Equation: $t_{f,x} = t_{f,base} + t_{f,HV} P_{HV}$

Follow-up Headway for Movement X (seconds) Where: $t_{f,x} =$

> $t_{f,base} =$ Base Follow-up Headway (from PennDOT Pub 46, Exhibit 10-12)

Adjustment Factor for Heavy Vehicles

Street with One Lane in Each Direction = 0.9 (seconds)

Street with Two or Three Lanes in Each Direction = 1.0 (seconds)

Percent of Heavy Vehicles

	FOLLOW-UP HEA	DWAY		
	Major/Minor:	MI	NOR	MAJOR
	Street:	Propos	ed Dwy	Old Schoolhouse Lane
	Movement:	SB Left	SB Right	EB Left
	t _{f,base}	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
AM	P _{HV}	0.02	0.02	0.02
Peak				
Hour				
	t _{f,x}	3.0	3.1	3.0
	t _{f,base}	3.0	3.1	3.0
	t _{f,HV}	0.9	0.9	0.9
PM	P _{HV}	0.02	0.02	0.02
Peak				
Hour				
	t	3.0	3.1	3.0

2023 Existing Conditions Synchro Reports

	-	•	•	•		~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f.		Ť	1	M	
Traffic Volume (vph)	333	14	9	450	10	6
Future Volume (vph)	333	14	9	450	10	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	399	0	10	517	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings
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Control Type: Unsignalized

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK			INDL M	INDIX
Lane Configurations	}	1.1	<u>*</u>	450		c
Traffic Vol, veh/h	333	14	9	450	10	6
Future Vol, veh/h	333	14	9	450	10	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		None
Storage Length	-	-	50	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	383	16	10	517	11	7
Major/Minor	Major1	N	/lajor2		Minor1	
						204
Conflicting Flow All	0	0	399	0	928	391
Stage 1	-	-	-	-	391	-
Stage 2	-	-	-	-	537	-
Critical Hdwy	-	-	4.3	-	6.2	6.1
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	876	-	349	705
Stage 1	-	-	_	-	799	-
Stage 2	_	_	_	-	685	_
Platoon blocked, %	_	_		_	000	
Mov Cap-1 Maneuver	_	_	876	_	345	705
Mov Cap-1 Maneuver	_	_	-	_	490	- 100
	-	-				
Stage 1	-	-	-	-	799	-
Stage 2	-	-	-	-	677	-
Approach	EB		WB		NB	
HCM Control Delay, s/			0.2		11.7	
HCM LOS			J.L		В	
I IOIVI LOO					D	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		553	_	-	876	-
HCM Lane V/C Ratio		0.033	_	_	0.012	-
HCM Control Delay (s/	veh)	11.7	_	_	9.2	_
HCM Lane LOS		В	_	_	A	_
HCM 95th %tile Q (veh	1)	0.1	_	_	0	_
TOW JOHN JOHN Q (VEI	'/	0.1	_		U	_

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1₃		ሻ	1	W	
Traffic Volume (vph)	335	5	13	484	2	14
Future Volume (vph)	335	5	13	484	2	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	391	0	15	556	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Onetral Trans. Heading allers	1					

Lanes, Volumes, Timings C:\Users\dlenker\Desktop\XBAM.syn

Control Type: Unsignalized

0.3					
EBT	EBR	WBL	WBT	NBL	NBR
	5				14
					14
					0
					Stop
					None
					-
					_
					_
					87
					7
					16
300	Ö	13	220	2	10
ajor1		/lajor2	N	Minor1	
0	0	391	0	974	388
-	-	-	-	388	-
-	-	_	_	586	-
_	-	4.3	_		6
_	_	-	_		-
_	_	-	_		-
-	-				3.2
_	_				697
_	_				-
_	-				_
		-		034	_
_	-	QQ1		350	697
-	-				
_	-				-
-	-		-		-
-	-	-	-	682	-
EB		WB		NB	
U		J.L			
				U	
١	VBLn1	EBT	EBR	WBL	WBT
	665	-	-	881	-
	0.028	-	-		-
h)	10.6	-	-	9.2	-
	В	-	-	Α	-
	0.1	-	_	0.1	_
F #	BBT 335 335 0 -ree	### BBT EBR ### BBT BBT	EBT EBR WBL 335 5 13 335 5 13 0 0 0 0 Free Free Free - None 225 01 87 87 87 5 0 0 385 6 15 sijor1 Major2 0 0 391 4.3 3 - 881 3 - 881 881 881 881 881 881 881 881 881 881 881 881 881 881	EBT EBR WBL WBT 335	EBT EBR WBL WBT NBL

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ _a		ሻ	†	, A	
Traffic Volume (vph)	353	6	2	650	12	12
Future Volume (vph)	353	6	2	650	12	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)		1			1	
Confl. Bikes (#/hr)						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	386	0	2	699	26	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Control Type: Unsignalized

Other

Area Type:

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1≯	LDIX	VVDL	<u>₩</u>	₩.	NDIX
Traffic Vol, veh/h	353	6	2	650	12	12
Future Vol, veh/h	353	6	2	650	12	12
	0	1	0	000	12	0
Conflicting Peds, #/hr		•				
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	380	6	2	699	13	13
Major/Minor	oior1		laie-2		Minor	
	ajor1		//ajor2		Minor1	004
Conflicting Flow All	0	0	387	0		384
Stage 1	-	-	-	-	384	-
Stage 2	-	-	-	-	704	-
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	884	-	254	712
Stage 1	_	-	-	_	749	-
Stage 2	_	_	_	_	530	-
Platoon blocked, %		_		_	000	
Mov Cap-1 Maneuver	-	-	883		253	711
Mov Cap-2 Maneuver	-	-	-	-	392	-
Stage 1	-	-	-	-	748	-
Stage 2	-	-	-	-	528	-
Approach	EB		WB		NB	
	0		0		12.5	
HCM Control Delay, s/v	U		U			
HCM LOS					В	
Minor Long/Major Myrot		NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Maior Mwmt		TULII		רטוע	WDL	וטוו
Minor Lane/Major Mvmt					000	
Capacity (veh/h)		505	-	-	883	-
Capacity (veh/h) HCM Lane V/C Ratio		505 0.051	-	-	0.002	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s/ve		505 0.051 12.5	-	-	0.002 9.1	-
Capacity (veh/h) HCM Lane V/C Ratio		505 0.051	-	-	0.002	-

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»		ሻ	†	, A	
Traffic Volume (vph)	375	1	16	627	2	21
Future Volume (vph)	375	1	16	627	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	396	0	17	660	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Control Type: Unsignalized

Other

Area Type:

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	LDIX	ሻ	<u> </u>	Y	NDIX
Traffic Vol, veh/h	375	1	16	627	2	21
Future Vol, veh/h	375	1	16	627	2	21
Conflicting Peds, #/hr	0	0	0	021	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- Clop	None
Storage Length	_	-	225	-	0	-
Veh in Median Storage		_	-	0	0	_
Grade, %	-1	_	_	2	-3	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mymt Flow	395	1	17	660	2	22
IVIVIIIL I IUW	333	ı	17	000		ZZ
Major/Minor	Major1	N	//ajor2		/linor1	
Conflicting Flow All	0	0	396	0	1090	396
Stage 1	-	-	-	-	396	-
Stage 2	-	-	-	-	694	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	-	-	845	-	315	708
Stage 1	_	-	-	-	831	-
Stage 2	-	-	_	-	626	_
Platoon blocked, %	_	-		_		
Mov Cap-1 Maneuver	-	-	845	-	309	708
Mov Cap-2 Maneuver	_	_	-	_	457	-
Stage 1	_	_	_	_	831	_
Stage 2	_	_	_	_	613	_
Olage 2		_	_		010	
Approach	EB		WB		NB	
HCM Control Delay, s/	v 0		0.2		10.5	
HCM LOS					В	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
	IL I					
Capacity (veh/h)		676	-	-	845	-
HCM Caretral Dalace (a)	l l. \	0.036	-	-	0.02	-
HCM Control Delay (s/	ven)	10.5	-	-	9.3	-
HCM Lane LOS	.\	В	-	-	A	-
HCM 95th %tile Q (veh	1)	0.1	-	-	0.1	-

2025 Base (No-Build) Condition Synchro Reports

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4		ሻ		W	
Traffic Volume (vph)	337	14	9	455	10	6
Future Volume (vph)	337	14	9	455	10	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	403	0	10	523	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
O (I.T. III !						

Intersection						
Int Delay, s/veh	0.3					
<u> </u>		EDD	MAID	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽				A	
Traffic Vol, veh/h	337	14	9	455	10	6
Future Vol, veh/h	337	14	9	455	10	6
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	387	16	10	523	11	7
WWITH CIOW	301	10	10	525		
Major/Minor Ma	ajor1	N	/lajor2	I	Minor1	
Conflicting Flow All	0	0	403	0	938	395
Stage 1	-	-	-	-	395	-
					543	
Stage 2	-	-	12	-		6.1
Critical Hdwy	-	-	4.3	-	6.2	
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	873	-	344	702
Stage 1	-	-	-	-	796	-
Stage 2	-	-	-	-	681	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	873	-	340	702
Mov Cap-2 Maneuver	-	-	-	-	486	-
Stage 1	-	-	_	_	796	-
Stage 2	_	_	_	_	674	_
Olago Z		_			014	
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.2		11.8	
HCM LOS	•		J.E		В	
TIOWI LOO					J	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		549	-	-	873	-
HCM Lane V/C Ratio		0.033	_	_	0.012	_
HCM Control Delay (s/ve	h)	11.8	-	-	9.2	-
HCM Lane LOS	,	В	_	_	A	_
HCM 95th %tile Q (veh)		0.1	_	_	0	_
How John Johne Q (Ven)		0.1	_	_	U	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- ↑		7	†	N/	
Traffic Volume (vph)	339	5	13	489	2	14
Future Volume (vph)	339	5	13	489	2	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	396	0	15	562	18	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7>	LDIX	7	<u>₩</u>	¥	ווטוז
Traffic Vol, veh/h	339	5	13	489	2	14
Future Vol, veh/h	339	5	13	489	2	14
		0				0
Conflicting Peds, #/hr			0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storag		-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	390	6	15	562	2	16
Major/Minor	Major1	N	//ajor2		Minor1	
		0	396	0	985	393
Conflicting Flow All	0					
Stage 1	-	-	-	-	393	-
Stage 2	-	-	-	-	592	-
Critical Hdwy	-	-	4.3	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy		-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	878	-	360	692
Stage 1	-	-	-	-	833	-
Stage 2	-	-	-	-	690	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	878	_	354	692
Mov Cap-2 Maneuver		_	-	-	500	-
Stage 1	_	_	_	_	833	_
	_	_	_	_	678	_
Stage 2	-	-	_	-	0/0	-
Approach	EB		WB		NB	
HCM Control Delay, s	/v 0		0.2		10.6	
HCM LOS					В	
3222						
						14/5-
Minor Lane/Major Mvr	nt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		660	-	-	878	-
HCM Lane V/C Ratio		0.028	-	-	0.017	-
HCM Control Delay (s	/veh)	10.6	-	-	9.2	-
HCM Lane LOS	,	В	-	-	Α	-
HCM 95th %tile Q (ve	h)	0.1	_	-	0.1	-
	,	V			J.,	

	-	•	•	•	•	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f.		ሻ	1	M	
Traffic Volume (vph)	357	6	2	657	12	12
Future Volume (vph)	357	6	2	657	12	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)		1			1	
Confl. Bikes (#/hr)						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	390	0	2	706	26	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\XBPM.syn

Intersection						
Int Delay, s/veh	0.3					
		EDD	WEL	MOT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			↑	Y	
Traffic Vol, veh/h	357	6	2	657	12	12
Future Vol, veh/h	357	6	2	657	12	12
Conflicting Peds, #/hr	0	1	0	0	1	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mymt Flow	384	6	2	706	13	13
IVIVIII I IOW	JU4	U		100	10	10
Major/Minor Ma	ajor1	N	/lajor2	N	Minor1	
Conflicting Flow All	0	0	391	0	1099	388
Stage 1	-	-	-	-	388	-
Stage 2	_	_	_	_	711	_
Critical Hdwy	_	_	4.3	_	6.4	6.1
Critical Hdwy Stg 1	_	_	4.5	_	5.37	0.1
, ,	-	-			5.37	
Critical Hdwy Stg 2		-	-	-		2 1
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	881	-	250	708
Stage 1	-	-	-	-	746	-
Stage 2	-	-	-	-	526	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	880	-	249	707
Mov Cap-2 Maneuver	-	-	-	-	389	-
Stage 1	-	-	-	-	745	-
Stage 2	-	-	_	-	524	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0		12.6	
HCM LOS					В	
Minor Long/Maior M.		JDI 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		502	-	-	880	-
HCM Lane V/C Ratio		0.051	-	-	0.002	-
HCM Control Delay (s/ve	eh)	12.6	-	-	9.1	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q (veh)		0.2	-	-	0	-

	-	•	•	•	1	_
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		ሻ	1	W	
Traffic Volume (vph)	379	1	16	634	2	21
Future Volume (vph)	379	1	16	634	2	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	400	0	17	667	24	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\XBPM.syn

Intersection						
Int Delay, s/veh	0.4					
<u> </u>		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		<u> </u>	<u>↑</u>	¥	0.1
	379	1	16	634	2	21
•	379	1	16	634	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
	399	1	17	667	2	22
		_				
	ajor1		/lajor2		Minor1	
Conflicting Flow All	0	0	400	0	1101	400
Stage 1	-	-	-	-	400	-
Stage 2	-	-	-	-	701	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	-	-	842	-	311	705
Stage 1	_	-	-	-	828	-
Stage 2	_	_	_	-	622	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	-	_	842	_	305	705
Mov Cap-2 Maneuver	_	_	-	_	454	-
Stage 1	_	_	_	_	828	_
Stage 2		_	_	_	610	-
Slaye Z	-	-	-	_	010	_
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.2		10.5	
HCM LOS	-		•		В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		673	-	-	842	-
HCM Lane V/C Ratio		0.036	-	-	0.02	-
HCM Control Delay (s/ve	h)	10.5	-	-	9.4	-
HCM Lane LOS		В			Α	_
I TOW LUTTO LOO		D	-	-	А	-

2035 Base (No-Build) Conditions Synchro Reports

Control Type: Unsignalized

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations			ሻ	1	W	
Traffic Volume (vph)	355	15	10	480	11	6
Future Volume (vph)	355	15	10	480	11	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	425	0	11	552	20	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings C:\Users\dlenker\Desktop\XBAM.syn

Intersection						
Int Delay, s/veh	0.3					
Movement	EDT	EDD	WBL	\\/DT	NBL	NBR
	EBT	EBR	WBL	WBT	NBL	NDK
Lane Configurations	∱	15				G
Traffic Vol, veh/h	355	15	10	480	11	6
Future Vol, veh/h	355	15	10	480	11	6
Conflicting Peds, #/hr	_ 0	0	_ 0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	408	17	11	552	13	7
Major/Minar	Mais =1		Ania-O		Min c = 1	
	Major1		//ajor2		Minor1	44-
Conflicting Flow All	0	0	425	0	991	417
Stage 1	-	-	-	-	417	-
Stage 2	-	-	-	-	574	-
Critical Hdwy	-	-	4.3	-	6.2	6.1
Critical Hdwy Stg 1	-		-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	858	-	320	682
Stage 1	-	-	-	-	778	-
Stage 2	-	-	-	-	659	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	858	_	316	682
Mov Cap-2 Maneuver	_	_	-	_	465	-
Stage 1	_	_	_	_	778	_
Stage 2			_	_	650	_
Slaye Z	<u>-</u>	-	-	<u>-</u>	000	<u>-</u>
Approach	EB		WB		NB	
HCM Control Delay, s/\	/ 0		0.2		12.1	
HCM LOS					В	
						=
Minor Lane/Major Mvm	t I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		524	-	-	858	-
HCM Lane V/C Ratio		0.037	-	-	0.013	-
HCM Control Delay (s/v	veh)	12.1	-	-	9.3	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q (veh)	0.1	-	-	0	-

	-	•	•	•	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f _a		7	†	N/	
Traffic Volume (vph)	357	5	14	516	2	15
Future Volume (vph)	357	5	14	516	2	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	416	0	16	593	19	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Intersection Summary

Area Type: Other

Intersection						
Int Delay, s/veh	0.3					
		EDD	WEL	WOT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		7	<u></u>	Y	
Traffic Vol, veh/h	357	5	14	516	2	15
Future Vol, veh/h	357	5	14	516	2	15
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage, #	ŧ 0	-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	410	6	16	593	2	17
IVIVIIIL I IUW	+10	U	10	JJJ		17
Major/Minor Ma	ajor1	N	/lajor2	_	Minor1	
Conflicting Flow All	0	0	416	0	1038	413
Stage 1	-	-	-	-	413	-
Stage 2	_	_	_	-	625	_
	-	-	4.2			
Critical Hdwy	-	-	4.3	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	864	-	337	675
Stage 1	-	-	-	-	818	-
Stage 2	-	-	-	-	669	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	864	-	331	675
Mov Cap-2 Maneuver	-	-	_	-	480	-
Stage 1	-	_	_	-	818	_
Stage 2	_	_	_	_	656	_
Glage Z	_	_	<u>-</u>	-	000	_
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.2		10.8	
HCM LOS	•		7.2		В	
TIOWI LOO						
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		644	-	-	864	-
HCM Lane V/C Ratio		0.03	_	_	0.019	_
HCM Control Delay (s/ve	h)	10.8	_	_	9.2	-
HCM Lane LOS	,	В	_	_	Α.Δ	_
HCM 95th %tile Q (veh)		0.1	_	_	0.1	
How som whe Q (ven)		0.1		-	U. I	_

	-	•	•	•	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- ↑		7	†	XX.	
Traffic Volume (vph)	377	6	2	693	13	13
Future Volume (vph)	377	6	2	693	13	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)		1			1	
Confl. Bikes (#/hr)						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	411	0	2	745	28	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other

Intersection						
Int Delay, s/veh	0.3					
Movement	EDT	EDD	WDL	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†		<u> </u>	↑	¥	
Traffic Vol, veh/h	377	6	2	693	13	13
Future Vol, veh/h	377	6	2	693	13	13
Conflicting Peds, #/hr	0	1	0	0	1	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mymt Flow	405	6	2	745	14	14
	100			1 10	17	17
Major/Minor Ma	ajor1	N	/lajor2	N	Minor1	
Conflicting Flow All	0	0	412	0	1159	409
Stage 1	-	-	-	-	409	-
Stage 2	_	-	_	_	750	_
Critical Hdwy	_	_	4.3	_	6.4	6.1
Critical Hdwy Stg 1	_	_	T.0 -	_	5.37	-
Critical Hdwy Stg 2	-	_	_	-	5.37	-
Follow-up Hdwy	-	_	3	-	3.2	3.1
	-		867		230	689
Pot Cap-1 Maneuver	-	-		-		
Stage 1	-	-	-	-	729	-
Stage 2	-	-	-	-	504	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	866	-	229	688
Mov Cap-2 Maneuver	-	-	-	-	370	-
Stage 1	-	-	-	-	728	-
Stage 2	-	-	-	-	502	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0		12.9	
HCM LOS					В	
Minor Lang/Major Mayort		VIDI ~1	EDT	EDD	WDI	MDT
Minor Lane/Major Mvmt	ſ	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		481	-	-	866	-
HCM Lane V/C Ratio		0.058	-	-	0.002	-
HCM Control Delay (s/ve	eh)	12.9	-	-	9.2	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q (veh)		0.2	-	-	0	-

	-	•	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- 1→		ሻ		W	
Traffic Volume (vph)	400	1	17	669	2	22
Future Volume (vph)	400	1	17	669	2	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	422	0	18	704	25	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\XBPM.syn

I. (
Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		ች		¥	
Traffic Vol, veh/h	400	1	17	669	2	22
Future Vol, veh/h	400	1	17	669	2	22
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storag	ge,# 0	-	-	0	0	-
Grade, %	-1	_	_	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	421	1	18	704	2	23
		•		701	_	
		_		_		
Major/Minor	Major1		//ajor2		Minor1	
Conflicting Flow All	0	0	422	0	1162	422
Stage 1	-	-	-	-	422	-
Stage 2	-	-	-	-	740	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3.1	-	3	3.1
Pot Cap-1 Maneuver	-	-	827	-	288	686
Stage 1	-	-	-	-	811	-
Stage 2	_	-	-	-	599	-
Platoon blocked, %	-	_		_		
Mov Cap-1 Maneuve	r -	-	827	_	282	686
Mov Cap-2 Maneuve		_	-	_	434	-
Stage 1	_	_	_	_	811	_
Stage 2		_	_	_	586	_
Olage 2			_		500	
Approach	EB		WB		NB	
HCM Control Delay,	s/v 0		0.2		10.7	
HCM LOS					В	
Minor Long /Mais a M	una h	NDL 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mv	mt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		654	-	-	827	-
HCM Lane V/C Ratio		0.039	-	-	0.022	-
HCM Control Delay (s/veh)	10.7	-	-	9.4	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q (ve	eh)	0.1	-	-	0.1	-

2025 Projected (Build) Conditions Synchro Reports

	-	•	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4		ሻ		W	
Traffic Volume (vph)	328	37	9	442	33	6
Future Volume (vph)	328	37	9	442	33	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	420	0	10	508	45	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
0 (i + 11) "	1					

Lanes, Volumes, Timings C:\Users\dlenker\Desktop\PAM.syn

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		ሻ	†	W	
Traffic Vol, veh/h	328	37	9	442	33	6
Future Vol, veh/h	328	37	9	442	33	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length	_	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	# 0 -2				-1	
		- 07	- 07	1		- 07
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	377	43	10	508	38	7
Major/Minor M	lajor1	Λ	/lajor2		Minor1	
Conflicting Flow All	0	0	420	0	927	399
					399	
Stage 1	-	-	-	-		-
Stage 2	-	-	4.0	-	528	- 0.4
Critical Hdwy	-	-	4.3	-	6.2	6.1
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	861	-	349	698
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	692	-
Platoon blocked, %	_	-		-		
Mov Cap-1 Maneuver	_	_	861	_	345	698
Mov Cap-2 Maneuver	_	_	-	_	491	-
Stage 1	_		_	_	793	_
Stage 2	_		_	_	684	_
Slaye 2	-	-	-	-	004	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.2		12.7	
HCM LOS	U		0.2		В	
I IOIVI LOO					ט	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		514	-	_	861	-
HCM Lane V/C Ratio		0.087	_		0.012	_
HCM Control Delay (s/ve	eh)	12.7	_	_	9.2	_
HCM Lane LOS	on)	12.7 B	_		9.2 A	_
HCM 95th %tile Q (veh)		0.3		-	0	
HOW Sour Mille Q (ven)		0.5	-	-	U	

	-	•	•	•		~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		7	^	***	
Traffic Volume (vph)	330	5	40	476	2	35
Future Volume (vph)	330	5	40	476	2	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	385	0	46	547	42	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>\$</u>	LDIX	7	<u>₩</u>	¥	ווטוו
Traffic Vol, veh/h	330	5		476		35
		5	40 40		2	35
Future Vol, veh/h	330	5		476		
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	379	6	46	547	2	40
Major/Minor	Major1		/laior?		Minor1	
	Major1		//ajor2			000
Conflicting Flow All	0	0	385	0	1021	382
Stage 1	-	-	-	-	382	-
Stage 2	-	-	-	-	639	-
Critical Hdwy	-	-	4.3	-	5.8	5.9
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	886	-	344	729
Stage 1	_	_	-	_	842	-
Stage 2	_	_	_	_	660	_
Platoon blocked, %	_	-	_	_	000	_
	-		206		306	720
Mov Cap-1 Maneuver		-	886	-	326	729
Mov Cap-2 Maneuver		-	-	-	471	-
Stage 1	-	-	-	-	842	-
Stage 2	-	-	-	-	626	-
Approach	EB		WB		NB	
HCM Control Delay, s.			0.7		10.4	
	/V U		0.7			
HCM LOS					В	
Minor Lane/Major Mvr	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		708		-	886	-
HCM Lane V/C Ratio		0.06	_		0.052	-
	(vob)	10.4			9.3	
HCM Long LOS	(Veil)		-	-		-
HCM Lane LOS	. \	В	-	-	A	-
HCM 95th %tile Q (ve	n)	0.2	-	-	0.2	-

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	•	-	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	- 1>		W	
Traffic Volume (vph)	23	11	25	24	11	20
Future Volume (vph)	23	11	25	24	11	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	30		30	
Link Distance (ft)		948	144		107	
Travel Time (s)		25.9	3.3		2.4	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	38	55	0	34	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Intersection						
Int Delay, s/veh	4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	EDL			WDR	SDL W	SDN
Traffic Vol, veh/h	23	र्व 11	1→ 25	24	11	20
Future Vol, veh/h	23	11	25	24	11	20
Conflicting Peds, #/hr	23	0	25 0	0	0	0
•	-	Free	Free	Free		_
Sign Control RT Channelized	Free -	None		None	Stop	Stop None
			-		-	
Storage Length		-	-	-	0	-
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	26	12	28	27	12	22
Major/Minor N	/lajor1	Λ	Major2	N	Minor2	
Conflicting Flow All	55	0	-	0	106	42
Stage 1	-	-	_	-	42	-
Stage 2	_	_	_	_	64	_
Critical Hdwy	4.3	-		-	6.42	6.22
Critical Hdwy Stg 1	4.5				5.42	0.22
, ,	-	-	-	-	5.42	
Critical Hdwy Stg 2	-	-	-	-		- 2.1
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1150	-	-	-	1038	1100
Stage 1	-	-	-	-	1146	-
Stage 2	-	-	-	-	1119	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1150	-	-	-	1014	1100
Mov Cap-2 Maneuver	-	-	-	-	1014	-
Stage 1	-	-	-	-	1120	-
Stage 2	-	-	-	-	1119	-
Approach	EB		WB		SB	
HCM Control Delay, s/v	5.5		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1150	-	-		1068
HCM Lane V/C Ratio		0.022	_	_		0.032
HCM Control Delay (s/v	eh)	8.2	0	-	_	8.5
HCM Lane LOS	511)	Α	A			Α
HCM 95th %tile Q (veh)	١	0.1	-	_	_	0.1
HOW JOHN JOHN W (VEII)	1	0.1		_		0.1

HCM 6th TWSC C:\Users\dlenker\Desktop\PAM.syn

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4	- 1>		W		
Traffic Volume (vph)	0	22	46	3	10	3	
Future Volume (vph)	0	22	46	3	10	3	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	16	16	16	16	12	12	
Grade (%)		1%	-1%		0%		
Storage Length (ft)	0			0	0	0	
Storage Lanes	0			0	1	0	
Taper Length (ft)	25				25		
Link Speed (mph)		25	25		25		
Link Distance (ft)		144	476		112		
Travel Time (s)		3.9	13.0		3.1		
Confl. Peds. (#/hr)							
Confl. Bikes (#/hr)							
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	
Parking (#/hr)							
Mid-Block Traffic (%)		0%	0%		0%		
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	24	54	0	14	0	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL	4	₩ <u>₩</u>	WDIX	Ŋ.	אומט
Lane Configurations	0	22	46	2	10	3
Traffic Vol. veh/h		22	46	3	10	
Future Vol, veh/h	0			3		3
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	110110	-	None		None
Storage Length	-	-	-	-	0	-
Veh in Median Storag	e,# -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	0	24	51	3	11	3
Major/Minor	Major1	N	Major2	N	Minor2	
						F2
Conflicting Flow All	54	0	-	0	77	53
Stage 1	-	-	-	-	53	-
Stage 2	-	-	-	-	24	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1			-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1151	-	-	-	1080	1084
Stage 1	-	-	_	_	1133	-
Stage 2	-	_	_	_	1169	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1151	_	_	_	1080	1084
Mov Cap-1 Maneuver		-	-	_	1080	1004
•		-		-		
Stage 1	-	-	-	-	1133	-
Stage 2	-	-	-	-	1169	-
Approach	EB		WB		SB	
HCM Control Delay, s			0		8.4	
HCM LOS	.,.				A	
TIOWI LOO						
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1151	-	-	-	1081
HCM Lane V/C Ratio		-	_	_		0.013
HCM Control Delay (s	s/veh)	0	-	-	-	8.4
HCM Lane LOS		A	_	_	_	A
HCM 95th %tile Q (ve	h)	0	_	_	_	0
HOW JOHN JOHNE Q (VE	111)	U	_	_		U

Control Type: Unsignalized

	-	•	•	•	•	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4		ሻ		W	
Traffic Volume (vph)	350	25	2	644	40	12
Future Volume (vph)	350	25	2	644	40	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)		1			1	
Confl. Bikes (#/hr)						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	403	0	2	692	56	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

Intersection						
Int Delay, s/veh	0.7					
		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	0.5	<u></u>	^	¥	40
Traffic Vol, veh/h	350	25	2	644	40	12
Future Vol, veh/h	350	25	2	644	40	12
Conflicting Peds, #/hr	0	_ 1	_ 0	0	1	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mvmt Flow	376	27	2	692	43	13
				_		
	ajor1		//ajor2		Minor1	
Conflicting Flow All	0	0	404	0	1088	391
Stage 1	-	-	-	-	391	-
Stage 2	-	-	-	-	697	-
Critical Hdwy	-	-	4.3	-	6.4	6.1
Critical Hdwy Stg 1	-	-	-	-	5.37	-
Critical Hdwy Stg 2	-	-	-	-	5.37	-
Follow-up Hdwy	-	-	3	-	3.2	3.1
Pot Cap-1 Maneuver	-	-	872	-	254	705
Stage 1	_	_	_	_	743	-
Stage 2	_	_	_	-	534	-
Platoon blocked, %	_	_		_	- JO 1	
Mov Cap-1 Maneuver	_	_	871	_	253	704
Mov Cap-1 Maneuver	_	_	- 07 1	_	393	- 704
Stage 1		_	_		742	-
	-		-	-		
Stage 2	-	-	-	-	532	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0		14.4	
HCM LOS			- 0		В	
TIOWI LOO					U	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		438	-	-	871	-
HCM Lane V/C Ratio		0.128	_	-	0.002	-
HCM Control Delay (s/ve	h)	14.4	-	-	9.1	-
HCM Lane LOS	,	В	_	_	A	_
HCM 95th %tile Q (veh)		0.4	_	-	0	-
		J .¬			- 0	

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- 1		ň		W	
Traffic Volume (vph)	372	1	41	621	2	44
Future Volume (vph)	372	1	41	621	2	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	393	0	43	654	48	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
0 (1						

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			1	¥	
Traffic Vol, veh/h	372	1	41	621	2	44
Future Vol, veh/h	372	1	41	621	2	44
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storag	ie.# 0	-	-	0	0	-
Grade, %	-1	-	_	2	-3	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mvmt Flow	392	1	43	654	2	46
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	393	0	1133	393
Stage 1	-	-	-	-	393	-
Stage 2	-	-	-	-	740	-
Critical Hdwy	-	-	4.4	-	5.8	6
Critical Hdwy Stg 1		-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy		-	3.1	-	3	3.1
Pot Cap-1 Maneuver	_	-	847	-	299	711
Stage 1	-	-	-	-	833	-
Stage 2	-	-	-	-	599	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	847	-	284	711
Mov Cap-2 Maneuver		-	-	-	430	-
Stage 1	-	_	_	_	833	-
Stage 2	_	_	_	_	568	_
Jugo 2					500	
Approach	EB		WB		NB	
HCM Control Delay, s	s/v 0		0.6		10.6	
HCM LOS					В	
Minor Lane/Major Mv	mt I	NBLn1	EBT	EBR	WBL	WBT
	int I					
Capacity (veh/h) HCM Lane V/C Ratio		691	-	-	847	-
	(vob)	0.07	-	-	0.051	-
HCM Long LOS	orveil)	10.6	-		9.5	-
HCM Lane LOS	. L \	В	-	-	A	-
HCM 95th %tile Q (ve	en)	0.2	-	-	0.2	-

Fiming Plan: PM Peak Hour

3: Old Schoolhouse Lane (West)/Old Schoolhouse Lane & Proposed Site Driveway (West)

Tilling Flant. Fly	I CUIT I IO	<u> </u>	2. 3.4 0			
	٠	→	←	*	/	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ર્ન	4		W	
Traffic Volume (vph)	19	22	22	22	12	24
Future Volume (vph)	19	22	22	22	12	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	30		30	
Link Distance (ft)		948	144		107	
Travel Time (s)		25.9	3.3		2.4	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	45	48	0	40	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EDT	WDT	WDD	CDI	SBR
	EBL	EBT	WBT	WBR	SBL	SBK
Lane Configurations		4	ĵ.		Y	
Traffic Vol, veh/h	19	22	22	22	12	24
Future Vol, veh/h	19	22	22	22	12	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-	None	· -	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	.# -	0	0	_	0	_
Grade, %	-, π	1	-1	<u>-</u>	0	_
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	21	24	24	24	13	27
Major/Minor I	Major1	N	//ajor2	N	Minor2	
Conflicting Flow All	48	0	- najorz	0	102	36
Stage 1					36	-
•	-	-	-	-		
Stage 2	-	-	-	-	66	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-		-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1156	-	_	-	1044	1108
Stage 1	-	_	_	_	1154	-
Stage 2					1117	_
	-	-		-	1117	-
Platoon blocked, %	4450	-	-	-	1005	4400
Mov Cap-1 Maneuver	1156	-	-	-	1025	1108
Mov Cap-2 Maneuver	-	-	-	-	1025	-
Stage 1	-	-	-	-	1133	-
Stage 2	-	-	-	-	1117	-
Approach	ED		WD		CD	
Approach	EB		WB		SB	
HCM Control Delay, s/	v 3.8		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBI n1
		1156				
Capacity (veh/h)			-	-		1079
HCM Lane V/C Ratio		0.018	-	-	-	0.037
HCM Control Delay (s/	veh)	8.2	0	-	-	8.5
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q (veh	1)	0.1	-	-	-	0.1
,						

HCM 6th TWSC Synchro 12 Report 02/05/2024 4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

Tilling Flatt. Flyi i	cakino	41	1. 014	30110011101	200 201107	Old Colle
	٠	-	←	•	\	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	1≽		W	
Traffic Volume (vph)	0	34	40	3	11	4
Future Volume (vph)	0	34	40	3	11	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	25		25	
Link Distance (ft)		144	476		112	
Travel Time (s)		3.9	13.0		3.1	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	38	47	0	16	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	1	וטוי	N/	ODIN
Traffic Vol, veh/h	0	34	40	3	11	4
Future Vol, veh/h	0	34	40	3	11	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized				None		None
Storage Length	-	NOHE -	-	NOHE -	0	INOHE -
Veh in Median Storage	- + +	0	0		0	-
	, 	1	-1		0	
Grade, %	-			-		-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	0	38	44	3	12	4
Major/Minor	Major1	N	Major2	I	Minor2	
Conflicting Flow All	47	0	-	0	84	46
Stage 1	-	-	-	-	46	-
Stage 2	_	-	-	_	38	_
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1			_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	-	5.42	_
Follow-up Hdwy	3	_	_	<u>-</u>	3	3.1
Pot Cap-1 Maneuver	1157		_	_	1070	1094
Stage 1	1101		-	_	1141	1034
Stage 2	_	-	_		1151	-
	-	_		-	1101	-
Platoon blocked, %	1157	-	-	-	1070	1004
Mov Cap-1 Maneuver	1157	-	-	-	1070	1094
Mov Cap-2 Maneuver	-	-	-	-	1070	-
Stage 1	-	-	-	-	1141	-
Stage 2	-	-	-	-	1151	-
Approach	EB		WB		SB	
HCM Control Delay, s/			0		8.4	
HCM LOS	. ,				A	
					,\	
				\	14/5-	.
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	
Capacity (veh/h)		1157	-	-		1076
HCM Lane V/C Ratio		-	-	-	-	0.015
HCM Control Delay (s/	veh)	0	-	-	-	8.4
HCM Lane LOS		Α	-	-	-	Α

HCM 6th TWSC Synchro 12 Report 02/05/2024 c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

2035 Projected (Build) Conditions Synchro Reports

	-	•	•	•	•	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		ሻ	1	N/	
Traffic Volume (vph)	346	38	10	467	34	6
Future Volume (vph)	346	38	10	467	34	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	7%	0%	6%	0%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	442	0	11	537	46	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Lanes, Volumes, Timings C:\Users\dlenker\Desktop\PAM.syn

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LUIX	YVDL	<u>₩</u>	₩.	NON
Traffic Vol, veh/h	346	38	10	T 467	T 34	6
Future Vol, veh/h	346	38	10	467	34	6
Conflicting Peds, #/hr	0	0	0	0	O Cton	O Ctop
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	4	7	0	6	0	0
Mvmt Flow	398	44	11	537	39	7
Major/Minor Major/Minor	ajor1	N	/lajor2	N	Minor1	
		0	442			420
Conflicting Flow All	0			0	979	420
Stage 1	-	-	-	-	420	-
Stage 2	-	-	-	-	559	-
Critical Hdwy	-	-	4.3	-	6.2	6.1
Critical Hdwy Stg 1	-	-	-	-	5.2	-
Critical Hdwy Stg 2	-	-	-	-	5.2	-
Follow-up Hdwy	-	-	3	-	3	3.1
Pot Cap-1 Maneuver	-	-	846	-	325	679
Stage 1	-	-	-	-	775	-
Stage 2	-	-	-	-	669	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	_	846	_	321	679
Mov Cap-1 Maneuver	_	_	-	_	470	-
	-					
Stage 1	-	-	-	-	775	-
Stage 2	-	-	-	-	660	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.2		13.1	
HCM LOS	U		0.2		В	
TIOIVI LOG					D	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		493	-	_	846	-
HCM Lane V/C Ratio		0.093	_	_	0.014	-
HCM Control Delay (s/ve	eh)	13.1	-	-	9.3	-
HCM Lane LOS	.,	В	_	_	A	-
HCM 95th %tile Q (veh)		0.3	_	_	0	_
HOW JOHN JOHN & (VEII)		0.0			U	

	-	•	•	•		-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		ሻ	†	W	
Traffic Volume (vph)	348	5	41	503	2	36
Future Volume (vph)	348	5	41	503	2	36
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	5%	0%	0%	6%	0%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	406	0	47	578	43	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7>	LDIX	YVDL T	<u>₩</u>	₩.	NDIX
Traffic Vol, veh/h	348	5	41	503	2	36
Future Vol, veh/h	348	5	41	503	2	36
		0	0		0	
Conflicting Peds, #/hr				0		O Cton
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storag		-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	5	0	0	6	0	7
Mvmt Flow	400	6	47	578	2	41
Major/Minor	Major1	N	//ajor2		Minor1	
	0	0	406	0	1075	403
Conflicting Flow All						
Stage 1	-	-	-	-	403	-
Stage 2	-	-	-	-	672	-
Critical Hdwy	-	-	4.3	-	5.8	6
Critical Hdwy Stg 1	-	-	-	-	4.8	-
Critical Hdwy Stg 2	-	-	-	-	4.8	-
Follow-up Hdwy		-	3	-	3	3.2
Pot Cap-1 Maneuver	-	-	871	-	321	684
Stage 1	-	-	-	-	825	-
Stage 2	-	-	-	-	640	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	871	-	304	684
Mov Cap-2 Maneuver		-	-	_	452	-
Stage 1	_	_	_	_	825	_
Stage 2		_	_	_	605	_
Slaye Z	-	<u>-</u>	_	-	000	<u>-</u>
Approach	EB		WB		NB	
HCM Control Delay, s	s/v 0		0.7		10.8	
HCM LOS					В	
J						
						=
Minor Lane/Major Mv	mt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		666	-	-	871	-
HCM Lane V/C Ratio		0.066	-	-	0.054	-
HCM Control Delay (s	s/veh)	10.8	-	-	9.4	-
HCM Lane LOS	,	В	-	-	Α	-
HCM 95th %tile Q (ve	eh)	0.2	-	-	0.2	-
	7					

Tilling Figure 7 (WFF				_	· ·	
	•	\rightarrow	•	•	-	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	- ↑		A.	
Traffic Volume (vph)	23	12	26	24	11	20
Future Volume (vph)	23	12	26	24	11	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	30		30	
Link Distance (ft)		948	144		107	
Travel Time (s)		25.9	3.3		2.4	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	39	56	0	34	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	t					

Lanes, Volumes, Timings C:\Users\dlenker\Desktop\PAM.syn

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LUL	4	₩	אטוע	¥/	אופט
Traffic Vol, veh/h	23	12	26	24	11	20
Future Vol, veh/h	23	12	26	24	11	20
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None		None
Storage Length		-	_	-	0	-
Veh in Median Storage,	# -	0	0	-	0	
Grade, %	π -	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	90	90	2	2	2
Mvmt Flow	26	13	29	27	12	22
IVIVIIIL FIOW	20	13	29	21	12	ZZ
Major/Minor M	lajor1	N	//ajor2	<u> </u>	Minor2	
Conflicting Flow All	56	0	-	0	108	43
Stage 1	-	-	-	-	43	-
Stage 2	-	-	-	-	65	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	_	-	-	3	3.1
	1149	-	-	-	1035	1098
Stage 1	_	_	-	-	1145	-
Stage 2	_	-	-	-	1118	-
Platoon blocked, %		_	_	-		
	1149	-	-	-	1011	1098
Mov Cap-2 Maneuver	-	_	_	_	1011	-
Stage 1	_	_	-	_	1119	-
Stage 2	_	_	_	_	1118	_
Olugo Z					1110	
Approach	EB		WB		SB	
HCM Control Delay, s/v	5.4		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR S	SBI n1
Capacity (veh/h)		1149	LDI	וטיי		1065
HCM Lane V/C Ratio		0.022	-	-		0.032
	oh)		-	-		
HCM Long LOS	en)	8.2	0	-	-	8.5
HCM Lane LOS HCM 95th %tile Q (veh)		Α	Α	-	-	Α
HUM 95TD %THE (J (Veh)		0.1	-	-	-	0.1

4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

	۶	→	+	•	\	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	- ↑		W	
Traffic Volume (vph)	0	23	47	3	10	3
Future Volume (vph)	0	23	47	3	10	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	25		25	
Link Distance (ft)		144	476		112	
Travel Time (s)		3.9	13.0		3.1	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	9%	0%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	26	55	0	14	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					

Area Type: Control Type: Unsignalized

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4		אטול	ÿ.	אופט
Traffic Vol, veh/h	0	23	1→ 47	3	10	3
Future Vol, veh/h	0	23	47	3	10	3
· · · · · · · · · · · · · · · · · · ·	0	23	0	0	0	0
Conflicting Peds, #/hr		Free	Free	Free		
Sign Control RT Channelized	Free				Stop	Stop
	-	None	-	None	-	None
Storage Length		-	-	-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	9	0	2	2	2
Mvmt Flow	0	26	52	3	11	3
Major/Minor	Major1	N	Major2	N	Minor2	
Conflicting Flow All	55	0	-	0	80	54
Stage 1	-	-	-	-	54	-
Stage 2	<u>-</u>	_	_	_	26	_
Critical Hdwy	4.3	_	_	_	6.42	6.22
Critical Hdwy Stg 1	7.0	_	_	_	5.42	- 0.22
Critical Hdwy Stg 1	-		_	_	5.42	-
Follow-up Hdwy	3	-		_	3.42	3.1
Pot Cap-1 Maneuver	1150	_	-		1076	1083
•	1130			-	1131	1005
Stage 1	-	-	-			
Stage 2	-	-	-	-	1166	-
Platoon blocked, %	4450	-	-	-	4070	4000
Mov Cap-1 Maneuver	1150	-	-	-	1076	1083
Mov Cap-2 Maneuver	-	-	-	-	1076	-
Stage 1	-	-	-	-	1131	-
Stage 2	-	-	-	-	1166	-
Approach	EB		WB		SB	
HCM Control Delay, s/			0		8.4	
HCM LOS	. 0				Α	
1 TOWN LOO						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1150	-	-	-	1078
HCM Lane V/C Ratio		-	-	-	-	0.013
HCM Control Delay (s/	veh)	0	-	-	-	8.4
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q (veh	1)	0	-	-	-	0
	,					

	-	•	•	•		_
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»		, j		, A	
Traffic Volume (vph)	370	25	2	680	41	13
Future Volume (vph)	370	25	2	680	41	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	16	14	16	16
Grade (%)	-2%			1%	-1%	
Storage Length (ft)		0	50		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	307			1469	293	
Travel Time (s)	6.0			28.6	8.0	
Confl. Peds. (#/hr)		1			1	
Confl. Bikes (#/hr)						
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	17%	0%	2%	17%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	425	0	2	731	58	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other

Control Type: Unsignalized

Intersection						
Int Delay, s/veh	0.7					
		EDD	WDL	WDT	NDI	NDD
	EBT.	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	0.5	<u></u>	^	Y	40
Traffic Vol, veh/h	370	25	2	680	41	13
Future Vol, veh/h	370	25	2	680	41	13
Conflicting Peds, #/hr	0	1	0	0	1	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	50	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	-2	-	-	1	-1	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	1	17	0	2	17	0
Mymt Flow	398	27	2	731	44	14
WWW. TOW	330	LI		701	77	17
Major/Minor Ma	ajor1	١	/lajor2	١	Minor1	
Conflicting Flow All	0	0	426	0	1149	413
Stage 1	-	-	-	-	413	-
Stage 2	_	_	_	_	736	_
Critical Hdwy	_	_	4.3	_	6.4	6.1
Critical Hdwy Stg 1	_	_		-	5.37	-
Critical Hdwy Stg 2		_	_	-	5.37	
Follow-up Hdwy	_	_	3	-	3.2	3.1
	-	_				685
Pot Cap-1 Maneuver	_	-	857	-	233	
Stage 1	-	-	-	-	726	-
Stage 2	-	-	-	-	511	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	856	-	232	684
Mov Cap-2 Maneuver	-	_	-	-	374	-
Stage 1	-	-	-	-	725	-
Stage 2	-	-	_	-	509	-
					500	
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0		14.9	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		420	-	-	856	-
HCM Lane V/C Ratio		0.138	-	-	0.003	-
HCM Control Delay (s/ve	h)	14.9	-	-	9.2	-
HCM Lane LOS	,	В	_	-	Α	-
HCM 95th %tile Q (veh)		0.5	-	-	0	-
		7.0			,	

	-	•	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	- 1		ሻ	↑	**	
Traffic Volume (vph)	393	1	42	656	2	45
Future Volume (vph)	393	1	42	656	2	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	16	12	16	16
Grade (%)	-1%			2%	-3%	
Storage Length (ft)		0	225		0	0
Storage Lanes		0	1		1	0
Taper Length (ft)			25		25	
Link Speed (mph)	35			35	25	
Link Distance (ft)	1469			327	476	
Travel Time (s)	28.6			6.4	13.0	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	13%	2%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	415	0	44	691	49	0
Sign Control	Free			Free	Stop	
Intersection Summary						

Area Type: Other

Control Type: Unsignalized

L. C						
Intersection	0.0					
Int Delay, s/veh	8.0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ		ች	1	¥	
Traffic Vol, veh/h	393	1	42	656	2	45
Future Vol, veh/h	393	1	42	656	2	45
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	225	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	-1	-	-	2	-3	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	2	0	5
Mymt Flow	414	1	44	691	2	47
WWW.	717		77	001		- 1
	ajor1	N	/lajor2	ľ	Minor1	
Conflicting Flow All	0	0	415	0	1194	415
Stage 1	-	-	-	-	415	-
Stage 2	-	_	-	-	779	-
Critical Hdwy	_	-	4.4	_	5.8	6
Critical Hdwy Stg 1	_	_		_	4.8	-
Critical Hdwy Stg 2	_	_	_	_	4.8	_
Follow-up Hdwy	_	_	3.1	_	3	3.1
Pot Cap-1 Maneuver		_	832	_	277	692
Stage 1	_		002	-	816	- 092
Stage 1	_	-			577	
	_	-	<u>-</u>	-	311	-
Platoon blocked, %	-	-	000	-	000	coo
Mov Cap-1 Maneuver	-	-	832	-	262	692
Mov Cap-2 Maneuver	-	-	-	-	411	-
Stage 1	-	-	-	-	816	-
Stage 2	-	-	-	-	546	-
Approach	EB		WB		NB	
HCM Control Delay, s/v	0		0.6		10.8	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		672			832	-
HCM Lane V/C Ratio		0.074	_		0.053	_
HCM Control Delay (s/ve	ah)	10.8			9.6	_
HCM Lane LOS	511)	В	_	_	9.0 A	_
HCM 95th %tile Q (veh)		0.2			0.2	
HOW Sour Mille Q (ven)		U.Z	-	-	0.2	_

Timing Plan: PM Peak Hour 3: Old Schoolhouse Lane (West)/Old Schoolhouse Lane & Proposed Site Driveway (West)

	٦	—	+	•	/	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	- 1>		N/	
Traffic Volume (vph)	19	23	23	22	12	24
Future Volume (vph)	19	23	23	22	12	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	30		30	
Link Distance (ft)		948	144		107	
Travel Time (s)		25.9	3.3		2.4	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	47	50	0	40	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	ĵ.		W	
Traffic Vol, veh/h	19	23	23	22	12	24
Future Vol, veh/h	19	23	23	22	12	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	_	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	21	26	26	24	13	27
N. 1. (N. 4)						
	Major1		//ajor2		Minor2	
Conflicting Flow All	50	0	-	0	106	38
Stage 1	-	-	-	-	38	-
Stage 2	-	-	-	-	68	-
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1154	-	-	-	1038	1105
Stage 1	-	-	-	-	1151	-
Stage 2	-	-	-	-	1114	-
Platoon blocked, %		-	_	-		
Mov Cap-1 Maneuver	1154	-	-	-	1019	1105
Mov Cap-2 Maneuver		_	_	_	1019	-
Stage 1	_	_	_	_	1130	_
Stage 2	_			_	1114	_
Glaye Z	-	_	_	_	1114	-
Approach	EB		WB		SB	
HCM Control Delay, s/	v 3.7		0		8.5	
HCM LOS					Α	
NA'		ED:	FDT	MET	MES	2DL 4
Minor Lane/Major Mvm	<u>it</u>	EBL	EBT	WBT	WBR S	
Capacity (veh/h)		1154	-	-		1075
HCM Lane V/C Ratio		0.018	-	-	-	0.037
HCM Control Delay (s/	veh)	8.2	0	-	-	8.5
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q (veh	1)	0.1	-	-	-	0.1
,						

HCM 6th TWSC Synchro 12 Report 02/05/2024 4: Old Schoolhouse Lane/Old Schoolhouse Lane (East) & Proposed Site Driveway (East)

Tilling Flatt. Flyi i	cakino	41	1. 014	3011001110	400 Edilo	Old Colle
	٠	-	←	•	\	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	ĵ»		W	
Traffic Volume (vph)	0	35	41	3	11	4
Future Volume (vph)	0	35	41	3	11	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	16	16	16	16	12	12
Grade (%)		1%	-1%		0%	
Storage Length (ft)	0			0	0	0
Storage Lanes	0			0	1	0
Taper Length (ft)	25				25	
Link Speed (mph)		25	25		25	
Link Distance (ft)		144	476		112	
Travel Time (s)		3.9	13.0		3.1	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	5%	11%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	39	49	0	16	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					

Lanes, Volumes, Timings c:\pwworking\projectwise\tpd_dlenker\d2296380\PPM.syn

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL			אטוע	→ N	ומט
Lane Configurations Traffic Vol, veh/h	0	€	∱	2	'T' 11	1
	0	35	41	3		4
Future Vol, veh/h	0	35	41	3	11	4
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	1	-1	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	5	11	2	2	2
Mvmt Flow	0	39	46	3	12	4
Major/Minor I	Major1	N	Major2	N	/linor2	
Conflicting Flow All	49	0	viajoiz -	0	87	48
Stage 1	49			-	48	-
_		-	-		39	
Stage 2	4.2	-	-	-		6.00
Critical Hdwy	4.3	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	3	-	-	-	3	3.1
Pot Cap-1 Maneuver	1155	-	-	-	1065	1091
Stage 1	-	-	-	-	1139	-
Stage 2	-	-	-	-	1150	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1155	-	-	-	1065	1091
Mov Cap-2 Maneuver	-	-	-	-	1065	-
Stage 1	-	-	_	-	1139	-
Stage 2	<u>-</u>	_	_	_	1150	_
Olugo Z						
Δ	ED.		1645		0.5	
Approach	EB		WB		SB	
HCM Control Delay, s/	v 0		0		8.4	
HCM LOS					Α	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1155		-		1072
HCM Lane V/C Ratio		1100	_			0.016
HCM Control Delay (s/	voh)	0	-		-	8.4
HCM Lane LOS	v e n)					
	.\	A	-	-	-	A
HCM 95th %tile Q (veh	1)	0	-	-	-	0

HCM 6th TWSC Synchro 12 Report 02/05/2024

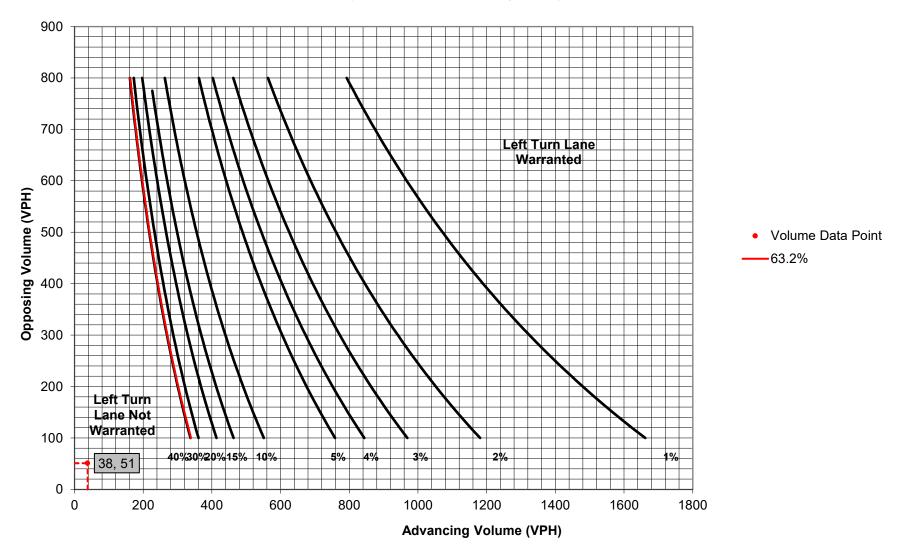
Appendix F:Auxiliary Turn Lane Warrant Analysis

		STI	IDV LOC	ΔΤΙΩΝ ΔΝ	ΝΠ ΔΝΔΙ ۷	SIS INFORMA	MOITA				
					ND ANALI	313 HVI OKIVIA	ATION				
	Mur	nicipality:		n Township		Analysis [2024			
		County:		nd County		Conducted		D			
PennDOT E	ingineering	g District:		8	Δ.	Checke	· ·	J\ TF			
			6 1 11			gency/Company Na			·b		
Intersection & App	proach De	scription: Old	Schoolhouse	Lane & Prop	osed Site Driv	eway (West) - East	bound Left				
	-	is Period:		cted (Build)		Number o	f Approach	Lanes:	1		
		sign Hour:		ak Hour		Undivided or	Divided Hi	ghway:	Undivided		
	ntersection			nalized !5				-	pe of Analysis		
Posteu	Speed Lim Type o	of Terrain:		ling		Left or Right-Tu	rn Lane An		Left Turn Lane		
				VOLUME	CALCULA	ATIONS					
			Lo		ne Volume C						
Movement		Include?	Volume	% Trucks	PCEV	1					
Wovemen	Left	Yes	23	2.0%	24		Ad	lvancing Volu	ıme: 38		
Advancing	Through	-	12	9.0%	14	1		pposing Volu			
- 3	Right	No			N/A	Left Turn Volume:					
	Left	No			N/A]					
Opposing	Through	-	26	0.0%	26						
	Right	Yes	24	2.0%	25	% Left	Turns in Ad	lvancing Volu	ime: 63.16%		
			Ri	ght Turn Lai	ne Volume C	Calculations					
Movement		Include?	Volume	% Trucks	PCEV]					
A discounting	Left	Yes			N/A				21/2		
Advancing	Through Right	-			N/A N/A	Advancing Volume: N/A Right Turn Volume: N/A					
			- 1.15			FINIDINGS		,			
				IN LAINE V	WARKAN	FINDINGS					
Let	t Turn La	ine Warrant F		<u> </u>		Right	Turn Lan	e Warrant I			
Applicable V	Narrant Fi	igure: Fig	gure 1			Applicable W	arrant Figu	ure: N	I/A		
,	Warrant I	Met?:	No			V	/arrant Me	et?: N	I/A		
			TURN	I LANE LE	NGTH CA	LCULATIONS					
Ir	ntersection	n Control:	Unsignalize	ed							
Design Hour Volur	me of Turn	ing Lane:	24								
	er Hour (A		60						_		
					Average	# of Vehicles/Cycle	2:	N/A			
	Per Hour (If	f Known):									
	Per Hour (II	f Known):		PennDOT Pub	olication 46, E	xhibit 11-6			_		
	Per Hour (II	f Known):				xhibit 11-6 eed (MPH)					
	·	f Known):		PennDOT Pub 25-35	Sp	xhibit 11-6 eed (MPH) 40-45	50	0-60			
	·				Sp	xhibit 11-6 eed (MPH)	50 High	Low			
	Туре с	of Traffic Contro	High	25-35 Low A	Turn Do High B or C	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C	High B or C	Low B or C			
	Туре с	of Traffic Contro	I High	25-35 Low	Turn Do	xhibit 11-6 eed (MPH) 40-45 emand Volume Low	High	Low			
	Туре с	of Traffic Contro	High	25-35 Low A A	Turn Do High B or C	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C	High B or C B or C	Low B or C	Feet		
	Туре с	of Traffic Contro	High	25-35 Low A A	Turn Do High B or C	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B	High B or C B or C	Low B or C B	Feet Feet		
	Туре с	of Traffic Contro	High	25-35 Low A A	Turn Do High B or C	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B e Length, Condition	High Bor C Bor C	Low B or C B			
	Туре с	of Traffic Contro	High	25-35 Low A A Left Turn	Turn D High Bor C C Lane Storage	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B e Length, Condition	High Bor C Bor C on A:	B or C B N/A N/A	Feet		
	Туре с	of Traffic Contro	High	25-35 Low A A Left Turn	Turn D High Bor C C Lane Storage	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B Conditio	High Bor C Bor C on A: on B: on C: on C: on C:	Bor C B N/A N/A N/A	Feet Feet Feet		
	Type o	of Traffic Contro Signalized Jnsignalized	High	25-35 Low A A Left Turn	Turn D High Bor C C Lane Storage	xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B Conditio	High Bor C Bor C on A: on B: on C: on C: on C:	Bor C B N/A N/A N/A N/A	Feet Feet Feet		



2/5/2024 Dwy - AM - EB LTL.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

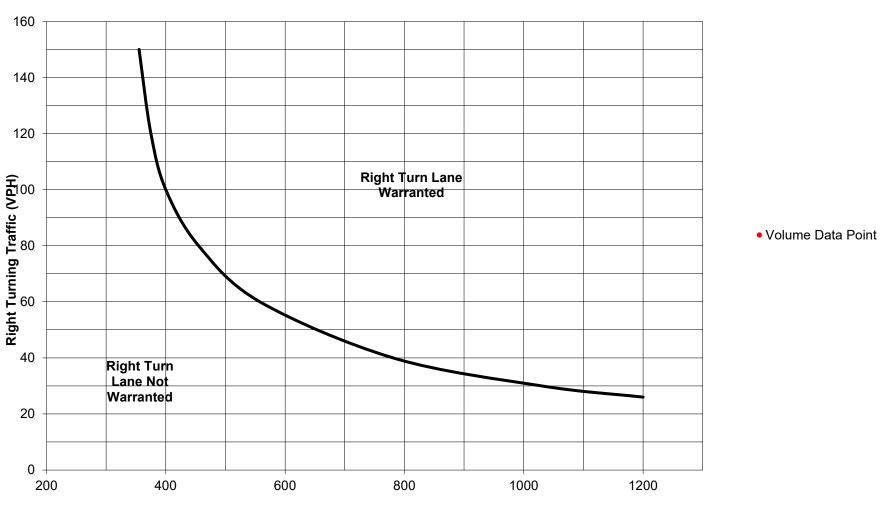


		Sī	TUDY LOC	ATION AN	ID ANALY	SIS INFORM	ATION					
	D.4							2/5/2	024			
		cipality: County:		n Township		Analysis	2/5/2 DI					
PennDOT Engi				nd County 8		Conducted By: DL Checked By: JW						
T CHILDOT LING	meering	District		0	Ag	ency/Company N	TP					
Intersection & Appro	ach Desc	cription: O	ld Schoolhouse	Lane & Propo	osed Site Drive	eway (West) - We	stbound Rig	ht				
	Analysis			cted (Build) ak Hour			of Approach		1 Undivide	d		
Inte	rsection (gn Hour:		nalized		Undivided or	Divided Hig	griway:	Ondivide	u		
Posted Spe				25				Ty	pe of Ana	lysis		
•	Type of		Rol	lling		Left or Right-Tu	ırn Lane Ana		ght Turn I			
				VOLUME	CALCULA	TIONS						
			Lo	eft Turn Lan	e Volume Ca	alculations						
Movement		Include?	Volume	% Trucks	PCEV							
	Left	Yes	23	2.0%	N/A		Ad	vancing Volu	me:	N/A		
	rough	-	12	9.0%	N/A			posing Volu		N/A		
_	Right	No			N/A			eft Turn Volu		N/A		
	Left	No			N/A							
Opposing Th	nrough	-	26	0.0%	N/A							
F	Right	Yes	24	2.0%	N/A	% Left	Turns in Ad	vancing Volu	me:	N/A		
			Ri	ght Turn Lar	ne Volume C	alculations						
Movement		Include?	Volume	% Trucks	PCEV							
	Left	Yes -	20	0.00/	0		Г1					
	nrough Right	-	26 24	0.0% 2.0%	26 25			vancing Volu ht Turn Volu		51 25		
			THE	ON LANE V	A/A DD A NIT	FINDINGS						
1 0 =				VIN LAINE V	VARRANI			101				
Left I	urn Lan	ie warran	t Findings	1		Rign	t Turn Lan	e Warrant F	indings			
Applicable Wa	rrant Fig	gure:	N/A			Applicable W	arrant Figu	re: Figu	ure 9			
Wa	arrant M	1et?:	N/A]		V	Warrant Me	et?: N	No			
			TURN	LANE LE	NGTH CAI	CULATIONS						
Inte	rsection	Control:	Unsignalize	ed								
Design Hour Volume			25									
Cycles Per I			60						-			
Cycles Per	Hour (If I	Known):			Average i	of Vehicles/Cycl	e:	N/A				
				PennDOT Pub	lication 46, Ex	chibit 11-6			_			
					Spe	eed (MPH)						
	Type of	Traffic Cont	trol	25-35	Turn Do	40-45	50	-60				
			High	Low	High	Low	High	Low				
		ignalized	A	А	B or C	B or C	B or C	B or C	1			
	Un	nsignalized	A	A	С	В	B or C	В	_			
				Right Turn I	Lane Storage	Length, Condition	on A:	N/A	Feet			
						Conditi	on B:	N/A	Feet			
						Conditi	on C:	N/A	Feet			
				Require	d Right Turn	Lane Storage Le	ngth:	N/A	Feet			
							Additio	nal Findings	-			
dditional Comments / J							Additio	N/A				
		ignalized nsignalized		A A	High B or C C	B or C B Length, Condition Condition Condition	B or C B or C on A: on B: on C: ngth:	N/A N/A N/A N/A	Feet Feet Feet			



2/5/2024 Dwy - AM - EB LTL.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



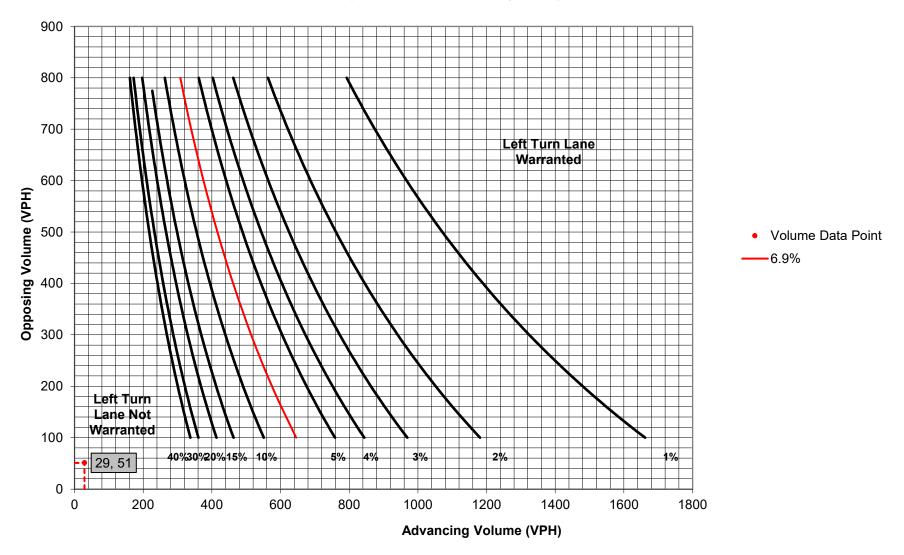
Advancing Volume including Right Turns (VPH)

			STUD	Y LOC	ATION AN	ID ANALY	SIS INFORM	IATION				
								_		2024		
	Mı	unicipality:			n Township		Analysis	2/5/2 D				
DownDOT	Engineeri	County: ng District:	(nd County 8		Conduct					
reiliboi	Engineen	ing District.			5		Checked By: JW Agency/Company Name: TPD					
Intonoction C A	D		Old Cab	aalbausa	Jana ⁹ Drane		eway (East) - Eas					
intersection & A	pproacn D	escription:	Olu Sch	loomouse	Lane & Propo	osed site Driv	eway (East) - Eas	ibouria Lei				
	-	sis Period:	20		cted (Build)		Number of Approach Lanes: 1					
		sign Hour:			ak Hour		Undivided o	r Divided	Highway:	Undivided		
		on Control:			nalized							
Poste		mit (MPH): of Terrain:			ling		Type of Analysis Left or Right-Turn Lane Analysis?: Left Turn Lane					
	- 7,6-0				VOLUME	CALCULA						
				1.4	eft Turn Lan							
							1					
Moveme	_	Include	? V	olume	% Trucks	PCEV	-		A	30		
A du (= = = : = =	Left	Yes		1	2.0%	2	-	•	Advancing Volu			
Advancing	Through			23	9.0%	27	-		Opposing Volu			
	Right Left	No No				N/A N/A	-		Left Turn Volu	ıme: 2		
Opposing	Through			47	0.0%	47	+					
Оррозін	Right	Yes		3	2.0%	47	% Lef	t Turns in A	Advancing Volu	ime: 6.90%		
				Rig	ght Turn Lar							
Moveme	nt	Include	2 V	olume	% Trucks	PCEV						
Wioverne	Left	Yes	. ,	Olullic	70 Trucks	N/A						
Advancing	Through			47	0.0%	N/A	-	Advancing Volume: N/A				
	Right	-		3	2.0%	N/A			Right Turn Volu			
				TUR	RN LANE V	VARRAN'	T FINDINGS					
Le	eft Turn L	ane Warra	ant Fin	dings			Righ	nt Turn La	ane Warrant F	indings		
Applicable	Warrant	Figure:	Figu	re 1			Applicable V	Varrant Fi	igure: N	I/A		
	Warrant	Met?:	No]		,	Warrant I	Met?: N	I/A		
						NCTUCA				.,,		
				TUKN	LANE LE	NGTH CA	LCULATIONS	•				
		on Control:	Uı	nsignalize	d							
Design Hour Volu		-		2								
		Assumed): If Known):		60		Average	# of Vehicles/Cyc	le:	N/A	1		
2,5.25					Down DOT Dub	_				_		
					PennDOT Pub		eed (MPH)			1		
	Type	of Traffic Co	ontrol		25-35		40-45		50-60			
	.,,,,			111-1-	1		emand Volume	115-6	12			
		Signalized		High A	Low A	High B or C	Low B or C	High B or C	Low B or C	-		
		Unsignalized	d	A	A	C	В	B or C	В]		
			_		Left Turn	Lane Storage	Length, Condit	on A:	N/A	Feet		
					zeit iuiiii	-and storage	Condit		N/A	Feet		
							Condit		N/A	Feet		
					Requir	ed Left Turn	Lane Storage Le	ngth:	N/A	Feet		
								Addit	ional Findings			
Additional Commen	ts / Justifica	ations:							N/A	<u> </u>		



2/5/2024 E Dwy - AM.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

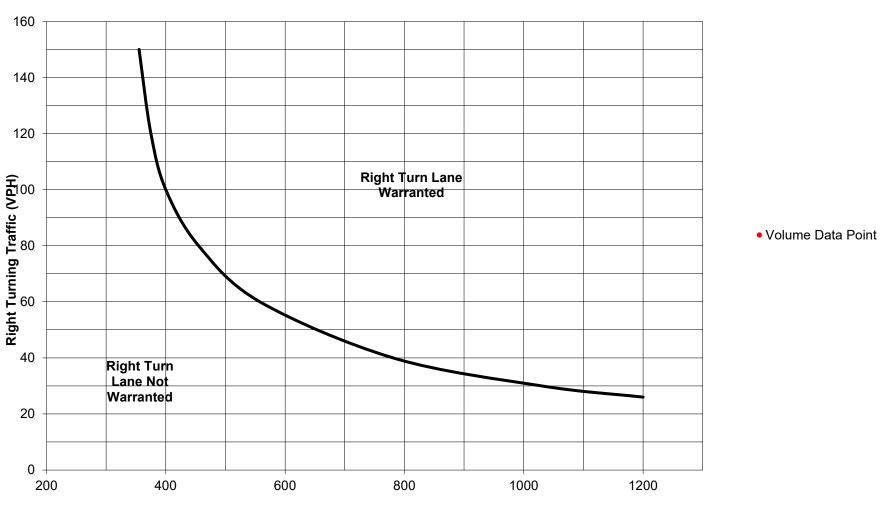


			STUDY	LOCA	ATION AN	ID ANA	LYSI	S INFORM	IATION			
	Mu	nicipality:	Upp	er Allen	Township			Analysis	Date:	2/5/2	.024	
		County:	Cur	nberlar	nd County			Conduct	ed By:	DI	L	
PennDOT	Engineerin	g District:		8	3			Check	ed By:	JV	/	
							Agen	cy/Company I	Name:	TP	D	
Intersection & A	oproach De	escription:	Old Schoo	lhouse	Lane & Propo	osed Site D	rivewa	ay (East) - We	stbound R	ight		
	Analy	sis Period:	2035	Projec	cted (Build)			Number	of Approa	ch Lanes:	1	
	De	sign Hour:	A	M Pea	Peak Hour Undivided or Divided High					Highway:	Undivided	
	Intersectio			Unsignalized								
Posteo	d Speed Lin			25				r afa a a Bialas T	·		pe of Analy	
	туре	of Terrain:		Roll	ing			Left or Right-T	urn Lane /	Analysis :: Ki	ght Turn Lar	ie
					VOLUME							
				Le	ft Turn Lan	e Volume	Calc	ulations				
Moveme		Include			% Trucks	PCEV						
	Left	Yes	1		2.0%	N/A				Advancing Volu		N/A
Advancing	Through	-	2	3	9.0%	N/A				Opposing Volu		N/A
	Right	No				N/A				Left Turn Volu	me:	N/A
Opposing	Left Through	No -	4	7	0.0%	N/A N/A						
Оррозіні	Right	Yes	3		2.0%	N/A		% Lef	t Turns in	Advancing Volu	me:	N/A
	•		•	Rig	jht Turn Lar	ne Volum	e Cal	culations		-		
Moveme	nt	Include	? Volu	me	% Trucks	PCEV						
	Left	Yes				0						
Advancing	Through			7	0.0%	47		Advancing Volume: 51				
	Right	-	3	3 2.0% 4 Right Turn Volum					me:	4		
				TUR	N LANE V	VARRA	NT F	INDINGS				
Le	ft Turn La	ane Warra	ınt Findir	ıgs				Rigl	nt Turn L	ane Warrant F	indings	
Applicable	Warrant F	igure:	N/A					Applicable V	Varrant F	igure: Figi	ure 9	
	Warrant	Met?:	N/A						Warrant I	Met?:	No	
			T	URN	LANE LE	NGTH (CALC	ULATIONS	S			
	Intersectio	n Cantral.		gnalized								
Design Hour Volu			OTIST	4	u							
-	Per Hour (A	-		60								
	Per Hour (I					Avera	ge#o	f Vehicles/Cyc	:le:	N/A		
				P	PennDOT Pub	lication 46	6, Exhi	bit 11-6			_	
						•	•	I (MPH)				
	Туре	of Traffic Co	ntrol		25-35	Turi		0-45 and Volume		50-60		
				High	Low		gh	Low	High	Low	1	
		Signalized		Α	A		or C	B or C	B or C	B or C		
		Unsignalized	1	Α	A	(2	В	B or C	В	_	
					Right Turn I	Lane Stora	age Le	ength, Condit	ion A:	N/A	Feet	
								Condit	ion B:	N/A	Feet	
								Condit	ion C:	N/A	Feet	
					Require	d Right Tu	ırn Laı	ne Storage Le	ength:	N/A	Feet	
									Addit	tional Findings	- :	
Additional C	ha / 1	Name								N/A		
Additional Comment	is / Justifica	tions:										



2/5/2024 E Dwy - AM.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



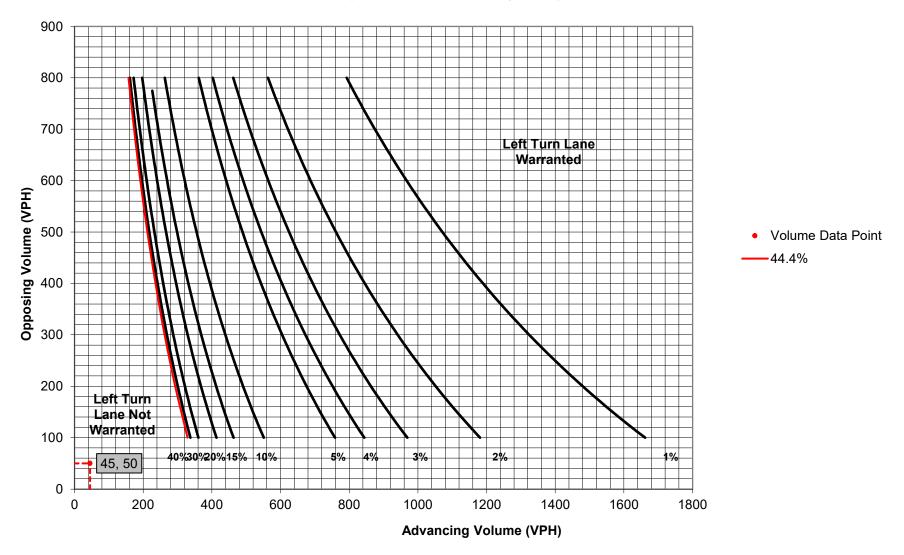
Advancing Volume including Right Turns (VPH)

		C.	TUDVIOC	ΛΤΙΩΝΙ ΛΝ	ID VNVI A	SIS INFORM	IATION			
			TODT LOC.	ATION AN	ND ANALT	313 HALOUM	ATION			
	Mur	nicipality:		n Township		Analysis		2/5/2		
		County:		ind County		Conducte		D		
PennDOT E	ngineerin	g District:	-	8		Check		JV		
					Ag	gency/Company N	Name:	ТР	D	
Intersection & App	oroach De	scription: O	ld Schoolhouse	e Lane & Propo	osed Site Drive	eway (West) - Eas	stbound Left			
	Analys	is Period:		cted (Build)			of Approach		1	
		ign Hour:		ak Hour		Undivided o	r Divided Hi	ghway:	Undivided	
	tersection			nalized						_
Posted S	Speed Lim			25		left en Biebt T			pe of Analysis eft Turn Lane	
	туре о	f Terrain:	KUI	lling		Left or Right-T	urn Lane An	alysisr: L	ert rum Lane	
					CALCULA					
			Lo	eft Turn Lan	e Volume C	alculations				
Movement	t	Include?	Volume	% Trucks	PCEV					
	Left	Yes	19	2.0%	20		Ad	vancing Volu	ıme: 4	5
Advancing	Through	-	23	5.0%	25		0	pposing Volu	ıme: 5	0
	Right	No			N/A		L	eft Turn Volu	ıme: 2	0
_	Left	No			N/A					
Opposing	Through	-	23	11.0%	27					
	Right	Yes	22	2.0%	23		Turns in Ad	vancing Volu	ıme: 44.4	14%
			Ri	ght Turn Lar	ne Volume C	alculations				
Movement		Include?	Volume	% Trucks	PCEV					
	Left	Yes	22	11.00/	N/A					/-
Advancing	Through	-	23	•				vancing Volu		/A
	Right		22	2.0%	IN/A	Right Turn Volume: N/A				
			TUF	RN LANE V	WARRANT	FINDINGS				
Left	t Turn La	ne Warran	t Findings			Righ	nt Turn Lan	e Warrant F	indings	
Applicable W	Varrant F	igure: F	igure 1]		Applicable V	Varrant Figu	ıre: N	I/A	
V	Warrant I	Met?:	No]		,	Warrant Me	et?: N	I/A	
			TURN	N LANE LE	NGTH CA	LCULATIONS	<u> </u>			
In Design Hour Volun	itersectior ne of Turn		Unsignalize 20	ea						
	er Hour (A		60							
	er Hour (If				Average	# of Vehicles/Cyc	le:	N/A		
				PennDOT Pub	lication 46, E	xhibit 11-6				
					Sp	eed (MPH)				
	Type	of Traffic Cont	trol	25-35		40-45	50)-60		
	,,,,,					emand Volume	111-1	1		
	-	Signalized	High A	Low A	High B or C	Low B or C	High B or C	Low B or C	-	
		Insignalized	A	A	C	В	B or C	В		
			•	Left Turn I	Lane Storage	Length, Conditi	on A:	N/A	Feet	
				LCIT TUITT	Lanc Storage	Conditi		N/A	Feet	
						Conditi		N/A	Feet	
				Requir	ed Left Turn	Lane Storage Le		N/A	Feet	
				nequil	Ca Ecit Iuill	-and storage Le			_	
							Additio	nal Findings N/A		
Additional Comments	/ Justificat	ions:							_	



2/5/2024 W Dwy - PM.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

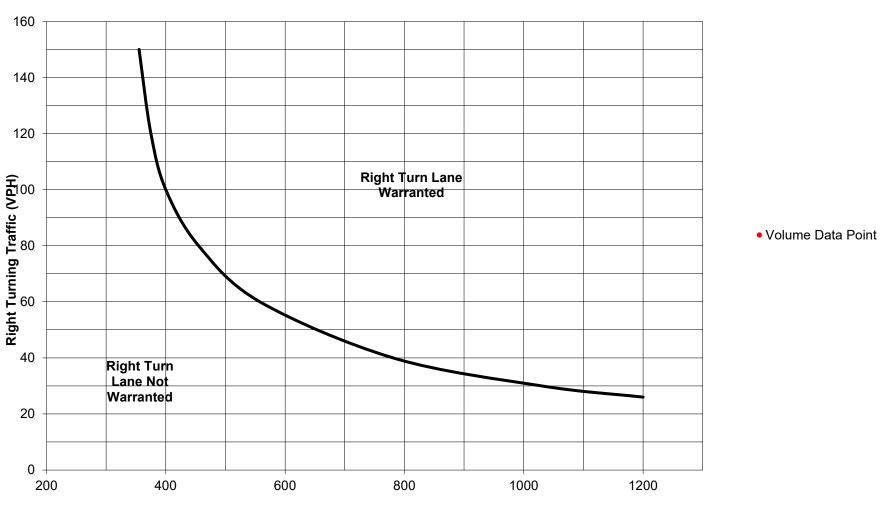


		310	JDY LOC	ATION AI	10 /111/121	SIS IIVI ORIVI			
	Mu	nicipality:	Upper Allei	n Township		Analysis	Date:	2/5/	2024
		County:	Cumberla	nd County		Conducte	d By:)L
PennDOT	Engineerin	g District:	8	8		Checke	d By:	J١	W
					A	gency/Company N	ame:	TI	PD
ntersection & A _l	oproach De	scription: Old S	Schoolhouse	Lane & Propo	osed Site Driv	eway (West) - We	stbound Rig	tht	
	-	sis Period:		cted (Build)			f Approach		1
		sign Hour:		ak Hour		Undivided or	Divided Hi	ghway:	Undivided
	Intersection Speed Lim			nalized !5				T-7	ype of Analysis
Postet		of Terrain:		ling		Left or Right-Tu	ırn Lane An		Right Turn Lane
				VOLUME	CALCULA	ATIONS			
			Le		e Volume C				
Moveme	nt	Include?	Volume	% Trucks	PCEV				
	Left	Yes	19	2.0%	N/A]	Ac	lvancing Vol	ume: N/A
Advancing	Through	-	23	5.0%	N/A			pposing Vol	
	Right	No			N/A		L	eft Turn Vol	ume: N/A
	Left	No			N/A				<u></u>
Opposing	Through	-	23	11.0%	N/A				
	Right	Yes	22	2.0%	N/A	.	Turns in Ac	Ivancing Vol	ume: N/A
			Rig	ght Turn Lar	ne Volume (Calculations			
Moveme	nt Left	Include? Yes	Volume	% Trucks	PCEV 0				
Advancing	Through	res	23	11.0%	27	-	Δι	Ivancing Vol	ume: 50
7.0.70	Right	-	22	2.0% 23 Right Turn Volume:					
			TUR	RN LANE V	NARRAN'	T FINDINGS			
Le	ft Turn La	ne Warrant F	indings			Right	t Turn Lar	e Warrant	Findings
Applicable	Warrant F	igure:	N/A]		Applicable W	arrant Fig	ure: Fig	ure 9
	Warrant	Met?:	N/A	- 		v	Varrant M	et?:	No
			TURN	I LANE LE	NGTH CA	LCULATIONS			
	Intersection	n Control:	Unsignalize						
esign Hour Volu			23	···					
Cycles	Per Hour (A	Assumed):	60						
Cycles	Per Hour (I	f Known):			Average	# of Vehicles/Cycl	e:	N/A	
			1	PennDOT Pub	olication 46, E				_
				25.25	Sp	eed (MPH)	=	2.00	
	Туре	of Traffic Contro	1	25-35	Turn D	40-45 emand Volume	5	0-60	-
	L		High	Low	High	Low	High	Low	
		Signalized	A	A	B or C	B or C	B or C	B or C	
		Jnsignalized	A	А	С	В	B or C	В	
				Right Turn	Lane Storage	Length, Condition		N/A	Feet
						Condition	on B:	N/A	Feet
						Condition	on C:	N/A	Feet
				Require	d Right Turn	Lane Storage Lei	ngth:	N/A	Feet
			_				Additio	nal Finding	
ditional Commen	ts / Justificat	tions:					Additio	onal Finding: N/	



2/5/2024 W Dwy - PM.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



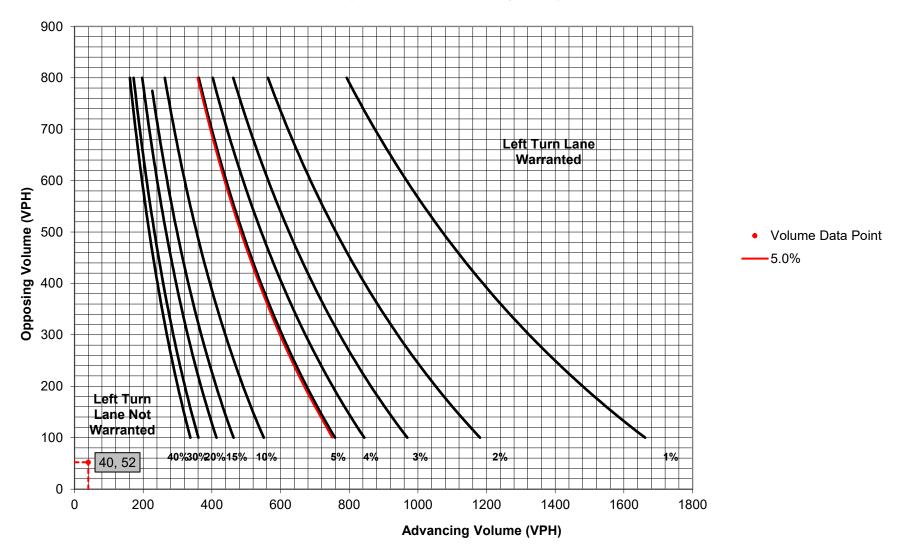
Advancing Volume including Right Turns (VPH)

		S	TUDY LOC	ATION AN	ID ANALY	SIS INFORM	ATION			
	Mur	nicipality:	Upper Alle	n Township		Analysis	Date:	2/5/2	2024	
		County:	Cumberla	and County		Conducte	d By:	D	L	
PennDOT Er	ngineering	g District:		8		Checked By: JW				
					Ag	gency/Company N	lame:	TP	D	
Intersection & App	roach Des	scription:	Old Schoolhous	e Lane & Propo	osed Site Driv	eway (East) - East	bound Left			
	Analys	is Period:	2035 Proje	ected (Build)			of Approach		1	
		ign Hour:		ak Hour						
	tersection		_	nalized						
Posted S	Speed Lim	it (MPH): f Terrain:		25 Iling		Left or Right-Tu	ırn Lane An		rpe of Analysis eft Turn Lane	
	туре о	i ieiiaiii.	No		CALCULA		ann Lane An	arysis: L	ert Turri Larie	
					CALCULA					
				eft Turn Lan		alculations				
Movement		Include?		% Trucks	PCEV				10	
A dum of the	Left	Yes	1	2.0%	2			vancing Volu		
Advancing	Through	- No	35	5.0%	38			pposing Volu		
	Right Left	No No			N/A N/A		L	eft Turn Volu	me:	
Opposing	Through	-	41	11.0%	48					
-668	Right	Yes	3	2.0%	4	% Left	Turns in Ad	vancing Volu	me: 5.00%	
			Ri	ght Turn Lar	ne Volume C	Calculations				
Movement		Include?	Volume	% Trucks	PCEV					
	Left	Yes			N/A					
Advancing	Through	-			N/A	N/A A			me: N/A	
	Right	-			N/A	N/A Right Turn Volume:				
			TUI	RN LANE V	WARRAN1	FINDINGS				
Left	Turn La	ne Warraı	nt Findings			Righ	t Turn Lan	e Warrant F	indings	
Applicable W			Figure 1]		Applicable W			I/A	
	Warrant N		No]			Varrant Me		I/A	
•				J					1/ A	
			TURI	N LANE LE	NGTH CA	LCULATIONS				
	tersection	_	Unsignalize 2	ed						
Design Hour Volum	er Hour (A		60							
	er Hour (A er Hour (If		00		Average	# of Vehicles/Cycl	e:	N/A	1	
5,0,0010	(11			Down DOT 2:	_					
				PennDOT Pub		eed (MPH)				
	Type o	of Traffic Cor	ntrol	25-35		40-45	50)-60		
	,,,,,,		High	Low	Turn De High	emand Volume Low	High	Low	-	
		Cianalizad	A	A	B or C	B or C	B or C	B or C	1	
		Signalized			С	В	B or C	В]	
		Insignalized	A	A	•					
			A	•	Lane Storage	Length, Condition	on A:	N/A	Feet	
			A	•	Lane Storage	Length, Condition		N/A N/A		
			A	•	Lane Storage	Condition	on B:	N/A	Feet	
			A	Left Turn I		Condition Condition	on B:	N/A N/A	Feet Feet	
			A	Left Turn I		Condition	on B: on C: ngth:	N/A N/A N/A	Feet Feet Feet	
			A	Left Turn I		Condition Condition	on B: on C: ngth:	N/A N/A N/A	Feet Feet Feet	
dditional Comments,	U	Insignalized	A	Left Turn I		Condition Condition	on B: on C: ngth:	N/A N/A N/A	Feet Feet Feet	



2/5/2024 E Dwy - PM.xlsx

Figure 1. Warrant for left turn lanes on two-lane roadways (speeds to 35 mph, unsignalized and signalized intersections)
(L = % Left Turns in Advancing Volume)

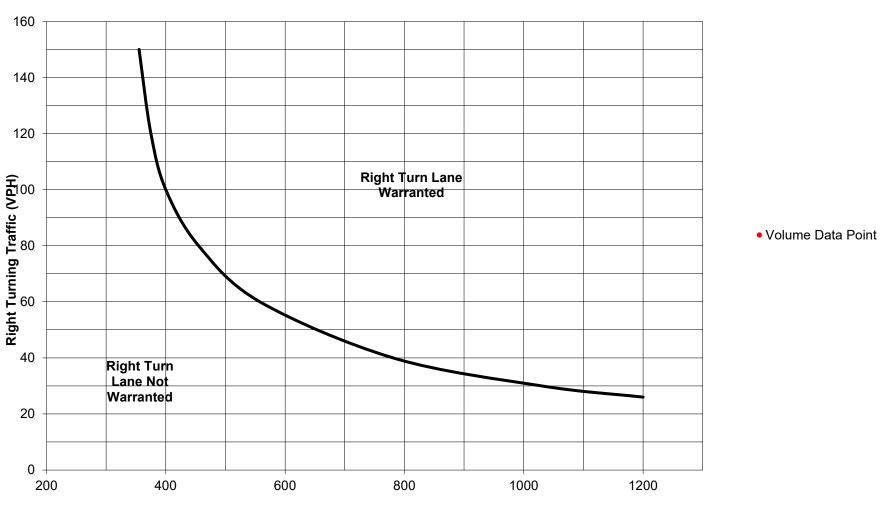


		51	UDY LOC	ATION AN	ND ANALY	313 HYPURIVIA	ION	
					_			
	Mur	nicipality:		n Township		Analysis Da		5/2024
PennDOT	Engineering	County:		nd County 8		Conducted E Checked E	·	JW
reiiiboi	Liigineering	g District.		,	As	gency/Company Nam	•	TPD
Intersection & A	pproach De	scription: Old	Schoolhouse	Lane & Propo		eway (East) - Westbo		
	-	sis Period:		cted (Build) ak Hour		Number of A Undivided or Di	pproach Lanes:	1 Undivided
1	Intersection			nalized		Onalviaca of D	viaca mgmway.	Ondivided
Poster	d Speed Lim	nit (MPH):		.5				Type of Analysis
	Туре о	of Terrain:	Rol	ling		Left or Right-Turn	Lane Analysis?:	Right Turn Lane
				VOLUME	CALCULA	ATIONS		
			Le	eft Turn Lan	ne Volume C	alculations		
Movemen	nt	Include?	Volume	% Trucks	PCEV]		
	Left	Yes	1	2.0%	N/A		Advancing Vo	
Advancing	Through	-	35	5.0%	N/A		Opposing Vo	
	Right	No			N/A		Left Turn Vo	olume: N/A
Onnesias	Left	No	41	11.00/	N/A			
Opposing	Through Right	- Yes	41 3	11.0% 2.0%	N/A N/A	% Loft Tu	rns in Advancing Vo	olume: N/A
	Mgnt	163	-		ne Volume C	1	This in Advancing vo	ounie.
Moveme	nt	Include?	Volume	% Trucks	PCEV]		
Movemen	Left	Yes	Volume	76 HUCKS	0			
Advancing	Through	-			0		Advancing Vo	olume: 0
	Right	-		0 Right Turn Volume:				
			TUR	N LANE V	WARRAN7	Γ FINDINGS		
Le	eft Turn La	ne Warrant	Findings			Right T	urn Lane Warran	t Findings
Applicable	Warrant F	igure:	N/A			Applicable War	rant Figure: Fi	gure 9
	Warrant I	Met?:	N/A			Wa	rrant Met?:	No
			14,74	<u> </u>				110
			TUDA		NOTILOA	LCILLATIONS		
					NGTH CA	LCULATIONS		
	Intersection		Unsignalize		NGTH CA	LCULATIONS		
Design Hour Volu	ume of Turn	ning Lane:	Unsignalize 0		NGTH CA	LCULATIONS		
Design Hour Volu	ume of Turn Per Hour (A	ning Lane:	Unsignalize				N/A	
Design Hour Volu	ume of Turn	ning Lane:	Unsignalize 0 60	d	Average	# of Vehicles/Cycle:	N/A	
Design Hour Volu	ume of Turn Per Hour (A	ning Lane:	Unsignalize 0 60	d	Average blication 46, E	# of Vehicles/Cycle:	N/A	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known):	Unsignalize 0 60	d	Average blication 46, E Sp	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45	N/A 50-60	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane:	Unsignalize 0 60	PennDOT Pub	Average blication 46, E Sp Turn Do	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume	50-60	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If Type o	ning Lane: Assumed): f Known):	Unsignalize 0 60	PennDOT Pub	Average blication 46, E Sp Turn Di High	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45	50-60 High Low	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known):	Unsignalize 0 60	PennDOT Pub	Average blication 46, E Sp Turn Do	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume Low	50-60 High Low	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro	Unsignalize 0 60 I High	25-35 Low A A	Average blication 46, E Sp Turn Di High B or C	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C	50-60 High Low BorC BorC BorC B	Feet
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro	Unsignalize 0 60 I High	25-35 Low A A	Average blication 46, E Sp Turn Di High B or C	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B	50-60 High Low Bor C Bor C Bor C B	Feet Feet
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro	Unsignalize 0 60 I High	25-35 Low A A	Average blication 46, E Sp Turn Di High B or C	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume Low Bor C B B e Length, Condition	50-60 High Low BorC BorC BorC B A: N/A B: N/A	
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro	Unsignalize 0 60 I High	PennDOT Pub 25-35 Low A A Right Turn	Average blication 46, E Sp Turn Di High B or C C Lane Storage	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45 emand Volume Low B or C B Part	50-60 High Low B or C B or C B or C B A: N/A B: N/A C: N/A	Feet
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro	Unsignalize 0 60 I High	PennDOT Pub 25-35 Low A A Right Turn	Average blication 46, E Sp Turn Di High B or C C Lane Storage	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45	High Low Bor C Bor C Bor C B A: N/A B: N/A C: N/A c: N/A Additional Findin	Feet Feet Feet
Design Hour Volu	ume of Turn Per Hour (A Per Hour (If	ning Lane: Assumed): f Known): of Traffic Contro Signalized Jnsignalized	Unsignalize 0 60 I High	PennDOT Pub 25-35 Low A A Right Turn	Average blication 46, E Sp Turn Di High B or C C Lane Storage	# of Vehicles/Cycle: xhibit 11-6 eed (MPH) 40-45	High Low Bor C Bor C Bor C B A: N/A B: N/A C: N/A c: N/A Additional Findin	Feet Feet Feet



2/5/2024 E Dwy - PM.xlsx

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



Advancing Volume including Right Turns (VPH)